

**Design Specifications:**

- ASCE 7-10
- ACI 318-14
- ACI 530-13
- AISC 360-10
- ASCE 341-10
- AISI S100-12
- ANSI / AWC NDS-15

**Design Loads:**

**Roof Loads:**

- Dead (Top Chord): 15 psf
- Dead (Bott. Chord): 10 psf
- Live (Top Chord): 20 psf
- Live (Bott. Chord): 5 psf

**Wind Loads:**

- Risk Category: III
- Velocity: 120 mph
- Exposure: C

**Seismic Loads:**

- Site Class: D
- Seismic Design Category: B
- Seismic Force-Resisting System: U.F.S.W.
- 0.0661 (O.R.C.M.F.)
- 6.5 (O.R.C.M.F.)
- E.L.F.P.

**Other Values:**

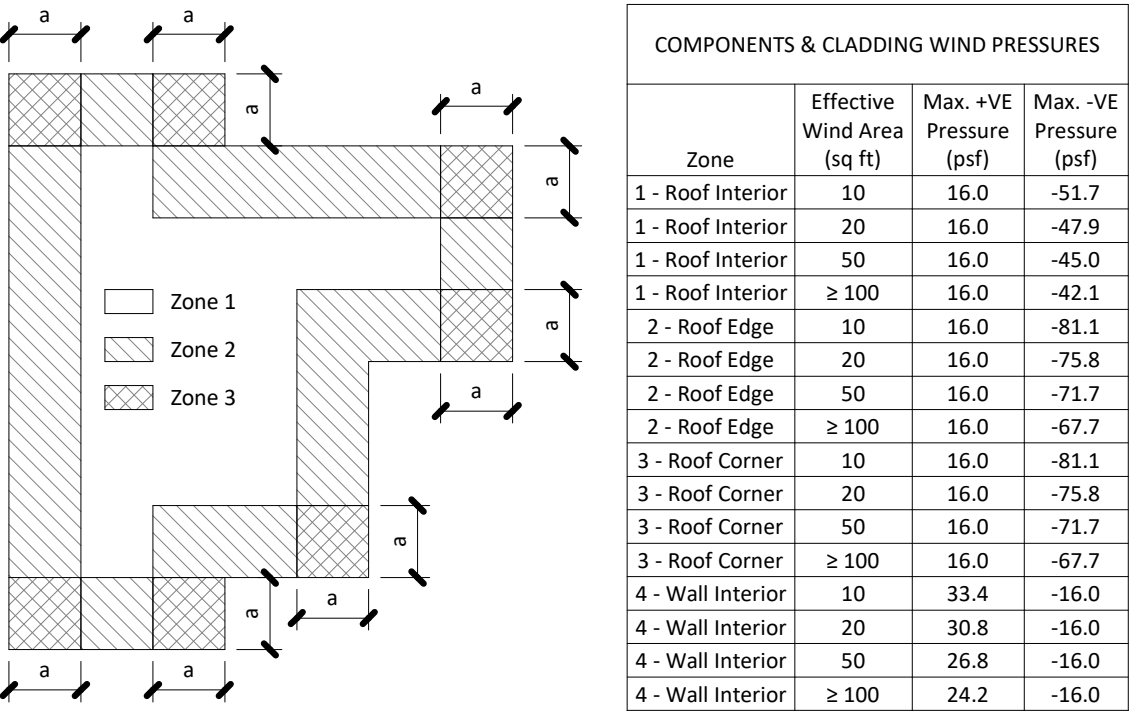
- 1.25
- 0.211 g
- 0.065 g
- 0.104 g

**Ceiling Loads:**

- Dead Load: 10 psf

**Design Loading Notes:**

- SEE COMPONENTS AND CLADDING TABLE FOR DESIGN WIND PRESSURES.
- SEE NET UPLIFT DIAGRAM FOR ROOF FRAMING DUE TO WIND PRESSURES.



**COMPONENTS & CLADDING WIND ZONE DIAGRAM**

- THE COMPONENTS & CLADDING (C&C) WIND PRESSURES SHOWN ASSUME A MEAN ROOF HEIGHT OF 65'-0" ABOVE FINISHED FLOOR ELEVATION. ALL COMPONENTS SHALL BE DESIGNED TO RESIST THE PROVIDED PRESSURES, WHICH SHALL BE CLEARLY DEFINED ON ALL SHOP DRAWINGS. REFER TO WIND ZONE DIAGRAM FOR ZONE LOCATIONS, PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM SURFACES, RESPECTIVELY.

- THE COMPONENTS & CLADDING WIND ZONE DIAGRAM IS GENERALIZED TO SHOW ALL POSSIBLE CONDITIONS. THE DIAGRAM SHAPE MAY NOT MATCH THE SPECIFIC LAYOUT FOR THIS PROJECT.

- A = 16'-0"

- INTERNAL PRESSURE COEFFICIENT = +0.18

**GENERAL:**

- THE STRUCTURAL SYSTEMS SHOWN ON THESE DOCUMENTS HAVE BEEN DESIGNED FOR THE FINAL, IN PLACE USAGE OF THE STRUCTURE BASED ON THE INTENDED OCCUPANCY AND CODE REQUIREMENTS. WHILE GENERAL CONTRACTIBILITY HAS BEEN CONSIDERED, THE STRUCTURAL SYSTEMS HAVE NOT BEEN DESIGNED TO ACCOMMODATE SPECIFIC CONSTRUCTION MEANS AND METHODS THAT MIGHT BE UTILIZED BY THE CONTRACTOR.
- THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING DIMENSIONS PRIOR TO FABRICATION.
- THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY OBSERVED DISCREPANCIES IN DIMENSIONS, DETAILING, OR OTHER ITEMS AS SHOWN ON THE PLANS OR SPECIFIED PRIOR TO PROCEEDING WITH WORK RELATING TO SAID DISCREPANCIES.
- THE CONTRACTOR SHALL NOT ALTER OR MODIFY WORK SHOWN ON THE STRUCTURAL DRAWINGS WITHOUT RECEIVING WRITTEN APPROVAL FROM THE ENGINEER.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUPPLYING SHOP DRAWINGS FOR WOOD JOISTS & TRUSSES, JOIST GIRDERS, BAR JOISTS, STRUCTURAL STEEL, METAL DECK, REINFORCING STEEL, CONCRETE MASONRY UNITS AND ACCESSORIES, PLAN AND ELEVATION VIEWS OF CONCRETE MASONRY WALL ELEVATIONS INCLUDING CONTROL JOINT AND EXPANSION JOINT LOCATIONS, MORTAR AND GROUT, AND CONCRETE MIX DESIGNS. SHOP DRAWINGS MUST BE REVIEWED FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIONS OF CONSTRUCTION, AND SAFETY PRECAUTIONS AND PROGRAMS INSTALLED THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR, AND SHALL BE STAMPED "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL. SHOP DRAWINGS SUBMITTED WITHOUT THE CONTRACTOR'S STAMPED APPROVAL WILL BE RETURNED "REJECTED". ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.
- SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR OTHER PERTINENT INFORMATION RELATED TO THE STRUCTURAL WORK AND COORDINATE AS REQUIRED. THESE STRUCTURAL DRAWINGS ARE INTENDED TO BE INCLUDED IN A COMPLETE SET OF CONSTRUCTION DOCUMENTS, INCLUDING BUT NOT LIMITED TO, ARCHITECTURAL DRAWINGS, CIVIL DRAWINGS, AND MECHANICAL/ELECTRICAL/PLUMBING DRAWINGS. CONTRACTOR SHALL VERIFY COORDINATION OF THESE DRAWINGS WITH CONTENTS OF ARCHITECTURAL SETS SPECIFIED AND ONLY PROCEED WITH BIDDING AND CONSTRUCTION AFTER SUCH HAS TAKEN PLACE.
- THE BUILDING AND THE INDEPENDENT STRUCTURAL COMPONENTS SHOWN IN THESE DOCUMENTS ARE NOT STRUCTURALLY STABLE UNTIL ALL CONNECTIONS, FRAMING, SHEAR WALLS, DIAPHRAGMS, PERMANENT BRACING, METAL DECKING, INTERIOR AND EXTERIOR CONCRETE SLABS ON GRADE, AND EXTERIOR OR INTERIOR LOAD-BEARING WALLS ARE COMPLETE AND HAVE ACHIEVED THEIR DESIGN STRENGTH. CONTRACTOR IS SOLELY RESPONSIBLE FOR MAINTAINING STRUCTURAL STABILITY DURING ERECTION AND CONSTRUCTION. TEMPORARY BRACING SYSTEMS SHALL REMAIN IN PLACE UNTIL ALL STRUCTURAL WORK IS COMPLETE.
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL EXISTING DIMENSIONS AND CONDITIONS OF THE EXISTING BUILDING AND REPORTING DISCREPANCIES FROM THE ASSUMED CONDITIONS SHOWN ON THE STRUCTURAL DRAWINGS TO THE ENGINEER OF RECORD PRIOR TO FABRICATION AND ERECTION OF ANY MEMBER.
- THE CONTRACTOR SHALL COORDINATE THE ROOF DRAINAGE SYSTEM WITH THE ARCHITECT AS REQUIRED TO ENSURE THAT NO MORE THAN 3 1/2" OF WATER CAN ACCUMULATE BEFORE ENTERING AN OVERFLOW DRAINAGE SYSTEM.
- CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, ORDINANCES REGULATIONS AND LAWS.
- CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACT DOCUMENTS DURING BIDDING SHALL IMMEDIATELY NOTIFY THE SEOR. THE SEOR WILL RESOLVE THE CONDITION AND ISSUE A WRITTEN CLARIFICATION.
- THE STRUCTURAL ENGINEER REVIEWS SUBMITTALS TO CONFIRM THAT THE SUBMITTAL IS IN GENERAL CONFORMANCE WITH THE DESIGN CONCEPT PRESENTED IN THE CONTRACT DOCUMENTS. QUANTITIES AND DIMENSIONS ARE NOT CHECKED. NOTATIONS ON SUBMITTALS DO NOT AUTHORIZE CHANGES TO THE CONTRACT SUM. CHECKING OF THE SUBMITTAL BY THE STRUCTURAL ENGINEER SHALL NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR DEVIATIONS FROM THE CONTRACT DOCUMENTS AND FROM ERRORS OR OMISSIONS IN THE SUBMITTAL.

- IN ADDITION TO THE ABOVE, THE STRUCTURAL ENGINEER'S REVIEW OF THE DELEGATED ENGINEER SUBMITTALS IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED, SIGNED AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA AND SOUND ENGINEERING JUDGEMENT IN PREPARATION OF THE SUBMITTAL. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. THE DELEGATED ENGINEER IS SOLELY RESPONSIBLE FOR HIS/HER DESIGN, INCLUDING BUT NOT LIMITED TO THE ACCURACY OF HIS/HER CALCULATIONS AND COMPLIANCE WITH THE APPLICABLE CODES AND STANDARDS.

15. THE USE OF DIGITAL FILES OR COPIES OF THE STRUCTURAL DRAWINGS SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZES AND QUANTITIES OF MATERIALS AS REQUIRED FOR THE PREPARATION OF COMPLETE AND ACCURATE SHOP DRAWINGS.

**DEFERRED SUBMITTALS:**

- DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE REGISTERED DESIGN PROFESSIONAL AND SHALL NOT BE INSTALLED UNTIL APPROVED BY SAID ENGINEER AND THE COR.

THE FOLLOWING ITEMS ARE CONSIDERED DEFERRED SUBMITTALS BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE:

- PRE-MANUFACTURED CANOPIES & THEIR CONNECTIONS
  - FORMWORK, SHORING, AND BACKSHORING
  - ROOF TOP EQUIPMENT AND ANCHORAGES
  - DETACHED ANCHORAGE DEVICES
- D. DRAWINGS AND CALCULATIONS FOR ALL MODULAR FRAMING UNITS

**STRUCTURAL ENGINEER SITE OBSERVATIONS:**

- THE CONTRACT STRUCTURAL DRAWINGS & SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE.
- THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS & PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

- PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF BSE STRUCTURAL ENGINEERS LLC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE CLIENT AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

**SLAB ON GRADE:**

- WELDED WIRE FABRIC SHALL BE SUPPLIED IN SHEETS ONLY. ROLLS WILL NOT BE PERMITTED. (AS REQUIRED ON CONSTRUCTION DOCUMENTS.)
- WELDED WIRE FABRIC SHALL BE SUPPORTED ON CHAIRS OR BLOCKS PRIOR TO CONCRETE PLACEMENT. MESH SHALL NOT BE HOOKED AND PULLED UP DURING CONCRETE PLACEMENT. (AS REQUIRED ON CONSTRUCTION DOCUMENTS.)
- WELDED WIRE FABRIC SHALL HAVE END AND EDGE LAPS OF ONE FULL MESH PLUS 2" BETWEEN CROSS WIRES. WIRE ALL LAPS SECURELY TOGETHER.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.
- FLOOR FINISH REQUIREMENTS: SLAB-ON-GRADE SHALL BE FINISHED TO OVERALL FLOOR FLATNESS, OVERALL FLOOR LEVELNESS, LOCAL FLOOR FLATNESS, AND LOCAL FLOOR LEVELNESS REQUIREMENTS AS DEFINED BY THE OWNER. COORDINATE REQUIREMENTS AS REQUIRED WITH G.C. PRIOR TO SLAB-ON-GRADE PLACEMENT. FLOOR FINISH REQUIREMENTS TO BE DETERMINED IN ACCORDANCE WITH ASTM E 1155.
- CONTRACTION JOINTS (ALSO KNOWN AS CONTROL JOINTS) ARE INTENDED TO CREATE WEAKENED PLANES IN THE CONCRETE AND REGULATE THE LOCATION OF CRACKS.
- THE MAXIMUM JOINT SPACING SHOULD BE 24 TO 36 TIMES THE THICKNESS OF THE SLAB NOT EXCEEDING 15 FEET.

- ALL PANELS SHOULD BE SQUARE OR NEARLY SQUARE. THE LENGTH SHOULD NOT EXCEED 1.5 TIMES THE WIDTH. AVOID L-SHAPED PANELS.

**FOUNDATIONS:**

- FOUNDATIONS FOR THIS PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH REQUIREMENTS SET FORTH IN A GEOTECHNICAL REPORT PREPARED BY GTA ENGINEERING SERVICES, DATED 2006-09-27, GTA PROJECT NO.: 50685GE. CONTINUOUS AND INDIVIDUAL FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE SOIL BEARING VALUE OF 5000 PSF. THE CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT FOR ALL REQUIREMENTS AND RECOMMENDATIONS PERTINENT TO THIS PROJECT.
- A FROST DEPTH OF 2'-8" BE UTILIZED FOR ALL EXTERIOR FOOTINGS IN NON-HEATED SPACE.
- ANCHOR RODS SHALL CONFORM TO ASTM F1554 GR. 36 (U.N.O.) AND SHALL BE LOCATED BY MEANS OF A TEMPLATE. PROVIDE A NUT ABOVE AND BELOW TEMPLATE TO ASSURE PROPER VERTICAL ALIGNMENT.
- ALL FOUNDATIONS SHALL BE SQUARE AND LEVEL.
- GROUT SHALL BE DRY AND STIFF TO PREVENT SHRINKAGE, WITH A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI. GROUT BELOW COLUMN BASE PLATES AND PRECAST PANELS AS REQUIRED. THOROUGHLY COMPACT GROUT BENEATH BASE PLATES.

**CONCRETE AND REINFORCING STEEL:**

- CONCRETE MIX DESIGNS SHALL MEET THE FOLLOWING REQUIREMENTS:

Location	Minimum Compressive Strength (psi)	Max. Aggregate Size	Max. Water/Cement Ratio	Slump (in.)	Air Entrainment (%)
Interior Slabs	5000	3/4"	0.50	4 ± 1	0
Exterior Slabs	5000	3/4"	0.50	4 ± 1	6 ± 1
Interior Foundations	5000	1"	0.50	4 ± 1	0
Perimeter Foundations	5000	1"	0.50	4 ± 1	6 ± 1
Exterior Walls & Pedestals	5000	3/4"	0.50	4 ± 1	6 ± 1
Lightweight Topping Slab	5000	1/2"	0.45	4 ± 1	0
Columns	5000	3/4"	0.50	4 ± 1	6 ± 1
Beams	5000	3/4"	0.50	4 ± 1	6 ± 1
Elevated Slabs	5000	3/4"	0.50	4 ± 1	6 ± 1

- FLY ASH SHALL NOT BE USED UNLESS APPROVED IN WRITING BY THE ENGINEER. FLY ASH, IF APPROVED, SHALL CONFORM TO ASTM C618 AND ACI 232.2R-96. FLY ASH SHALL BE LIMITED TO TYPES C & F AND SHALL NOT EXCEED 25% OF THE TOTAL CEMENT WT.
- THE USE OF ADMIXTURES TO INCREASE THE SLUMP SHALL NOT BE USED UNLESS APPROVED IN WRITING BY THE ENGINEER.
- ALL CONCRETE IS REINFORCED UNLESS SPECIFICALLY CALLED OUT AS UNREINFORCED. REINFORCE ALL CONCRETE NOT OTHERWISE SHOWN WITH SAME STEEL AS IN SIMILAR SECTIONS OR AREAS.
- CONSTRUCTION JOINTS IN GRADE BEAMS SHALL BE AT MIDSPAN UNLESS NOTED OTHERWISE. REINFORCING STEEL SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS UNLESS NOTED OTHERWISE.
- NO ALUMINUM ITEMS SHALL BE EMBEDDED IN ANY CONCRETE OR PLACED IN CONTACT WITH CONCRETE.
- REINFORCING BARS #4 AND LARGER (EXCEPT TIES AND STIRRUPS) SHALL MEET ASTM A615 WITH SUPPLEMENTARY REQUIREMENTS (S1), GRADE 60. SMALLER BARS SHALL BE GRADE 40. FREE FROM OIL, SCALE, AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF THE ACI STANDARDS AND SPECIFICATIONS.
- CONCRETE COVERAGE OF REINFORCEMENT SHALL HAVE THE FOLLOWING CLEAR DISTANCES UNLESS NOTED OTHERWISE ON THE DRAWINGS:

ELEMENTS	BOT	TOP
FOOTING AND PILE CAPS	3"	2"
GRADE BEAMS WITH SIDES FORMED	3"	2"
BEAMS ABOVE GRADE	1 1/2"	1 1/2"
COLUMNS	-	1 1/2"
SLABS ON GRADE	2"	2"
SLABS ABOVE GRADE	3/4"	3/4"
SLABS EXP. TO WEATHER(S)	1 1/2"	1 1/2"
SLABS EXP. TO WEATHER(LARGER)	2"	2"
WALLS RETAINING FILL	1 1/2"	1 1/2"
WALLS ABOVE GRADE	1 1/4"	1 1/4"
CONCRETE CAST AGAINST EARTH	3"	3"

- EMBEDDED AND ALL REINFORCING BARS MARKED CONTINUOUS SHALL BE EMBEDDED TO DEVELOP THE FULL TENSILE CAPACITY OF THE BAR. LAPS SHALL BE CLASS B TENSION LAPS UNLESS SPECIFIED OTHERWISE ON THE DRAWINGS. UNLESS SHOWN OTHERWISE, SPLICE TOP BARS NEAR MIDSPAN AND SPLICE BOTTOM BARS OVER SUPPORTS.

- SUPPLY CORNER BARS 4"-0" LONG (MIN. 2'-0" IN EACH DIRECTION) IN OUTSIDE FACE OF WALL AT CORNERS OF ALL WALLS AND GRADE BEAMS, MATCHING SIZE AND SPACING OF HORIZONTAL BARS. WHERE THERE ARE NO VERTICAL BARS IN OUTSIDE FACE OF WALL, SUPPLY THREE (3) - #4 VERTICAL SUPPORT BARS FOR CORNER BARS.

- ALL BARS ARE TO BE SUPPORTED IN FORMS AND SPACED WITH WIRE BAR SUPPORTS PER ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES" (LATEST EDITION). BARS SHALL BE SECURELY WIRED PER THE LATEST EDITION OF CRENSHAW'S RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS. ACCESSORIES FOR EXPOSED CONCRETE SHALL BE PLASTIC OR SHALL HAVE PLASTIC-TIPPED FEET.

12. CONCRETE PLACED DURING COLD WEATHER SHALL CONFORM TO THE REQUIREMENTS OF THE MOST RECENT VERSION OF ACI 308R. COLD WEATHER IS DEFINED AS A PERIOD WHEN, FOR MORE THAN 3 SUCCESSIVE DAYS, THE MEAN DAILY TEMPERATURE DROPS BELOW 40°F.

13. CONCRETE PLACED DURING HOT WEATHER SHALL CONFORM TO THE REQUIREMENTS OF THE MOST RECENT VERSION OF ACI 305R. HOT WEATHER IS DEFINED AS THAT COMBINATION OF AIR TEMPERATURE, CONCRETE TEMPERATURE, RELATIVE HUMIDITY AND WIND SPEED THAT WILL CAUSE A RATE OF EVAPORATION OF 0.2 LB/SQ.FT./HR. OR MORE AS DEFINED BY FIGURE 2.1.5 OF ACI 305R.

14. DO NOT ADD WATER TO CONCRETE DURING DELIVERY, AT PROJECT SITE, OR DURING PLACEMENT, UNLESS APPROVED BY THE ENGINEER.

15. PROVIDE 3/4" CHAMFER ON ALL EXPOSED CORNERS UNLESS NOTED OTHERWISE ON ARCHITECTURAL OR STRUCTURAL CONSTRUCTION DOCUMENTS.

16. ALL COLD JOINTS SHALL BE ROUGHENED AND CLEANED UNLESS NOTED OTHERWISE.

17. VERTICAL CONTROL JOINTS IN WALLS SHALL BE PLACED AT 30'-0" MAXIMUM SPACING UNLESS NOTED OTHERWISE. LOCATE JOINTS BESIDE PIERS MONOLITHIC WITH WALLS, NEAR CORNERS, AND IN CONCEALED LOCATIONS WHERE POSSIBLE. CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CONTROL JOINTS AT CONTRACTORS DISCRETION. COORDINATE LOCATION OF CONTROL JOINTS WITH ARCHITECT.

18. REFER TO THE GEOTECHNICAL REPORT FOR BEHIND WALL DRAINAGE RECOMMENDATIONS. G.C. TO COORDINATE WITH CIVIL DRAWINGS AS REQUIRED. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION WATERPROOFING AND INSULATION REQUIREMENTS.

19. UTILITIES SHALL NOT PENETRATE BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, U.N.O.

**POST-INSTALLED ANCHORS:**

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED IN THE CONSTRUCTION DOCUMENTS OR APPROVED BY THE ENGINEER.
- THE CONTRACTOR SHALL OBTAIN WRITTEN APPROVAL FROM THE ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS FOR MISPLACED-PLACED ANCHORS.

- CARE SHALL BE TAKEN WITH PLACING POST-INSTALLED ANCHORS TO AVOID DAMAGING EXISTING REINFORCEMENT.
- THE HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

- POST-INSTALLED ANCHORS SHALL MEET ACI 318 APPENDIX D CRITERIA. THE FOLLOWING ARE ACCEPTABLE POST-INSTALLED ANCHORS:

- ADHESIVE ANCHORING SYSTEMS REFERRED TO IN THESE DRAWINGS SHALL BE ONE OF THE FOLLOWING:
  - HILTI HIT HY 200
  - POWERS AC108+ GOLD
  - SIMPSON STRONG-TIE SET-3G
  - OR APPROVED EQUIVALENT

- ALL SCREW ANCHORS REFERRED TO IN THESE DRAWINGS SHALL BE ONE OF THE FOLLOWING:
  - HILTI KH-EZ
  - POWERS WEDGE BOLT+
  - SIMPSON STRONG-TIE TITAN HD
  - OR APPROVED EQUIVALENT

**MASONRY:**

- MORTAR SHALL BE TYPE S FOR ALL MASONRY WORK AND MUST ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT THE 28-DAY TEST. MASONRY UNITS SHALL HAVE A MINIMUM STRENGTH OF FM = 1900 PSI.
- MASONRY GROUT SHALL BE A COARSE-TYPE GROUT AND MUST ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT THE 28-DAY TEST. SLUMP SHALL RANGE FROM 8" MINIMUM TO 10" MAXIMUM. GROUT MATERIALS AND PROPORTIONS SHALL CONFORM TO ASTM C476.

- ALL MASONRY SHALL BE REINFORCED WITH HORIZONTAL 9 GAGE TRUSS TYPE REINFORCEMENT AT 16" O.C. VERTICAL OR AS SHOWN ON THE DRAWINGS.

- VERTICAL REINFORCING SHALL BE INSTALLED AS NOTED ON THE DRAWINGS. REINFORCING BARS SHALL BE LAPPED AS SPECIFIED ON THE DESIGN DRAWINGS. IF NO LAP LENGTH IS SHOWN, CONTACT THE ENGINEER.

- VERTICAL CONTROL JOINTS IN MASONRY SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL AT LOCATIONS SHOWN ON THE ARCHITECTURAL DRAWINGS. JOINTS SHALL BE SPACED AT A MAXIMUM OF 25'-0" APART AND COORDINATED WITH THE ARCHITECT. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT MASONRY CONTROL JOINTS. REFER TO TYPICAL DETAILS FOR ADDITIONAL INFORMATION.

- LINTELS OVER OPENINGS SHALL BE INSTALLED AS INDICATED ON THE DRAWINGS. IF NO LINTELS ARE INDICATED, NOTIFY THE ENGINEER.

- PROVIDE AT LEAST (1) VERTICAL REBAR AT EACH END OF EACH WALL, SIDE OF CONTROL JOINTS, JAMBS, CORNER, AND INTERSECTION OF ALL REINFORCED MASONRY WALLS. SIZE OF REBAR TO MATCH THE SIZE OF TYPICAL VERTICAL REINFORCING SHOWN.

- PROVIDE (1) CORNER BAR AT EACH HORIZONTAL BOND BEAM. SIZE OF REBAR TO MATCH TYPICAL BOND BEAM REINFORCING SHOWN.

- SUBMIT SHOP DRAWINGS INCLUDING PLAN AND ELEVATION VIEWS OF REINFORCED MASONRY WALLS INCLUDING BOND BEAMS, CONTROL JOINTS, EXPANSION JOINTS, AND LINTELS.

- ALL STEEL BEAMS BEARING ON MASONRY SHALL HAVE (3) CORES MINIMUM GROUTED FULL DIRECTLY BELOW THE BEARING LOCATIONS UNLESS NOTED OTHERWISE.

- ALL BOND BEAM REINFORCING SHALL CONTINUE THROUGH CONTROL JOINTS.

- ALL CELLS CONTAINING REINFORCEMENT, BOLTS, OR OTHER METAL ANCHORS SHALL BE GROUTED SOLID. ANY CELLS BELOW GRADE SHALL BE GROUTED SOLID WHETHER REINFORCED OR NOT.

- FOR MASONRY BELOW GRADE: TYPE "M" WITH A MINIMUM AVERAGE STRENGTH OF 2500 PSI

- FOR MASONRY ABOVE GRADE: TYPE "S" WITH A MINIMUM AVERAGE STRENGTH OF 1800 PSI

**STRUCTURAL STEEL:**

- ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING (U.N.O.):

- BOLTS SHALL BE AS FOLLOWS (U.N.O.):

- CONNECTION BOLTS: ASTM A325
- ANCHOR RODS: ASTM F1554, GRADE 36
- SHEAR STUDS: ASTM A108, GRADE 1015 THROUGH 1020

- WELDING SHALL CONFORM TO THE LATEST PUBLICATION OF APPLICABLE CODES SET FORTH BY THE AMERICAN WELDING SOCIETY. WELDING ELECTRODES SHALL BE E70XX.

- ALL EXTERIOR STEEL EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED AND/OR PAINTED PER ARCHITECT UNLESS NOTED OTHER WISE.

- ALL OPENINGS IN THE ROOF SHALL BE FRAMED WITH A 4 X 4 X 1/4 ANGLE MINIMUM, UNLESS NOTED OTHERWISE. MECHANICAL UNITS SHALL BE SUPPORTED WITH STRUCTURAL STEEL FRAMES AS REQUIRED. IF FRAMING IS NOT SHOWN FOR MECHANICAL UNITS, NOTIFY THE ENGINEER.

- ALL STEEL STAIRS SHALL BE DESIGNED BY THE STEEL STAIR MANUFACTURER IN COMPLIANCE WITH THE GOVERNING BUILDING CODE TO MEET 100 PSF DESIGN LIVE LOAD.

- SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON THE STRUCTURAL DRAWINGS.

- REFER TO ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS (WHERE APPLICABLE) FOR PAINTING AND FIREPROOFING.

- ALL STEEL SHALL RECEIVE SHOP AND FIELD TOUCH-UP COATS OF PAINT IN ACCORDANCE WITH SSPC SPECIFICATIONS.

**ROUGH CARPENTRY:**

- ALL ROOF, FLOOR AND WALL SHEATHING SHALL BE APA RATED, WITH EXTERIOR GLUE. ROOF SHEATHING SHALL HAVE THE PANEL IDENTIFICATION INDEX OF 24/16. FLOOR SHEATHING SHALL HAVE AN IDENTIFICATION INDEX OF 48/24.

- PLYWOOD SHEATHING SHALL BE ATTACHED TO FRAMING MEMBERS AS DESCRIBED BELOW:

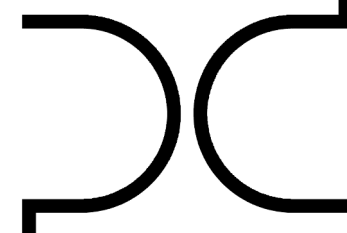
Locations	Plywood Thickness (in.)	Tongue & Groove	Nail Size	Nail Type	Min. Penetration into Support (in.)	Nail Spacing @ Panel Edges	Nail Spacing @ Interior Support	Nail Spacing @ Diaphragm Boundary	Blocked
Roof	23/32"	Yes	10d	Common	1 1/2"	6"	12"	6"	No
Ceiling	7/16"	Yes	10d	Common	1 1/2"	6"	12"	6"	No
Walls	7/16"	No	8d	Common	1 3/8"	6"	12"	6"	Yes
Floor	23/32"	Yes	10d	Common	1 1/2"	6"	12"	6"	No

- ALL DIMENSION LUMBER USED IN LOAD-BEARING WALLS, FLOOR JOISTS, EXTERIOR LINTELS, INTERIOR LINTELS, ALL BEARING AND JAMB STUDS, COLUMNS AND BEAMS, SHALL HAVE THE FOLLOWING MINIMUM DESIGN VALUES (U.N.O.):

- F: 900 PSI
- FV: 180 PSI
- FC (PARA): 1,350 PSI
- E: 1,600,000 PSI

THESE VALUES ARE BASED ON ALLOWABLE STRESSES PROVIDED IN THE GOVERNING NDS AND DO NOT INCLUDE ADJUSTMENT FACTORS.

THE FOLLOWING SPECIES AND COMMERCIAL GRADES CONFORM TO THE ABOVE MINIMUM DESIGN VALUES:



PURE DESIGN



**ARCHITECT**  
PURE DESIGN  
11264 N. MADISON AVE.  
KANSAS CITY, MO 64155  
PHONE: 316.617.2052

**STRUCTURAL ENGINEER**  
BSE STRUCTURAL ENGINEERS  
11320 W. 79TH ST.  
LENEXA, KS 66214  
PHONE: 913.492.7400

**MEP ENGINEERS**  
PKMR  
13300 W. 98TH ST.  
LENEXA, KS 66215  
PHONE: 913.492.2400

**CIVIL ENGINEERS**  
Russell Planning & Engineering Inc.  
934 Main Avenue, Unit C  
Durango CO, 81301  
Phone 970-385-4546

**INTERIOR DESIGNER**  
EDGEID  
4838 South Highland Drive  
Holladay, UT 84117  
p 801-642-4100  
www.edgeid.com

**KITCHEN CONSULTANT**  
Culmeex  
311 4th Ave South  
P.O. Box 365 | Sartell, MN 56377  
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DUAL HOTEL DURANGO  
- AC & HIS



JOB NO: 18-007

DATE: 2019.03.07

ISSUE RECORD:

REVISIONS:

NEW 2019.06.14

STRUCTURAL NOTES (CONT.)

S-102

**SPECIAL INSPECTOR:**

- THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION IN ACCORDANCE WITH THE BUILDING CODE.
  - REINFORCED MASONRY CONSTRUCTION - LEVEL 1 INSPECTION
  - CONCRETE & MASONRY GROUT DESIGN MIX
  - PLACING OF CONCRETE & REINFORCING STEEL
  - BOLTS & ANCHORS EMBEDDED IN CONCRETE & MASONRY
  - CONCRETE FORMWORK
  - STRUCTURAL STEEL FABRICATION
  - STRUCTURAL STEEL BOLTING & WELDING
  - INSPECTION OF ROOF & DECK ATTACHMENT
  - POST INSTALLED ANCHORS IN MASONRY & CONCRETE
  - IN-SITU SOILS, EXCAVATIONS, FILLING & COMPACTION
- THE CONTRACTOR SHALL REQUEST SPECIAL INSPECTION OF THE ITEMS LISTED ABOVE PRIOR TO THOSE ITEMS BECOMING INACCESSIBLE & UNOBSERVABLE DUE TO PROGRESSION OF THE WORK.
- THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
- THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.
- THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER AND ARCHITECT OF RECORD, AND OTHER DESIGNATED PERSONS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN IF UNCORRECTED, TO THE PROPER DESIGN AUTHORITY AND TO THE BUILDING OFFICIAL.

- THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE GOVERNING BUILDING CODES.

**EARTHWORK:**

- THE INSPECTOR MUST VERIFY THAT THE PREPARATION OF THE NATURAL GROUND AND THE PLACEMENT OF ENGINEERED FILL IS PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS AS STATED IN THE GEOTECHNICAL REPORT.
- THE INSPECTOR MUST MONITOR THE PLACEMENT OF ALL FILL TO DETERMINE WHETHER THE TYPE OF MATERIAL, MOISTURE CONTENT, AND DEGREE OF COMPACTION ARE WITHIN THE RECOMMENDED LIMITS CONTAINED IN THE GEOTECHNICAL REPORT. PROCEED WITH SUBSEQUENT EARTHWORK ONLY AFTER TEST RESULTS FOR PREVIOUSLY COMPLETED WORK COMPLY WITH RECOMMENDED LIMITS CONTAINED IN THE GEOTECHNICAL REPORT.
- ALL SUBGRADE SUPPORTING FOOTINGS AND SLABS MUST BE INSPECTED IMMEDIATELY PRIOR TO THE PLACEMENT OF REINFORCED CONCRETE.
- PAVED AND BUILDING SLAB AREAS SHALL BE TESTED AT SUBGRADE AND AT EACH COMPACTED FILL AND BACKFILL LAYER, AT LEAST ONCE FOR EVERY 2000 SQ. FT. OR LESS OF PAVED OR BUILDING SLAB AREAS, BUT IN NO CASE FEWER THAN 3 TESTS.
- FOUNDATION WALL BACKFILL SHALL BE TESTED AT EACH COMPACTED INITIAL AND FINAL BACKFILL LAYER, AT LEAST ONCE FOR EACH 100 FT. OR LESS OF WALL LENGTH, BUT NO FEWER THAN 2 TESTS.
- TRENCH BACKFILL SHALL BE TESTED AT EACH COMPACTED INITIAL AND FINAL BACKFILL LAYER, AT LEAST ONCE FOR EACH 150 FT. OR LESS OF TRENCH LENGTH, BUT NO FEWER THAN 2 TESTS.
- TEST COMPACTION OF SOILS-IN-PLACE IN ACCORDANCE WITH ASTM D 1556, ASTM D 2167, ASTM D 2922, AND ASTM D 2937, AS APPLICABLE.
- TEST REPORTING: TEST RESULTS MUST BE REPORTED TO BSE AND THE GENERAL CONTRACTOR IN WRITING WITHIN 24 HOURS AFTER TESTING, VIA FAX. REPORTS MUST CONTAIN THE PROJECT NAME, THE DATE OF THE TEST AND THE LOCATION OF THE TEST.
- DO NOT BACKFILL CANTILEVERED RETAINING WALLS UNTIL CONCRETE REACHES 75% OF THE SPECIFIED 28 DAY STRENGTH.
- DO NOT BACKFILL UNTIL COMPLETION OF INSTALLATION AND INSPECTION OF ANY WATERPROOFING.

**CONCRETE:**

- STRENGTH TEST CYLINDERS SHALL BE PREPARED FOR EACH DAY'S POUR OF EACH CONCRETE MIX AND AT A MINIMUM FREQUENCY OF EVERY 50 CU. YD. ON ALL CONCRETE PLACED. CONFORM TO ASTM C39.
- FOUR (4) TEST CYLINDERS ARE TO BE MADE AND CURED ON SITE FOR THE FIRST 24 HOURS. TEST ONE OF THE SPECIMENS AT 7 DAYS AND TWO AT 28 DAYS. HOLD THE FOURTH SPECIMEN IN RESERVE FOR LATER TESTING IF NEEDED.
- SLUMP, AIR CONTENT AND TEMPERATURE TESTS SHALL BE CONDUCTED AT A MINIMUM WHEN STRENGTH SPECIMENS ARE MADE AND AT ANY OTHER TIMES AS SPECIFIED BY THE ENGINEER.
- PERFORM SLUMP TESTS ON A REPRESENTATIVE CONCRETE SAMPLE AT THE POINT OF DISCHARGE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY SEEMS TO HAVE CHANGED. THE MAXIMUM ALLOWABLE FIELD SLUMP IS 5 INCHES. CONFORM TO ASTM C143.
- PERFORM AIR CONTENT TESTS ON ALL CONCRETE SPECIFIED TO BE AIR-ENTRAINED. CONFORM TO ASTM C231.
- PERFORM A TEMPERATURE TEST EVERY HOUR WHEN AIR TEMPERATURE IS 40°F AND BELOW, OR WHEN AIR TEMPERATURE IS 80°F AND ABOVE. CONFORM TO ASTM C 1064.
- PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOB SITE, THE INSPECTOR SHALL VERIFY THAT THE REINFORCING STEEL IS IN CONFORMANCE WITH THE CITY-APPROVED PLANS, SPECIFICATIONS AND SHOP DRAWINGS. THE INSPECTOR SHALL CONFIRM THAT THE REINFORCING STEEL IS OF THE CORRECT SIZE AND GRADE AND ENSURE THAT THE PROPER SPACING, CLEARANCES, SPICE LENGTHS AND EMBEDDED TEMS HAVE BEEN PROVIDED. ALL REINFORCING STEEL SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF CONCRETE AND BE SECURED AGAINST DISPLACEMENT.
- THE INSPECTOR SHALL VERIFY THAT THE BOLT SIZE, LOCATION AND EMBEDMENT LENGTH OF ALL ANCHOR BOLTS ARE IN CONFORMANCE WITH THE CITY-APPROVED PLANS, SPECIFICATIONS AND SHOP DRAWINGS.
- ANCHOR RODS 3/4"Ø OR SMALLER MAY BE FLOATED IN PLACE FOLLOWING CONCRETE PLACEMENT, PROVIDED THAT ANCHOR BOLTS ARE WORKED EASILY BY HAND INTO THE FRESH CONCRETE TO ALLOW FOR FULL CONTACT WITH THE SHANK OF THE BOLT. BOLTS SHALL BE PLACED BY MEANS OF A TEMPLATE AND SHALL BE WORKED INTO CONCRETE IN VERTICAL ALIGNMENT.
- CALCIUM CHLORIDES SHALL NOT BE UTILIZED IN THE WORK.
- ADEQUATE VERTICAL AND HORIZONTAL SHORING SHALL BE PROVIDED TO SAFELY SUPPORT ALL CONSTRUCTION LOADS.

**STRUCTURAL STEEL:**

- BOLTS - BOLTS THAT ARE NOT IDENTIFIED AS BEING SUP-CRITICAL NOR IN DIRECT TENSION NEED NOT BE INSPECTED OTHER THAN TO VERIFY THAT THE PILES OF CONNECTED ELEMENTS ARE BROUGHT INTO SNUG-TIGHT CONDITION IN PROPERLY-ALIGNED HOLES.
- FIELD WELDING: INSPECTION IS REQUIRED FOR SINGLE-PASS FILLET WELDS, MULTI-PASS FILLET WELDS, COMPLETE- AND PARTIAL-PENETRATION GROOVE WELDS, FLOOR AND ROOF DECK WELDING, AND STAIRS AND RAILING SYSTEMS. PRIOR TO THE START OF THE WORK, MATERIALS, QUALIFICATIONS OF WELDING PROCEDURES AND WELDER QUALIFICATIONS SHALL BE VERIFIED. PROVIDE CONTINUOUS OR PERIODIC INSPECTION OF THE STRUCTURAL WELDING AS INDICATED IN TABLE 1704.3 OF THE REFERENCED IBC. INSPECTIONS MAY OCCUR PERIODICALLY, AS DEFINED BELOW. A VISUAL INSPECTION TO ENSURE PROPER TYPE, SIZE, LENGTH AND QUALITY OF ALL FIELD WELDS IS REQUIRED PRIOR TO WORK BEING CONCEALED BY OTHER MATERIALS.
- PERIODIC INSPECTION: "PERIODIC" IS DEFINED AS GENERALLY ONCE A WEEK AT A MINIMUM, AND MORE OFTEN AS NEEDED TO OBSERVE WORK REQUIRING INSPECTIONS, AS OUTLINED ABOVE, PRIOR TO BEING COVERED BY SUBSEQUENT CONSTRUCTION.
- SHEAR CONNECTOR STUD WELDS WILL BE INSPECTED AND TESTED ACCORDING TO AWS D1.1 FOR STUD WELDING. SHEAR CONNECTOR STUD WELDS SHALL BE VISUALLY INSPECTED. BEND TESTS SHALL BE PERFORMED IF VISUAL INSPECTIONS REVEAL LESS THAN A 360-DEGREE FLASH OR WELDING REPAIRS TO ANY SHEAR CONNECTOR STUD.
- STRUCTURAL STEEL BAR JOISTS AND METAL BUILDINGS FABRICATED ON THE PREMISES OF A FACILITY/PLANT NOT CERTIFIED BY A NATIONALLY RECOGNIZED ORGANIZATION, SHALL HAVE IN-PLANT SPECIAL INSPECTIONS. ASIC, ICBO, CWS AND SII ARE CERTIFIED FABRICATORS.
- TEST REPORTING: TEST RESULTS MUST BE REPORTED TO BSE AND THE GENERAL CONTRACTOR IN WRITING WITHIN 24 HOURS OF TESTING, VIA FAX OR EMAIL. REPORTS MUST CONTAIN THE PROJECT NAME, THE DATE OF THE TEST AND THE LOCATION OF THE TEST.

**MASONRY:**

- MORTAR PROPERTIES, GROUT, BRICK, CONCRETE MASONRY UNIT AND PRISM TESTS AND EVALUATIONS ARE TO BE PERFORMED DURING CONSTRUCTION FOR EACH 5,000 SQ. FT. OF WALL AREA OR PORTION THEREOF.
- MORTAR PROPERTIES ARE TO BE TESTED PER ASTM C 780.
- GROUT WILL BE SAMPLED AND TESTED FOR COMPRESSIVE STRENGTH PER ASTM C 1019.
- BRICK TESTS FOR EACH TYPE AND GRADE OF BRICK INDICATED ARE TO BE PERFORMED ACCORDING TO ASTM C 67.
- CONCRETE MASONRY UNIT TESTS FOR EACH TYPE OF CONCRETE MASONRY UNIT INDICATED ARE TO BE PERFORMED PER ASTM C 140.
- MASONRY PRISMS ARE TO BE TESTED PER ASTM C 1314. PREPARE ONE (1) SET OF PRISMS FOR TESTING AT 7 DAYS AND ONE (1) SET FOR TESTING AT 28 DAYS.
- SPECIAL INSPECTION OF MASONRY CONSTRUCTION IS REQUIRED DURING PREPARATION AND TAKING OF ANY REQUIRED PRISMS OR TEST SPECIMENS, PLACING OF ALL MASONRY UNITS, PLACEMENT OF REINFORCEMENT AND INSPECTION OF GROUT SPACE IMMEDIATELY PRIOR TO CLOSING CLEANOUTS, AND DURING ALL GROUTING OPERATIONS.
- TEST REPORTING: TEST RESULTS MUST BE REPORTED TO BS AND THE GENERAL CONTRACTOR IN WRITING WITHIN 24 HOURS OF TESTING, VIA FAX. REPORTS MUST CONTAIN THE PROJECT NAME, THE DATE OF THE TEST AND THE LOCATION OF THE TEST.

**STRUCTURAL WOOD:**

- SPECIAL INSPECTIONS OF STRUCTURAL WOOD FRAMING TO BE PERFORMED IN ACCORDANCE WITH SECTION 1705.11.2. OF THE REFERENCED IBC.
- PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLD-DOWNS.
- PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE WIND FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLD-DOWNS.
- TEST REPORTING: TEST RESULTS MUST BE REPORTED TO BSE AND THE GENERAL CONTRACTOR IN WRITING WITHIN 24 HOURS OF TESTING, VIA FAX. REPORTS MUST CONTAIN THE PROJECT NAME, THE DATE OF THE TEST AND THE LOCATION OF THE TEST.



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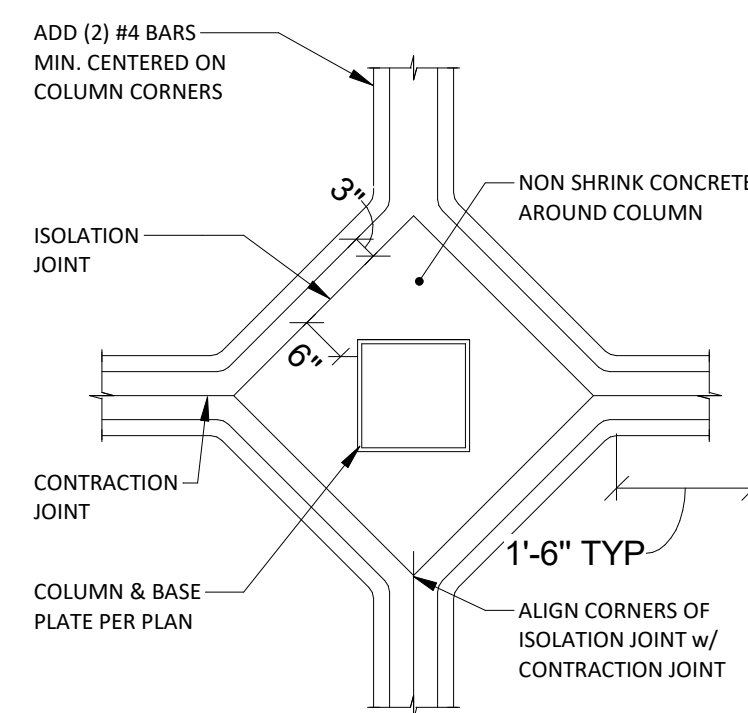
COLUMN FOOTING SCHEDULES					
MK	SIZE	TYPE	REBAR	REMARK	TOP ELEV.
CF15.0	15'-0"x15'-0"x3'-0"		(15) #4 BAR TOP. EACH WAY (18) #8 BAR BOTT. EACH WAY	COLUMN FOOTING	(-)1'-0"
CF15.0A	15'-0"x15'-0"x3'-0"		(15) #4 BAR TOP. EACH WAY (20) #7 BAR BOTT. EACH WAY	COLUMN FOOTING	(-)1'-0"
CF16.0	16'-0"x16'-0"x3'-0"		(16) #4 BAR TOP. EACH WAY (24) #8 BAR BOTT. EACH WAY	COLUMN FOOTING	(-)1'-0"
CF15.0B	15'-0"x15'-6"x2'-0"		(15) #4 BAR TOP. EACH WAY (24) #7 BAR BOTT. EACH WAY	COLUMN FOOTING	(-)1'-0"

NOTE:  
 1. ADJACENT GRADE ASSUMED TO BE (-)0'-6". CONTRACTOR FIELD VERIFY.  
 2. REGARDLESS OF TOP OF FOOTING ELEVATIONS SHOWN IN THE SCHEDULE ABOVE, ALL EXTERIOR WALL OR PAD FOOTINGS SHALL BE LOCATED SUCH THAT A MINIMUM OF 1'-0" OF FILL EXISTS BETWEEN BOTTOM OF FOOTING AND FINAL GRADE ELEVATION.

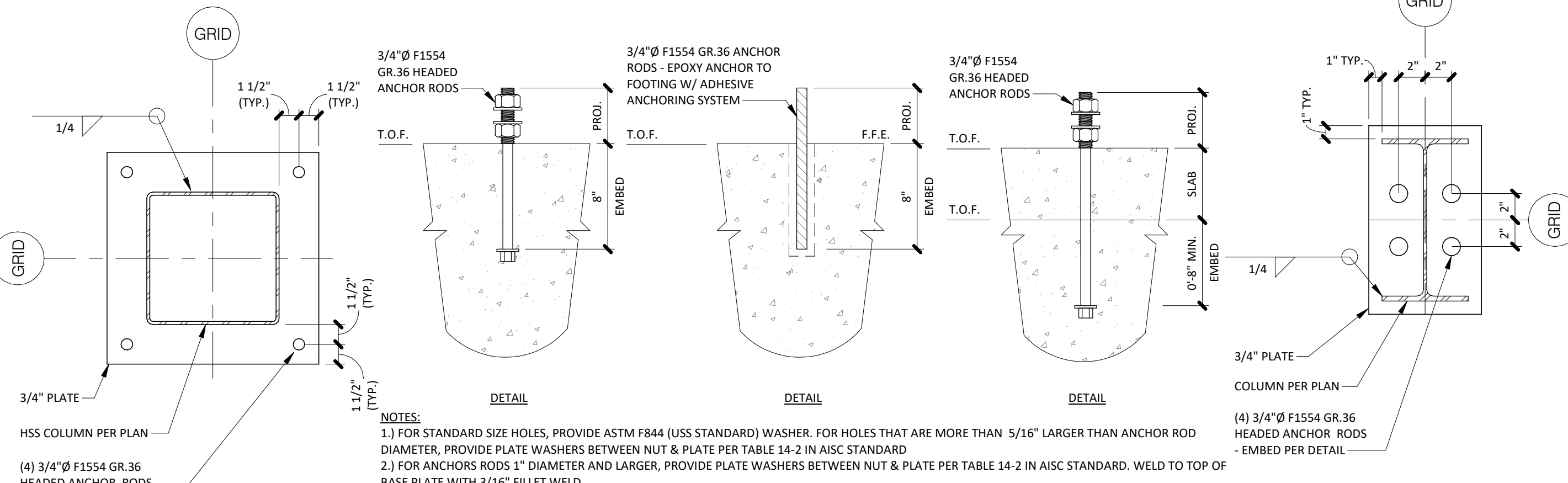
COLUMN / WALL FOOTING SCHEDULE					
MK	SIZE	TYPE	REBAR	REMARK	TOP ELEV.
WF4.0	4'-0" x1'-0"		(5) #5 BOTT. LONG. (5) #5 @ 12" BOTT. TRAN. (5) #5 TOP. LONG. (5) #5 @ 12" TOP. TRAN.	WALL FOOTING	(-)1'-0" EXT.
WF8.5	8'-6" x3'-0"		(13) #7 BOTT. LONG. (7) #7 @ 8" BOTT. TRAN. (13) #7 TOP. LONG. (7) #7 @ 8" TOP. TRAN.	COLUMN FOOTING	(-)1'-0" EXT.
EF1	21'-4"x11'-0" x1'-4"		#5 @ 8" TOP & BOTT. EA. WAY	ELEVATOR FOOTING SUMP PIT	(-)5'-0" (-)6'-6"
EF2	21'-4"x11'-2" x1'-4"		#5 @ 8" TOP & BOTT. EA. WAY	ELEVATOR FOOTING SUMP PIT	(-)5'-0" (-)6'-6"

AMMENITIES COLUMN SCHEDULE				
MARK	SIZE	TYPE	TIE DOWN	REMARK
SC1	HSS 5"x5"x3/8"		SEE BASE PLATE SCHEDULE	STEEL COL.
SC2	HSS 4"x4"x3/8"		SEE BASE PLATE SCHEDULE	STEEL COL.
SC3	HSS 4"x4"x1/2"		SEE BASE PLATE SCHEDULE	STEEL COL.
SC4	HSS 5"x5"x1/2"		SEE BASE PLATE SCHEDULE	STEEL COL.

MASONRY COLUMN SCHEDULE				
MARK	SIZE	TYPE	VERT. REBARS	REMARK
MC1	8"x24"		MASONRY STRENGTH: 1.9KSI. 3-#5VERT.	CMU COLUMN.



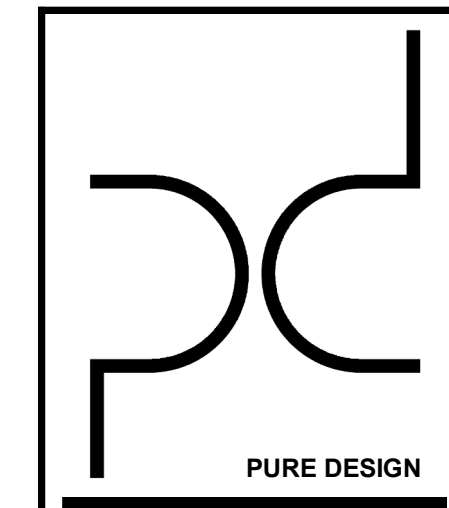
**TYP. SLAB BLOCKOUT AT INTERIOR COLUMNS**  
 SCALE: 1/2"=1'-0"



NOTES:  
 1.) FOR STANDARD SIZE HOLES, PROVIDE ASTM F844 (USS STANDARD) WASHER. FOR HOLES THAT ARE MORE THAN 5/16" LARGER THAN ANCHOR ROD DIAMETER, PROVIDE PLATE WASHERS BETWEEN NUT & PLATE PER TABLE 14-2 IN AISC STANDARD  
 2.) FOR ANCHORS RODS 1" DIAMETER AND LARGER, PROVIDE PLATE WASHERS BETWEEN NUT & PLATE PER TABLE 14-2 IN AISC STANDARD. WELD TO TOP OF BASE PLATE WITH 3/16" FILLET WELD.

COLUMN SCHEDULE					
MARK	SIZE	TYPE	STIRRUPS PREVIEW	REINF.	REMARK
CC1	18"x60" 24"x60" @ SIM.			20 - #9 #4 TIES @ 8"	RC. COL.
CC2	36"x36"			20 - #8 #4 TIES @ 8"	RC. COL.
CC3	24"x24"			12 - #7 #4 TIES @ 8"	RC. COL.

CONCRETE BEAM SCHEDULE													
BEAM NUMBER	SIZE		BOTTOM REINFORCEMENT			TOP REINFORCEMENT			SHEAR STIRRUPS			SIDE FACE REINFORCEMENT PER FACE	REMARK
	B	D	LEFT	MID SPAN	RIGHT	LEFT	MID SPAN	RIGHT	LEFT	MID SPAN	RIGHT		
CB1	8"	16"	2-#5	2-#5	2-#5	2-#5	2-#5	2-#5	2L-#3@8" O.C.	2L-#3@8" O.C.	2L-#3@8" O.C.		
CB2	16"	60"	2-#8 2-#6 2-#6	2-#8 2-#6 2-#6	2-#8 2-#6 2-#6	3-#8	3-#8	3-#8	2L-#3@6" O.C.	2L-#3@6" O.C.	2L-#3@6" O.C.	8-#5	
CB3	24"	36"	5-#8	5-#8 5-#8	5-#8	5-#8	5-#8	5-#8	4L-#3@5" O.C.	4L-#3@10" O.C.	4L-#3@5" O.C.	2-#5	
CB4	12"	42"	3-#6	3-#6 3-#6	3-#6	3-#6	3-#6	3-#6	2L-#3@8" O.C.	2L-#3@10" O.C.	2L-#3@8" O.C.	3-#5	
CB4A	12"	42"	4-#6	4-#6 4-#6	4-#6	4-#6	4-#6	4-#6	2L-#3@6" O.C.	2L-#3@10" O.C.	2L-#3@6" O.C.	3-#5	
CB4B	12"	42"	3-#6	3-#6	3-#6	3-#6	3-#6	3-#6	2L-#3@8" O.C.	2L-#3@8" O.C.	2L-#3@8" O.C.	3-#5	
CB5	16"	62"	2-#8 2-#8	2-#8 2-#8	2-#8 2-#8	3-#8	3-#8	3-#8	2L-#3@6" O.C.	2L-#3@6" O.C.	2L-#3@6" O.C.	8-#5	



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DUAL HOTEL DURANGO  
 - AC & HIS



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SCHEDULES

S-103

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COMBINED FOOTING SCHEDULE

MK	SIZE	DEPTH	TYPE	TOP ELEV.
CCF1	15'-0"X30'-0"	3'-0"		(-1)'-0"
CCF1A	15'-0"X30'-0"	3'-0"		(-1)'-0"
CCF2	27'-6"X16'-0"	2'-0"		(-1)'-0"
CCF2A	27'-0"X16'-0"	2'-0"		(-1)'-0"
CCF2B	27'-6"X16'-0"	2'-0"		(-1)'-0"
CCF3	28'-6"X12'-10"	2'-0"		(-1)'-0"
SF1.0	16'-6"X26'-0"	2'-0"		(-1)'-0"

COMBINED FOOTING SCHEDULE

MK	SIZE	DEPTH	TYPE	TOP ELEV.
SF2.0	32'-2"X16'-9 3/4"	2'-0"		(-1)'-0"
SF3.0	25'-6"X13'-1 1/4"	2'-0"		(-1)'-0"
CCF4	21'-5 1/2"X54'-4" SEE PLAN	2'-0"		(-1)'-0"
CCF5	8'-6"X66'-8"	3'-0"		(-1)'-0"
CCF6	8'-6"X56'-4 3/8"	3'-0"		(-1)'-0"

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**DUAL HOTEL DURANGO - AC & HIS**

AC HOTELS

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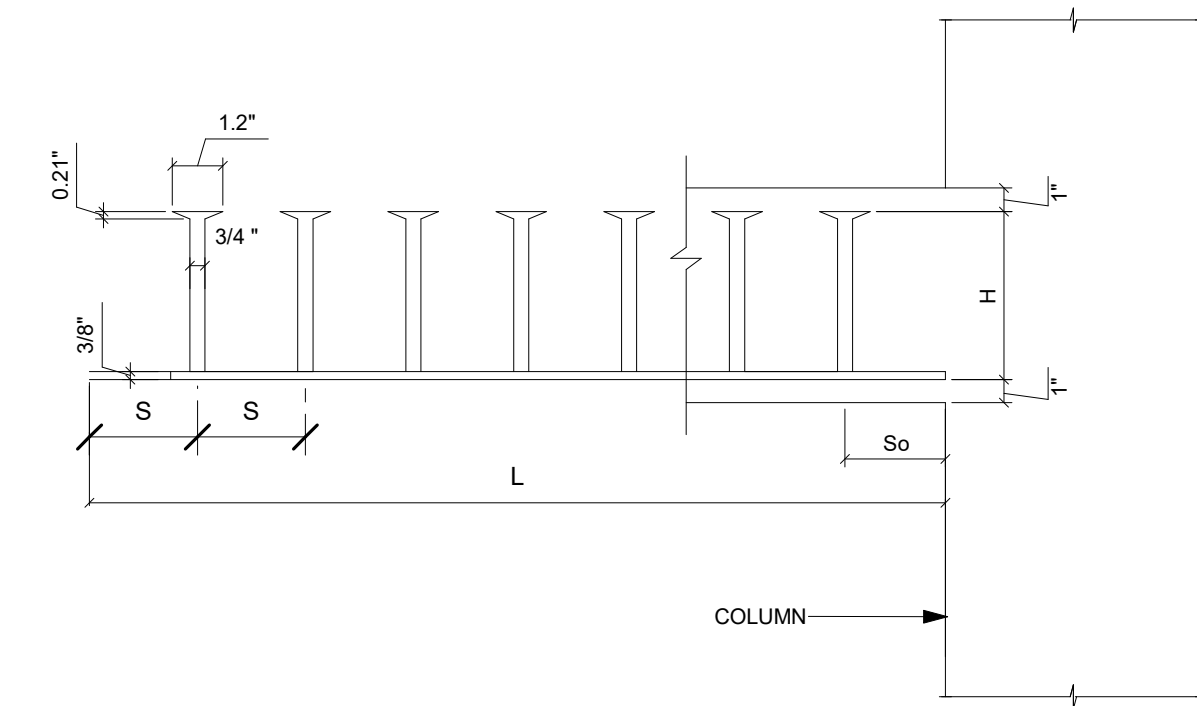
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SCHEDULES (CONT.)

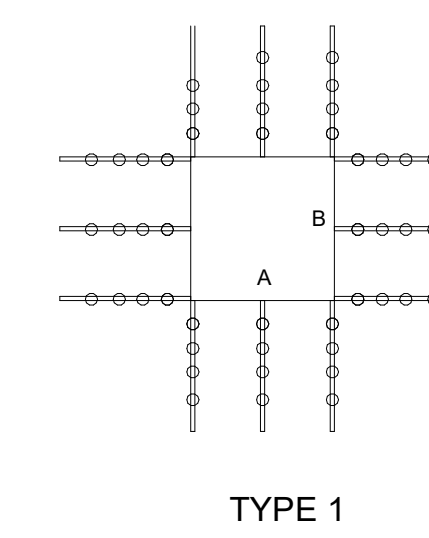
**S-104**

STUD RAIL SCHEDULE

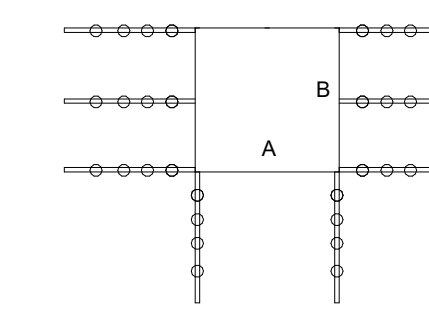
COLUMN LOCATION (SEE GRIDLINES)	LEVEL	THICKNESS OF STUDS PLATE. (INCH)	TYPE	STUD DIAMETER (IN)	NUMBER OF STUD RAIL		#STUDS PER RAIL	SPACING(S <sub>o</sub> ) (INCH)	SPACING(S) (INCH)	L.O.R(L) (INCH)	H (INCH)
					SIDE A	SIDE B					
GL@AR-A11 & AN-H1.5 & HE-A7	2ND PT	3/8	2	3/4	3	3	6	6	6	37	12
AN-A7 & AN-A14 & AN-H4 &	2ND PT	3/8	1	3/4	3	3	6	6	6	37	12
GL@HK-A3	2ND PT	3/8	1	3/4	3	3	8	6	6	49	12
GL@HE-A3	2ND PT	3/8	1	3/4	2	2	6	6	6	37	12
GL@AD-A1 & HE-H12	2ND PT	3/8	2	3/4	3	3	5	6	6	31	12
GL@HE-A11	2ND PT	3/8	1	3/4	3	3	7	6	6	43	12
GL@HE-H4 & H6.5-HE	2ND PT	3/8	1	3/4	4	4	8	6	6	49	12
GL@H1.9-AA & H9-AA	2ND PT	3/8	2	3/4	3	3	7	6	6	43	12
GL@H4-AA	2ND PT	3/8	2	3/4	2	2	5	6	6	31	12
GL@H6-AA	2ND PT	3/8	2	3/4	2	2	6	6	6	37	12
GL@AR-A11 & AA-A8	3RD PT	3/8	2	3/4	2	2	10	6	6	61	12
GL@AR-A14 & HK-H15	3RD PT	3/8	2	3/4	3	3	9	6	6	55	12
GL@H6.5-AN	3RD PT	3/8	1	3/4	3	3	9	6	6	55	12
GL@AR-H1.5 & AR-H6.5 & HF.5-H12 & HF.5-A11 & AA-A6	3RD PT	3/8	2	3/4	3	3	8	6	6	49	12
GL@AR-H4 & GL@HK-H1.5 & AA-H10.7	3RD PT	3/8	2	3/4	3	3	8	6	6	49	12
GL@AR-H13.5	3RD PT	3/8	2	3/4	3	3	7	6	6	43	12
GL@AN-H13.6	3RD PT	3/8	4	3/4		24	13	6	6	79	12
GL@AN-H9.8	3RD PT	3/8	4	3/4		24	14	6	6	85	12
GL@H4-AN	3RD PT	3/8	4	3/4		20	11	6	6	67	12
GL@HK-A1	3RD PT	3/8	5	3/4		24	15	6	6	91	12
GL@HK-A3	3RD PT	3/8	4	3/4		32	15	6	6	91	12
GL@HK-H4	3RD PT	3/8	1	3/4	4	4	10	6	6	61	12
GL@HK-H6.5 & HK-H13.5	3RD PT	3/8	1	3/4	4	4	11	6	6	67	12
GL@HK-H9.8	3RD PT	3/8	4	3/4		20	14	6	6	85	12
GL@HF.5-H15	3RD PT	3/8	2	3/4	3	3	11	6	6	67	12
GL@HE-H15	3RD PT	3/8	2	3/4	3	3	11	6	6	67	12
GL@AC.5-H15 & AA-H13	3RD PT	3/8	3	3/4	3	3	9	6	6	55	12
GL@HE-H12	3RD PT	3/8	1	3/4	4	4	9	6	6	55	12
GL@AA-H9	3RD PT	3/8	2	3/4	4	5	14	6	6	85	12
GL@AA-H6	3RD PT	3/8	2	3/4	4	4	19	6	6	115	12
GL@HE-H6.5	3RD PT	3/8	4	3/4		28	13	6	6	79	12
GL@HF.5-H6.5 & HE-A7	3RD PT	3/8	1	3/4	4	4	10	6	6	61	12
GL@AA-A11	3RD PT	3/8	5	3/4		18	10	6	6	61	12
GL@HF.5-H4 & HF.5-A3	3RD PT	3/8	1	3/4	4	4	10	6	6	61	12
GL@HE-H4	3RD PT	3/8	4	3/4		26	13	6	6	79	12
GL@AA-H4	3RD PT	3/8	5	3/4	4	4	17	6	6	103	12
GL@AA-H1.9	3RD PT	3/8	5	3/4	4	4	16	6	6	97	12
GL@AA-A14	3RD PT	3/8	5	3/4	4	4	23	6	6	139	12
GL@HE-A11	3RD PT	3/8	4	3/4		32	14	6	6	85	12
GL@HE-A3	3RD PT	3/8	4	3/4		24	11	6	6	67	12
GL@AJ-A1 & AG-A1	3RD PT	3/8	5	3/4		12	7	6	6	43	12



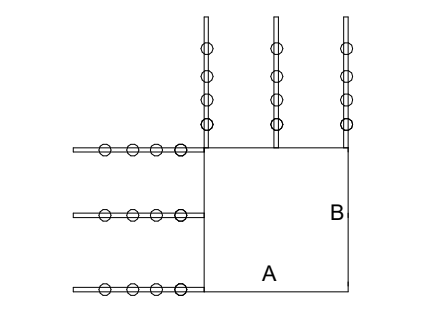
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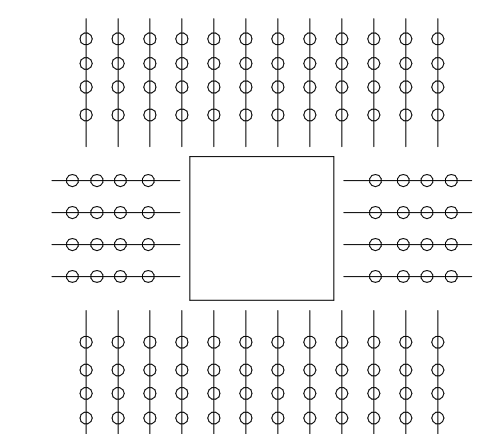
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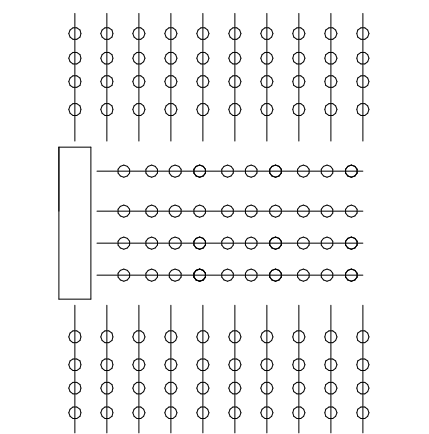
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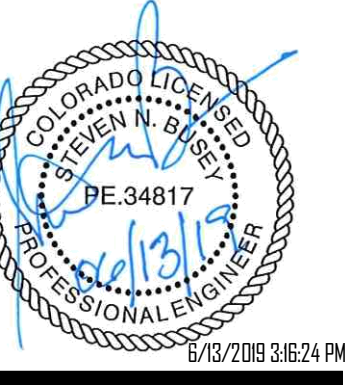
TYPE 3



TYPE 4



TYPE 5



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DUAL HOTEL DURANGO  
- AC & HIS



JOB NO: 18-007  
DATE: 2018.03.07

ISSUE RECORD:

REVISIONS:

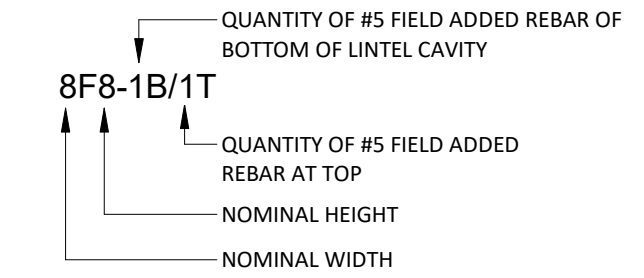
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SCHEDULES (CONT.)

S-105

**BSE STRUCTURAL ENGINEERS**  
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www.BSEStructural.com  
Project Number 18-550

MASONRY WALL SCHEDULE							
MARK	FLOOR LEVEL	SW/BW	MASONRY UNIT PROPERTIES			REINFORCEMENT	
			UNIT STRENGTH (PSI)	WALL THICKNESS (IN)	GROUT SPACING (IN)	VERTICAL REBAR	HORIZONTAL REINFORCEMENT
MW01	1ST FLOOR PLAN	SW	4800	8"	16"	# 5 @ 16" O.C.	9GAGE GALV. @8"
MW02	2ND FLOOR PLAN	SW	2800	8"	24"	# 5 @ 24" O.C.	9GAGE GALV. @8"
MW03	ELEVATOR AND ABOVE 3RD FLOOR PLAN	SW	1900	8"	32"	# 5 @ 32" O.C.	9GAGE GALV. @8"
MW04	3RD FLOOR PLAN	SW	1900	6"	8"	# 4 DOWELS @ 48" SPA.	N/A
MW05	3RD FLOOR PLAN	SW	1900	12"	12"	# 4 DOWELS @ 48" SPA.	N/A



BOND BEAM SCHEDULE			
MARK	SIZE	TYPE	REBAR
BB1	8"x16"		(2) - #5 HORIZONTAL EACH COURSE

AMMENITIES WOOD BEAM SCHEDULE								
MARK	SIZE	TYPE	GRADE	HANGER TO STEEL COLUMN	HANGER TO WOOD BEAM	HANGER TO WOOD COLUMN	HANGER TO CMU WALL	HANGER TO STEEL BEAM
WB1	(2) 2"x8"		SPF NO. 2	-	-	HUC38 WITH 10-16D TO THE WOOD COLUMN & 4-10DX1 1/2" TO THE WOOD BEAM CAPACITY #1490	HU26-2 WITH 12-14X2 3/4 TITEN BOLTS TO CMU AND 6-10D TO WOOD BEAM (#3950)	-
WB2	(2) 2"x10"		SPF NO. 2	-	-	HUC210-2 WITH 18-16D TO WOOD COLUMN AND 10-10D TO WOOD BEAM (#2680)	HU26-2 WITH 12-14X2 3/4 TITEN BOLTS TO CMU AND 6-10D TO WOOD BEAM (#3950)	-
WB3	(3) 2"x10"		SPF NO. 2	-	-	HU210-3 WITH 18-16D TO WOOD COLUMN AND 10-10D TO WOOD BEAM (#3250)	LGUM210-3-SDS WITH 8-3/8"x4" TO THE CMU WALL & 8-1/4"x2 1/2" TO THE WOOD BEAM. CAPACITY #9715	-
WB4	(4) 2"x12"		SPF NO. 2	-	ECC64 WITH 2 MACHINE BOLTS TO THE WOOD BEAM & 2 MACHINE BOLTS TO THE WOOD POST. CAPACITY #12030	HU210-4 WITH 18-16D TO WOOD COLUMN AND 8-16D TO WOOD BEAM (#3250)	LGUM210-4 WITH 8-3/8"x4" TITEN HD BOLTS TO CMU AND 8-1/4"x2 1/2" SDS TO WOOD BEAM (#9860)	HHRB12 WITH 4-3/16 WELD TO STEEL BEAM AND 6-NS4A TO WOOD BEAM (#6235)

AMMENITIES LVL BEAM SCHEDULE							
MARK	SIZE	TYPE	GRADE	HANGER TO STEEL COLUMN	HANGER TO WOOD BEAM	HANGER TO WOOD COLUMN	HANGER TO CMU WALL
WB5	(3) 1 3/4"x11 1/4"		1.9E	-	SIMPSON HGU55.50/14 WITH 22-16d TO BEAM & 66-16d TO WOOD COLUMN. CAPACITY # 8685	HUCQ610-SDS WITH 12-1/4"x2 1/2" TO WOOD COLUMN AND 6-1/4"x2 1/2" TO WOOD BEAM (#4680)	LGUM210-3-SDS WITH 8-3/8"x4" TO THE CMU WALL & 8-1/4"x2 1/2" TO THE WOOD BEAM. CAPACITY #9715
WB6	(3) 1 3/4"x11 1/4"		1.9E	-	-	HUCQ612-SDS WITH 14-1/4"x2 1/2" TO WOOD COLUMN AND 6-1/4"x2 1/2" TO WOOD BEAM (# 5315)	-
WB7	(3) 1 3/4"x14"		1.9E	-	-	HGU5.50-SDS WITH 36-1/4"x2 1/2" TO WOOD COLUMN AND 24-1/4"x2 1/2" TO WOOD BEAM WITH ONE FLANGE CONCEALED (#14145)	HGUM5.50 SDS WITH 8-5/8"x5" TITEN HD BOLTS TO CMU AND 24-1/4"x2 1/2" SDS TO WOOD BEAM WITH ONE FLANGE CONCEALED (# 14940)

### SPECIFICATIONS ON THE LVL HEADER

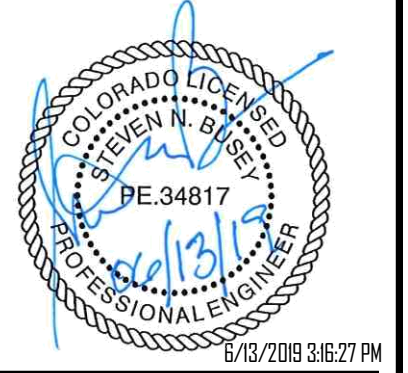
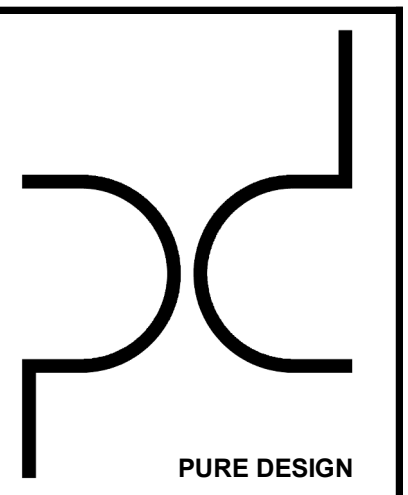
COMPOSITE TYPE AND GRADE;  
TENSION PARALLEL TO GRAIN;  
COMPRESSION PARALLEL TO GRAIN;  
COMPRESSION PERPENDICULAR TO GRAIN;  
SHEAR PARALLEL TO GRAIN;  
MODULUS OF ELASTICITY;  
REFERENCE MODULUS OF ELASTICITY;  
MEAN SHEAR MODULUS;  
AVERAGE DENSITY;

MICROLLAM LVL, 1.9E-2600FB GRADE  
FB = 2600 LB/IN<sup>2</sup>  
FT = 1555 LB/IN<sup>2</sup>  
FC = 2510 LB/IN<sup>2</sup>  
FC\_PERP = 750 LB/IN<sup>2</sup>  
FV = 285 LB/IN<sup>2</sup>  
E = 1900000 LB/IN<sup>2</sup>  
EMIN = 985710 LB/IN<sup>2</sup>  
GDEF = E / 16 = 118750 LB/IN<sup>2</sup>  
R = 42 LB/FT<sup>3</sup>

### AMMENITIES WOOD HEADER SCHEDULE

MARK	SIZE	TYPE	GRADE
H1	(2)2"x8"		SPF NO. 2
H2	(3)2"x8"		SPF NO. 2
H3	(4)2"x10"		SPF NO. 2
H4	(4)2"x8"		SPF NO. 2

NOTE 1 : ALL HEADERS SCHEDULED ARE MINIMUMS AND CAN BE INCREASED IN DEPTH OR PLY'S AS REQUIRED.



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1-320-259-6557

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ABERDEEN, SD 57401  
PHONE: 605.225.1712



**DUAL HOTEL DURANGO**  
**- AC & HIS**

AC HOTELS

JOB NO: 18-007  
DATE: 2018.03.07

ISSUE RECORD:  
REVISIONS:

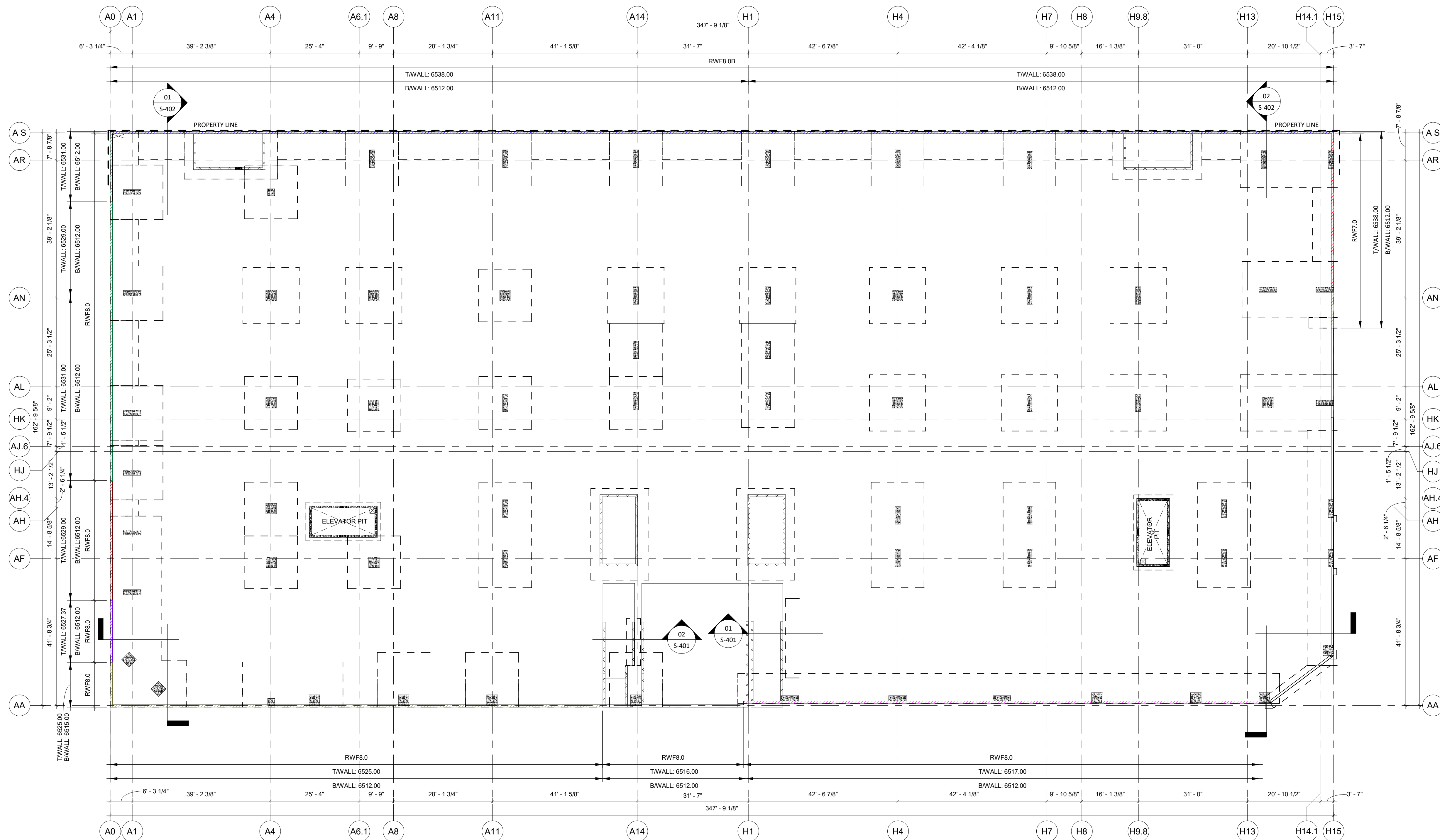
NEW 2019.06.14

SCHEDULES (CONT.)

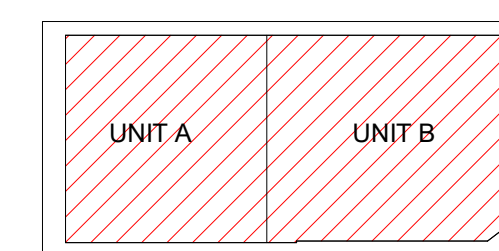
**S-106**

### FOUNDATION NOTES

- NOTES:
1. ALL EXTERIOR SLAB-ON-GRADE SHALL BE A MIN. OF 4" AND REINFORCED W/ 6x6 W2.9x2.9 OVER A 10 MIL. VAPOR BARRIER W/ 6" LAP.
  2. VERIFY ALL DIMENSIONS W/ ARCHITECTURAL BEFORE BEGINNING CONSTRUCTION. DIMENSIONS ON ARCHITECTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER DIMENSIONS ON STRUCTURAL DRAWINGS.
  3. REFER TO ARCH DWGS FOR OPENING DIMENSIONS NOT SHOWN ON PLAN BELOW.
  4. SEE ARCH & CIVIL PLANS FOR SLOPES AND GRADES FOR DRAINAGE.
  5. THE CENTROIDS OF COLUMNS & THEIR FOOTINGS SHALL BE COINCIDENT U.N.O.
  6. FTG. ELEVATION SEE FTG. SCHEDULE ON S-103 & S-104
  7. COORDINATE FLOOR OUTLETS AND UNDERGROUND ELECTRICAL CONDUIT WITH ELECTRICAL CONTRACTOR.
  8. FOR THE CONFLICTS BETWEEN PLUMBING PIPES AND STRUCTURAL WALL FOOTINGS GC HAS 2 OPTIONS AS BELOW:  
 OPTION 1: GC WILL REFER TO A TYPICAL DETAIL OF SLEEVE WHICH WE ALREADY HAVE IN OUR DRAWING. WHILE IN CASE HE GOES FOR OPTION 2: THE GC WILL APPROACH US THROUGH AN RFI, POST WHICH WE WILL TELL HIM WHICH FOOTING TO LOWER AND BY HOW MUCH.



② FOUNDATION PLAN  
1/16" = 1'-0"



KEY PLAN

**BSE STRUCTURAL ENGINEERS**  
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 1132 West 79th Street  
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 Phone 913.492.7400  
 www.BSEStructural.com  
 Project Number 18-550

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**DUAL HOTEL DURANGO**  
 - AC & HIS

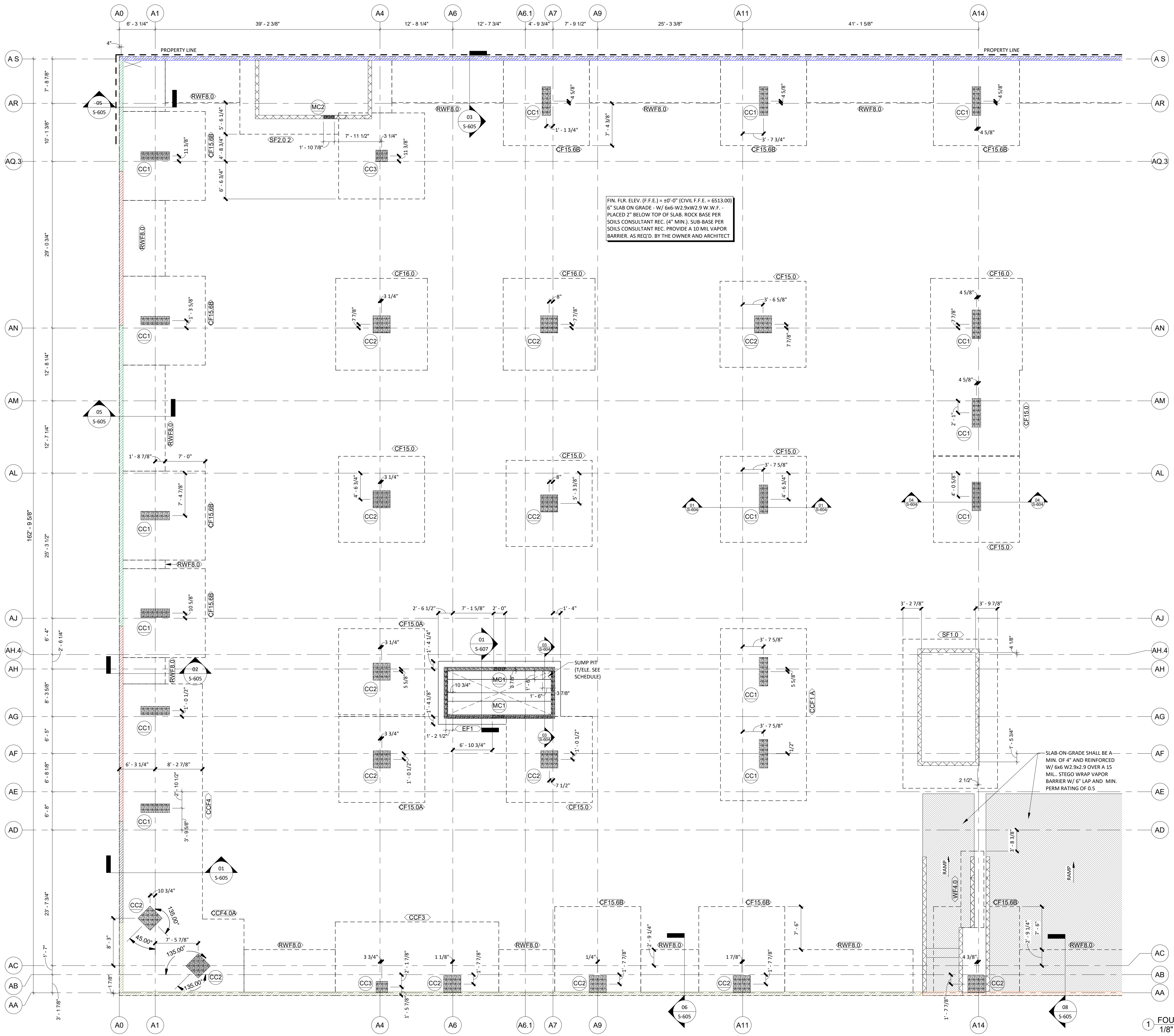
AC HOTELS

JOB NO: 18-007  
 DATE: 2018.03.07  
 ISSUE RECORD:  
 REVISIONS:

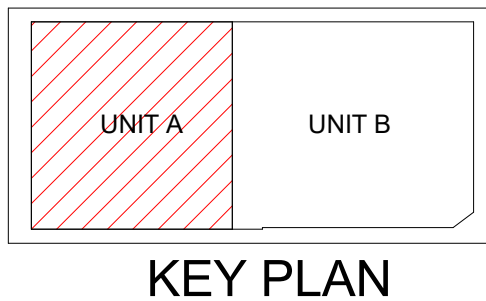
NEW 2019.06.14

FOUNDATION PLAN

**S-201.1**



FIN. FLR. ELEV. (F.F.E.) = ±0'-0" (CIVIL F.F.E. = 6513.00)  
 6" SLAB ON GRADE - W/ 6x6-W2.9xW2.9 W.W.F. -  
 PLACED 2" BELOW TOP OF SLAB. ROCK BASE PER  
 SOILS CONSULTANT REC. (4" MIN.). SUB-BASE PER  
 SOILS CONSULTANT REC. PROVIDE A 10 MIL VAPOR  
 BARRIER. AS REQ'D. BY THE OWNER AND ARCHITECT



- NOTES:
- 1) ALL EXTERIOR SLAB-ON-GRADE SHALL BE A MIN. OF 4" AND REINFORCED W/ 6x6 W2.9x2.9 OVER A 15 MIL. STEGO WRAP VAPOR BARRIER W/ 6" LAP AND MIN. PERM RATING OF 0.5
  - 2) VERIFY ALL DIMENSIONS W/ ARCHITECTURAL BEFORE BEGINNING CONSTRUCTION. DIMENSIONS ON ARCHITECTURAL DRAWINGS SHALL TAKE PRECEDENT OVER DIMENSIONS ON STRUCTURAL DRAWINGS.
  - 3) REFER TO ARCH DWGS FOR OPENING DIMENSIONS NOT SHOWN ON PLAN.
  - 4) SEE ARCH & CIVIL PLANS FOR SLOPES AND GRADES FOR DRAINAGE.
  - 5) THE CENTROIDS OF COLUMNS & THEIR FOOTINGS SHALL BE COINCIDENT U.N.O.
  - 6) FTG. ELEVATION SEE FTG. SCHEDULE ON S-103 & S-104
  - 7) COORDINATE FLOOR OUTLETS AND UNDERGROUND ELECTRICAL CONDUIT WITH ELECTRICAL CONTRACTOR.
  - 8) FOR THE CONFLICTS BETWEEN PLUMBING PIPES AND STRUCTURAL WALL FOOTINGS GC HAS 2 OPTIONS AS BELOW :  
 OPTION 1: GC WILL REFER TO A TYPICAL DETAIL OF SLEEVE WHICH WE ALREADY HAVE IN OUR DRAWING, WHILE IN CASE HE GOES FOR OPTION 2: THE GC WILL APPROACH US THROUGH AN RFI, POST WHICH WE WILL TELL HIM WHICH FOOTING TO LOWER AND BY HOW MUCH.
  - 9) SEE DRAWING S-101 & S-102 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST.
  - 10) REFERENCE DRAWING S-604 THRU S606 FOR TYPICAL FOUNDATION DETAILS.
  - 11) MASONRY CMU WALLS ARE 8" U.N.O.
  - 12) REFERENCE DRAWING S-609 & S-106 FOR TYPICAL MASONRY LINTEL DETAILS.
  - 13) REFER TO GEOTECHNICAL REPORT FOR BEHIND WALL DRAINAGE RECOMMENDATIONS. COORD. W/ CIVIL AS REQ'D. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION WATERPROOFING & INSULATION REQUIREMENTS.

1 FOUNDATION PLAN PART A  
 1/8" = 1'-0"

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**DUAL HOTEL DURANGO**  
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AC HOTELS

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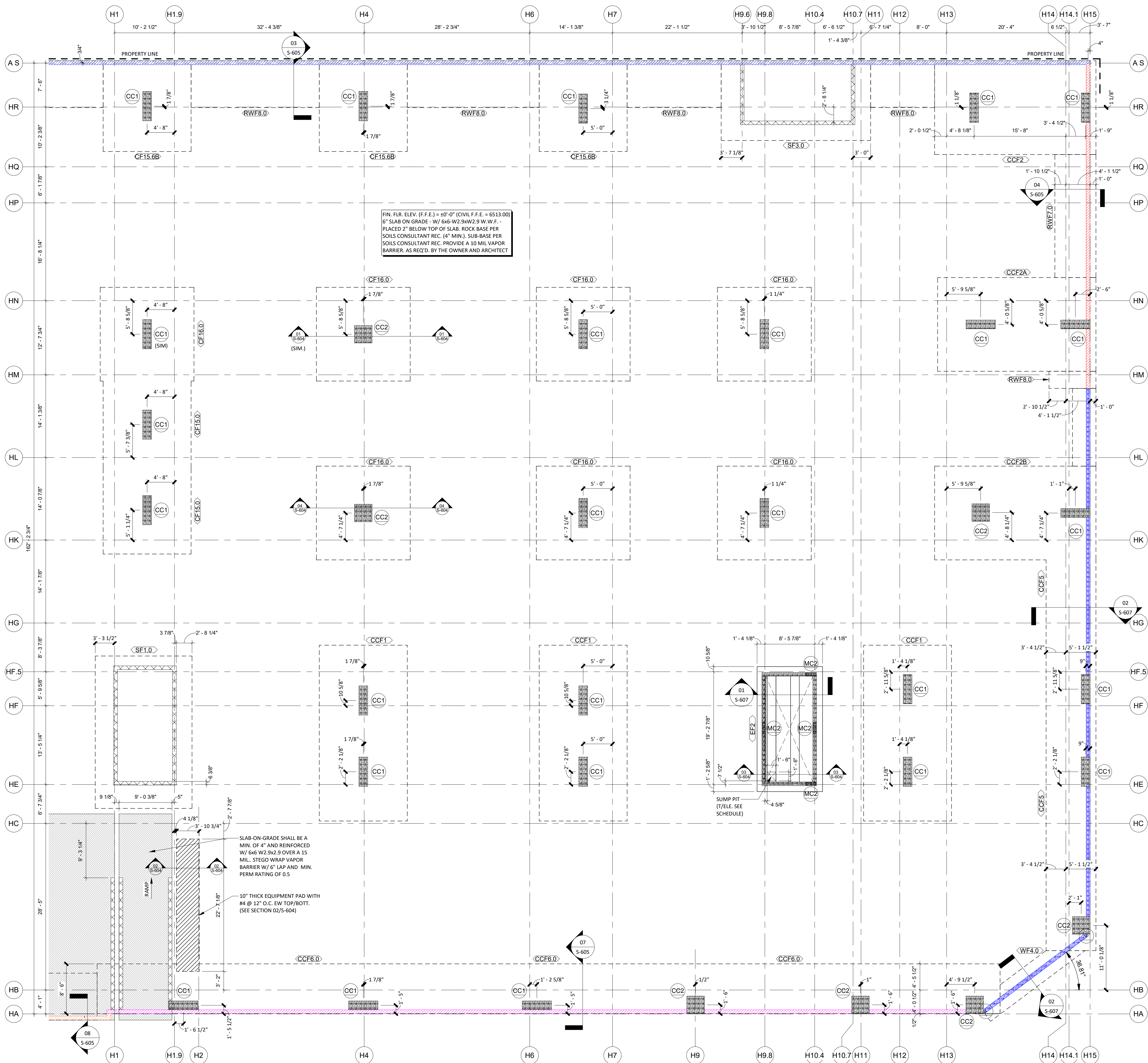
NEW 2019.06.14

**BSE STRUCTURAL ENGINEERS**

FOUNDATION PLAN PART A

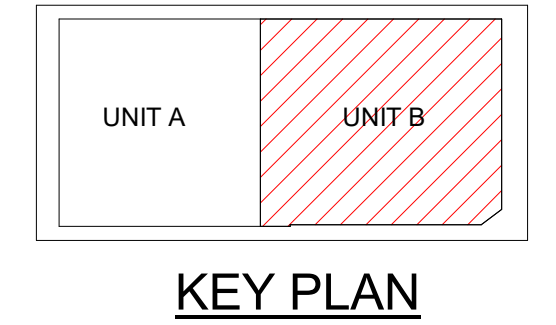
**S-2011A**





FIN. FLR. ELEV. (F.F.E.) = 50'-0" (CIVIL F.F.E. = 6513.00)  
 6" SLAB ON GRADE - W/ 6x6-W2.9xW2.9 W.W.F. -  
 PLACED 2" BELOW TOP OF SLAB. ROCK BASE PER  
 SOILS CONSULTANT REC. (4" MIN.). SUB-BASE PER  
 SOILS CONSULTANT REC. PROVIDE A 10 MIL VAPOR  
 BARRIER, AS REQ'D. BY THE OWNER AND ARCHITECT

SLAB-ON-GRADE SHALL BE A  
 MIN. OF 4" AND REINFORCED  
 W/ 6x6 W2.9x2.9 OVER A 15  
 MIL. STEGO WRAP VAPOR  
 BARRIER W/ 6" LAP AND MIN.  
 PERM RATING OF 0.5  
 10" THICK EQUIPMENT PAD WITH  
 #4 @ 12" O.C. EW TOP/BOTT.  
 (SEE SECTION 02/5-604)



- NOTES:
- 1.) ALL EXTERIOR SLAB-ON-GRADE SHALL BE A MIN. OF 4" AND REINFORCED W/ 6x6 W2.9x2.9 OVER A 10 MIL VAPOR BARRIER W/ 6" LAP.
  - 2.) VERIFY ALL DIMENSIONS W/ ARCHITECTURAL BEFORE BEGINNING CONSTRUCTION. DIMENSIONS ON ARCHITECTURAL DRAWINGS SHALL TAKE PRECEDENT OVER DIMENSIONS ON STRUCTURAL DRAWINGS.
  - 3.) REFER TO ARCH DWGS FOR OPENING DIMENSIONS NOT SHOWN ON PLAN.
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  - 6.) FTG. ELEVATION SEE FTG. SCHEDULE ON S-103.85-104
  - 7.) COORDINATE FLOOR OUTLETS AND UNDERGROUND ELECTRICAL CONDUIT WITH ELECTRICAL CONTRACTOR.
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  - 11.) MASONRY CMU WALLS ARE 8" U.N.O.
  - 12.) REFERENCE DRAWING S-609 & S-106 FOR TYPICAL MASONRY LINTEL DETAILS.
  - 13.) REFER TO GEOTECHNICAL REPORT FOR BEHIND WALL DRAINAGE RECOMMENDATIONS. COORD. W/ CIVIL AS REQ'D. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION WATERPROOFING & INSULATION REQUIREMENTS.

FOUNDATION PLAN PART B  
 1/8" = 1'-0"

**BSE STRUCTURAL ENGINEERS**  
 NORTH  
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 Phone 913.492.7400  
 www.BSEstructural.com  
 Project Number 18-550

PURE DESIGN

ARCHITECT  
 PURE DESIGN  
 11264 N. MADISON AVE.  
 KANSAS CITY, MO 64155  
 PHONE: 316.617.2052

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CIVIL ENGINEERS  
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 Phone 970-385-4546

INTERIOR DESIGNER  
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 Culnery  
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 1-320-259-6557

OWNER  
 LAMONT COMPANIES  
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 ABERDEEN, SD 57401  
 PHONE: 605.225.1712

**DUAL HOTEL DURANGO**  
 - AC & HIS

AC HOTELS

JOB NO: 18-007  
 DATE: 2019.03.07  
 ISSUE RECORD:  
 REVISIONS:

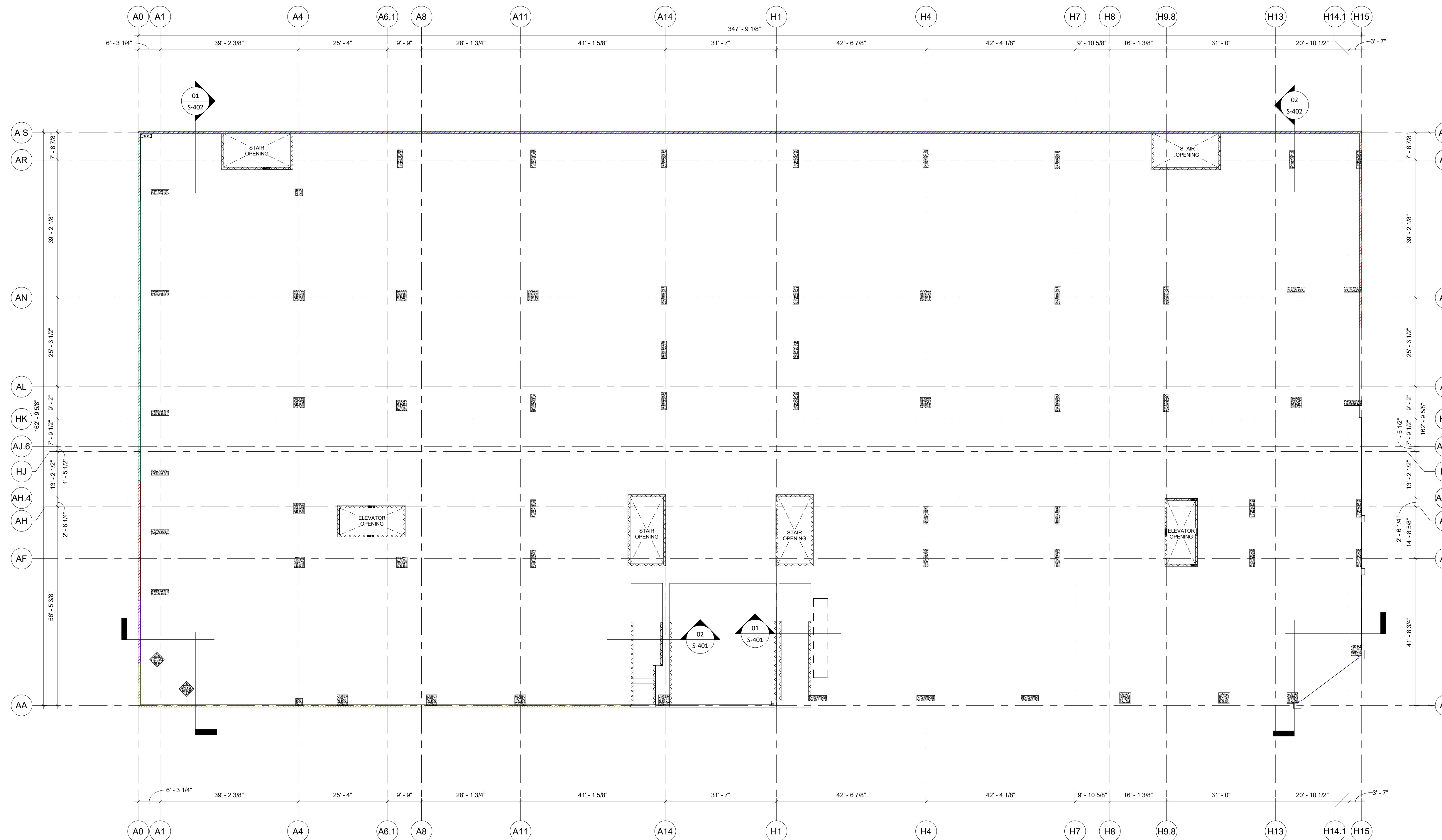
**NEW 2019.06.14**

FOUNDATION PLAN PART B

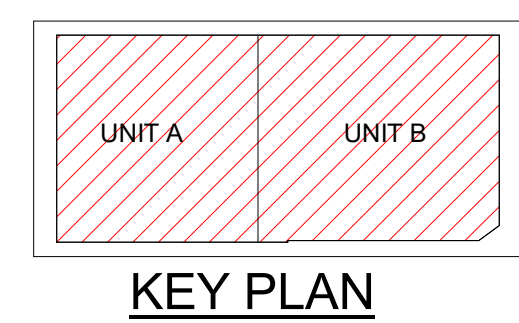
**S-201.B**

**WALL & COLUMN NOTES**

1. COLUMNS & SHEAR-BEARING WALLS AT THIS PLAN ELEVATION FROM 0'-0" TO 10'-10"
2. SEE SCHEDULES ON S103 FOR COLUMN AND WALL TYPE.
3. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.



1 1ST FLOOR WALL AND COLUMN PLAN  
1/16" = 1'-0"



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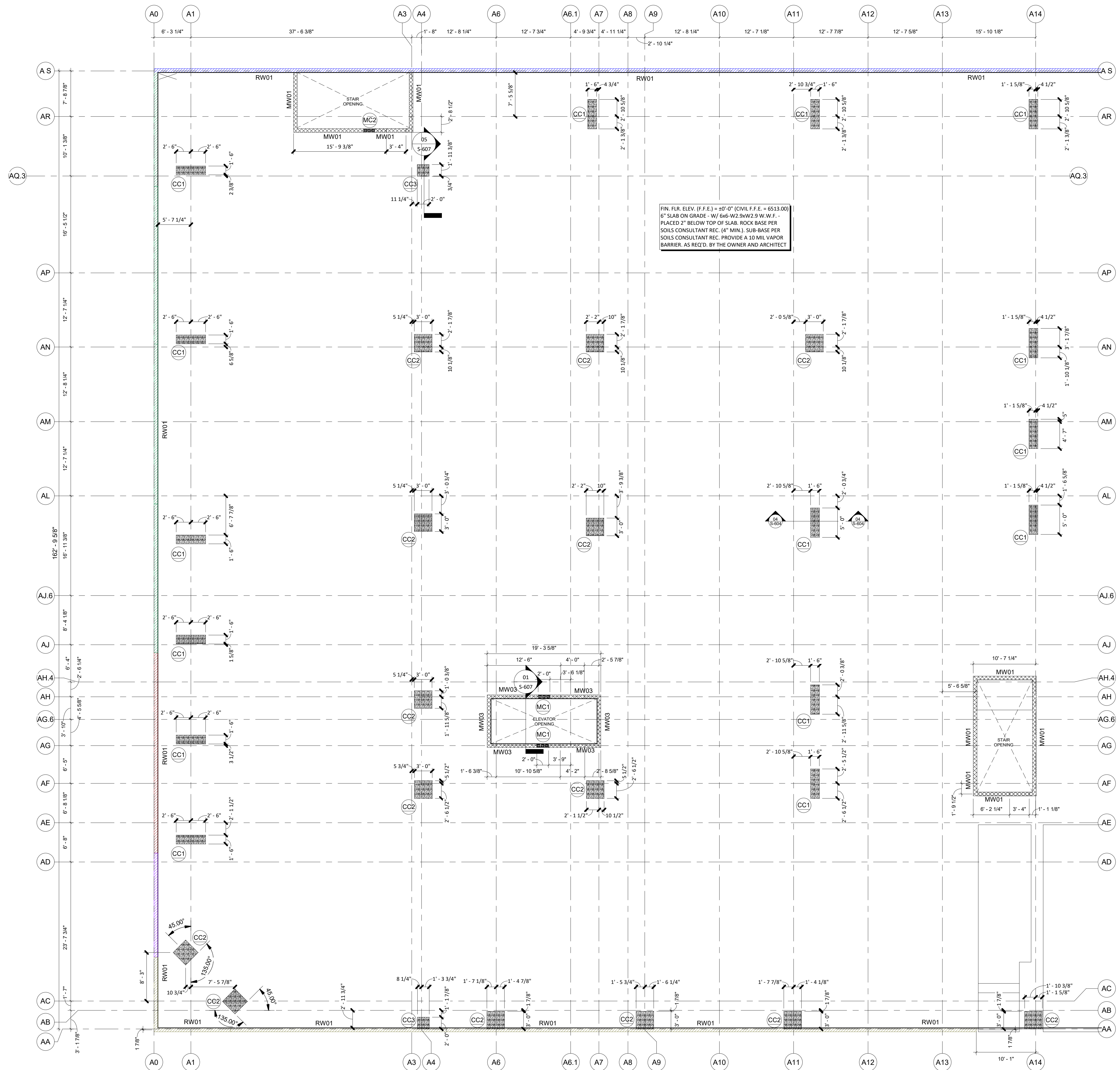
**DUAL HOTEL DURANGO**  
 - AC & HIS

JOB NO: 18-007  
 DATE: 2018.03.07  
 ISSUE RECORD:  
 REVISIONS:

NEW 2019.06.14

1ST FLOOR WALL AND COLUMN PLAN

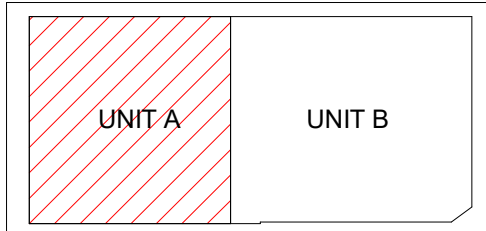
**S-201.2**



FIN. FLR. ELEV. (F.F.E.) = +0'-0" (CIVIL F.F.E. = 6513.00)  
 6" SLAB ON GRADE - W/ 6x6-W2.9xW2.9 W.W.F. -  
 PLACED 2" BELOW TOP OF SLAB. ROCK BASE PER  
 SOILS CONSULTANT REC. (4" MIN). SUB-BASE PER  
 SOILS CONSULTANT REC. PROVIDE A 10 MIL VAPOR  
 BARRIER, AS REQ'D. BY THE OWNER AND ARCHITECT

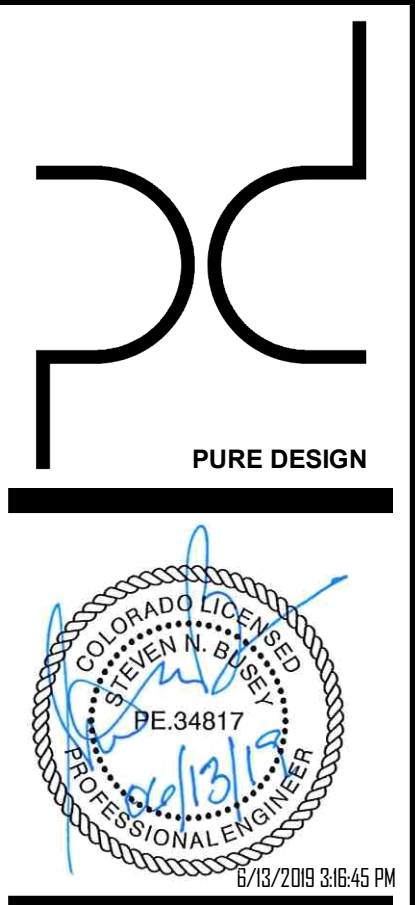
### WALL & COLUMN NOTES

1. COLUMNS & SHEAR-BEARING WALLS AT THIS PLAN ELEVATION FROM 0'-0" TO 10'-10"
2. SEE SCHEDULES ON S103 FOR COLUMN AND WALL TYPE.
3. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.



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1 1ST FLOOR WALL AND COLUMN PLAN PART A  
 1/8" = 1'-0"



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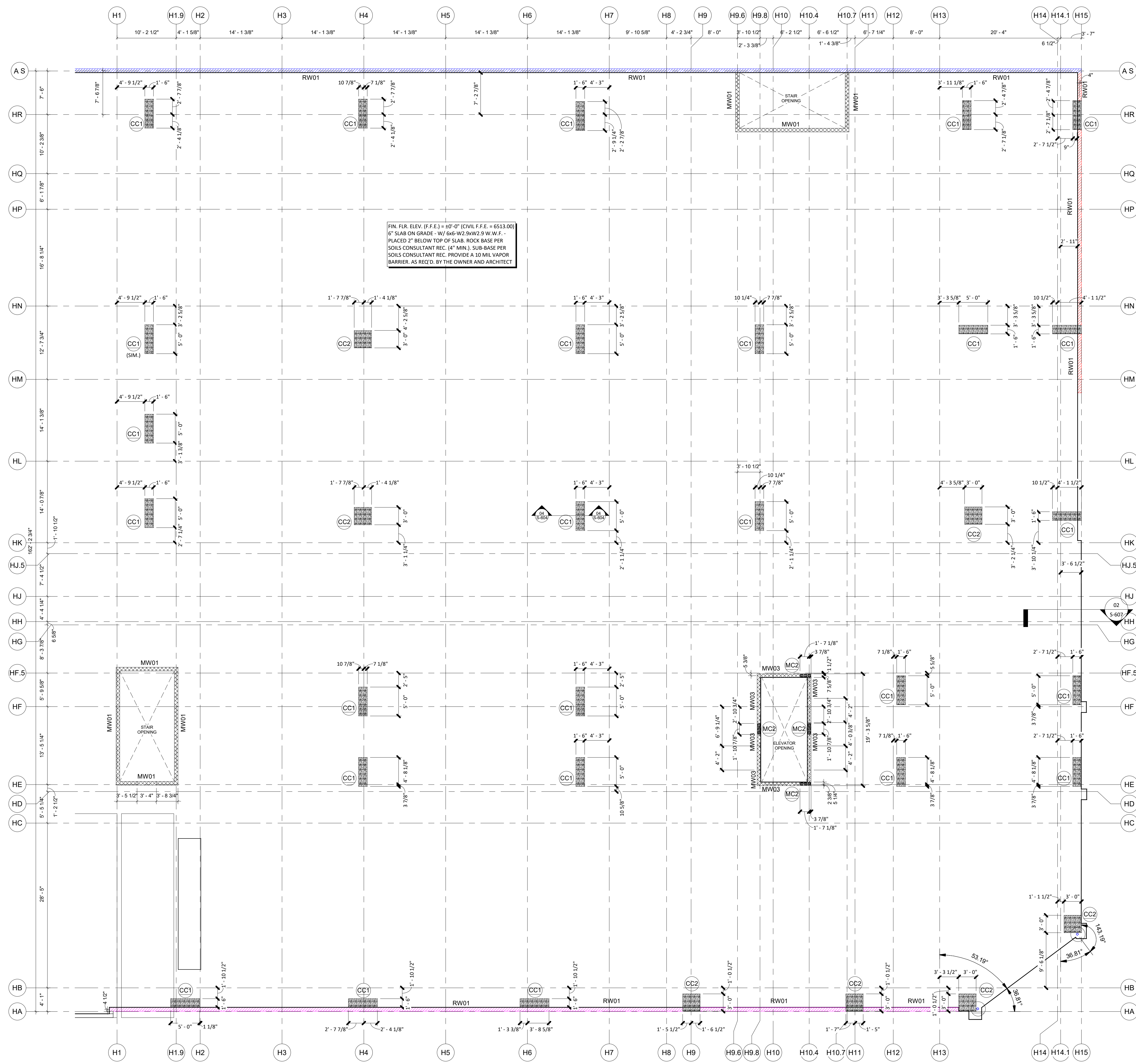
**OWNER**  
 LAMONT COMPANIES  
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**DUAL HOTEL DURANGO**  
**- AC & HIS**

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 REVISIONS:

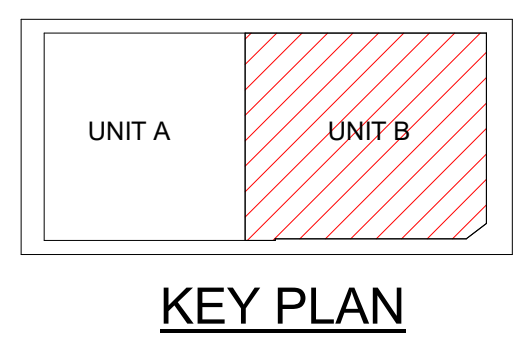
NEW 2019.06.14

1ST FLOOR WALL AND COLUMN PLAN  
 PART A  
**S-201.2A**



**WALL & COLUMN NOTES**

- COLUMNS & SHEAR/BEARING WALLS AT THIS PLAN ELEVATION FROM 0'-0" TO 10'-10"
- SEE SCHEDULES ON S103 FOR COLUMN AND WALL TYPE.
- REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.



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1 1ST FLOOR WALL AND COLUMN PLAN PART B  
 1/8" = 1'-0"

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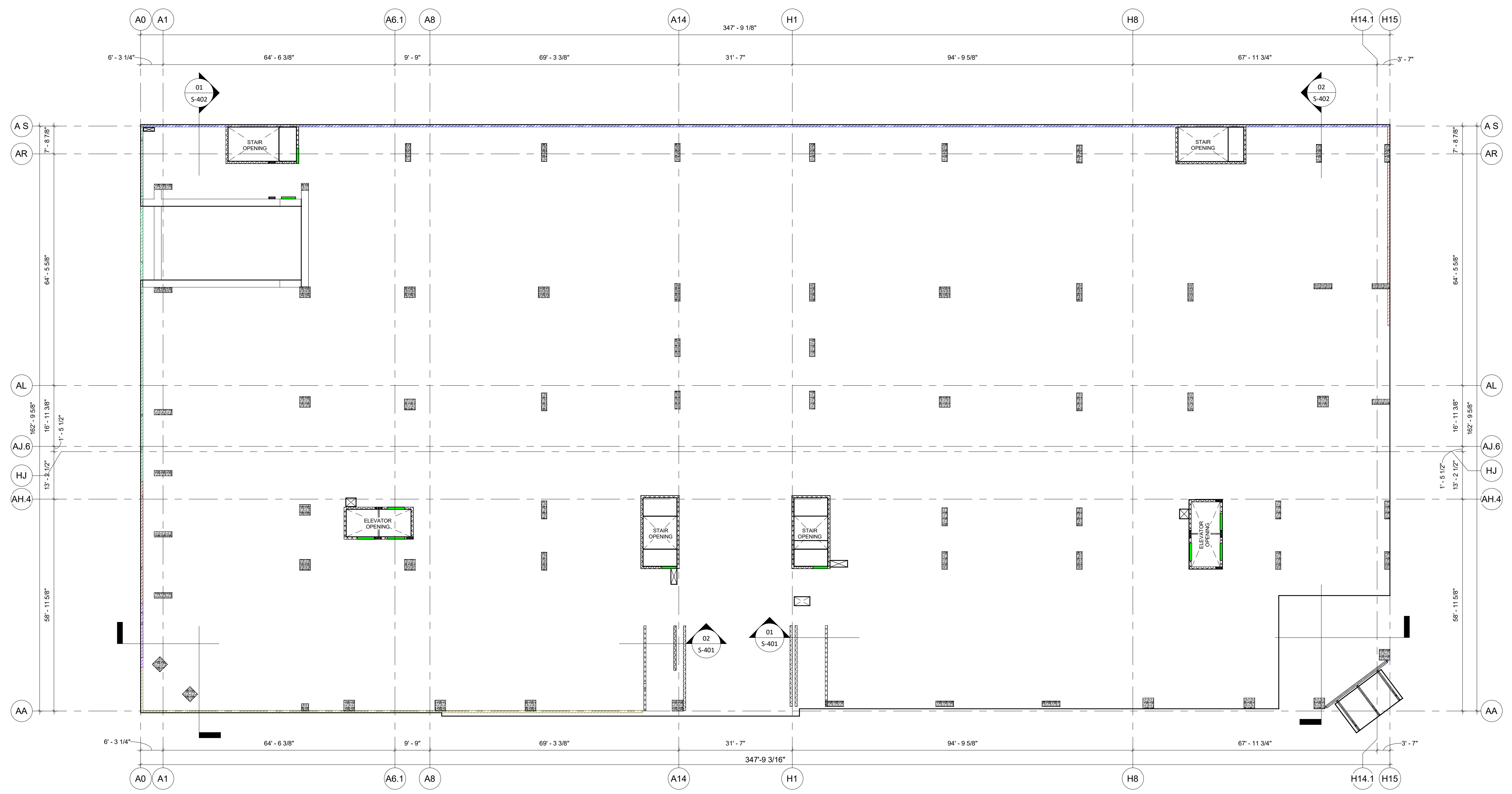
**DUAL HOTEL DURANGO**  
 - AC & HIS

JOB NO: 18-007  
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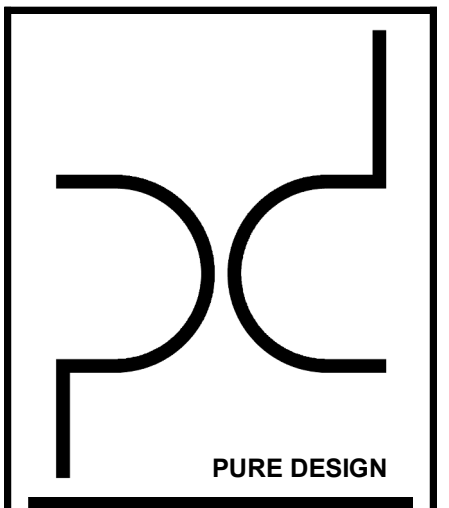
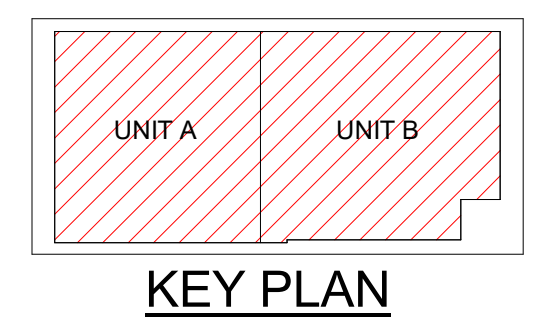
1ST FLOOR WALL AND COLUMN PLAN  
 PART B

**S-201.2B**

**FLOOR FRAMING PLAN NOTES**  
 1. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.  
 2. ELEVATION OF SECOND FINISH FLOOR IS +12'-0" U.N.O.



1 2ND FLOOR FRAMING PLAN  
 1/16" = 1'-0"



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**DUAL HOTEL DURANGO**  
 - AC & HIS

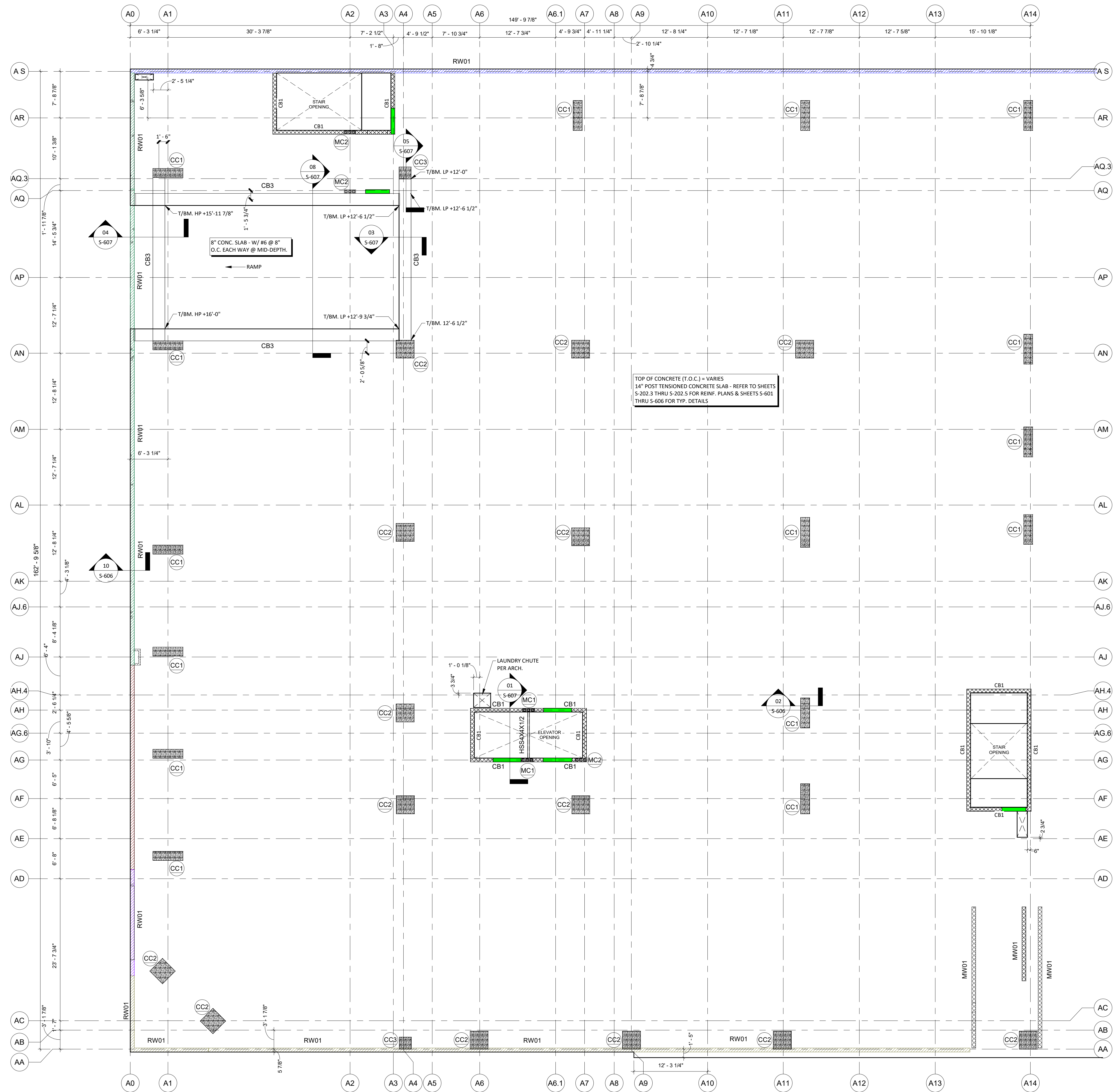
AC HOTELS

**JOB NO:** 18-007  
**DATE:** 2018.03.07  
**ISSUE RECORD:**  
**REVISIONS:**

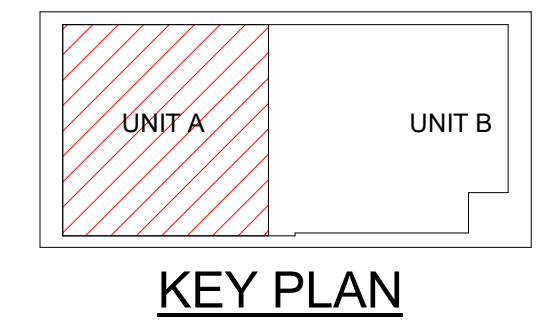
**NEW 2019.06.14**

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2ND FLOOR FRAMING PLAN  
**S-202.2**



TOP OF CONCRETE (T.O.C.) = VARIES  
 14" POST TENSIONED CONCRETE SLAB - REFER TO SHEETS  
 S-202.3 THRU S-202.5 FOR REIN. PLANS & SHEETS S-601  
 THRU S-606 FOR TYP. DETAILS



- NOTES:**
- 1.) SEE DRAWING S-101 & S-102 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST.
  - 2.) REFERENCE DRAWING S-601 THRU S-606 FOR TYPICAL FRAMING DETAILS.
  - 3.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL WALL OPENINGS.
  - 4.) MASONRY CMU WALLS ARE 8" U.N.D.
  - 5.) REFERENCE DRAWING S-609 & S-106 FOR TYPICAL MASONRY LINTEL DETAILS.
  - 6.) ALL EXTERIOR LINTELS SUPPORTING STONE OR BRICK SHALL BE GALVANIZED U.N.D. ON ARCHITECTURAL DRAWINGS.
  - 7.) G.C. TO COORD. ALL OPENING LOCATIONS W/ ARCHITECTURAL DRAWINGS, M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONC.

1 2ND FLOOR FRAMING PLAN PART A  
 1/8" = 1'-0"

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**PURE DESIGN**

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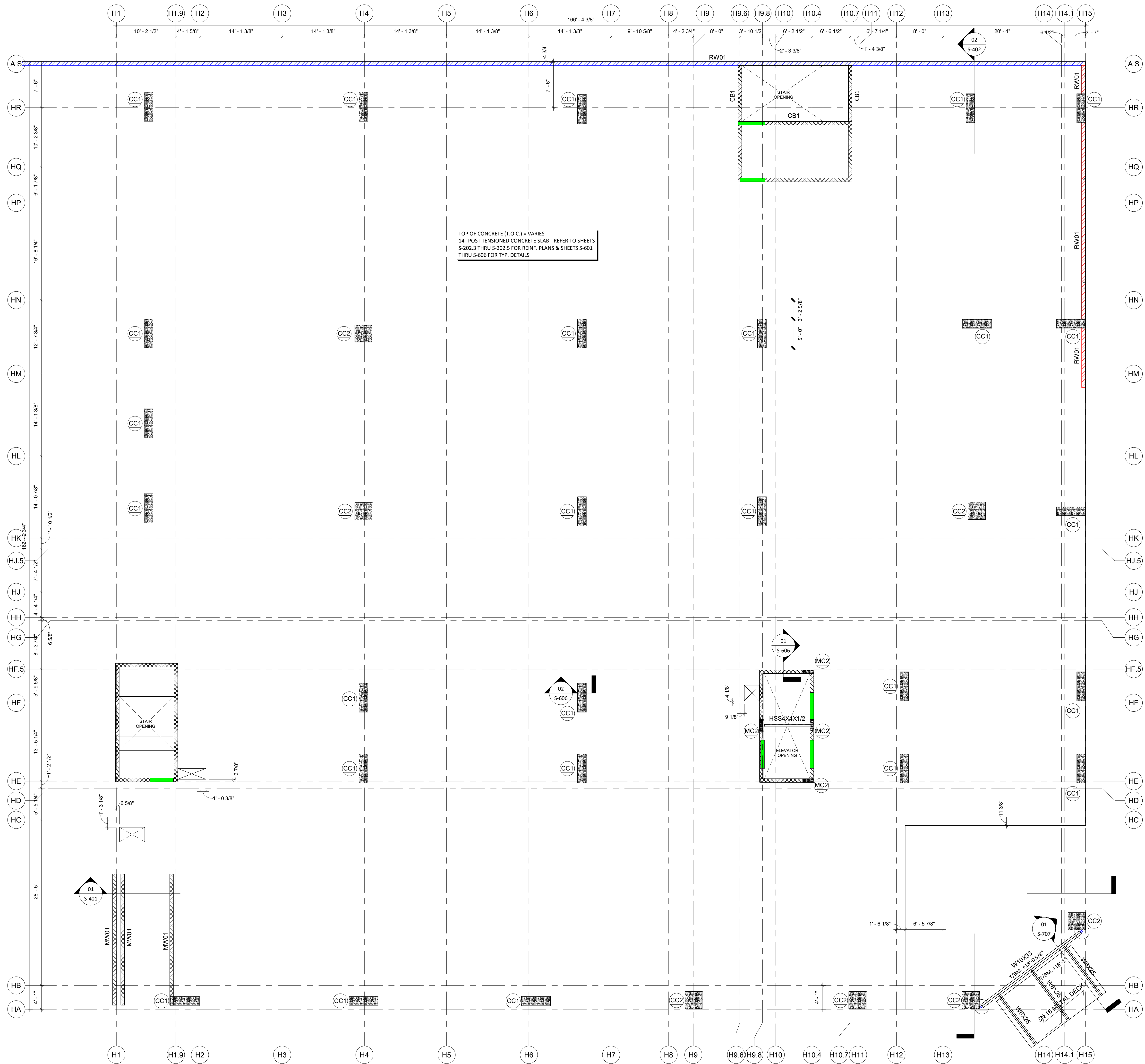
OWNER  
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**DUAL HOTEL DURANGO**  
 - AC & HIS

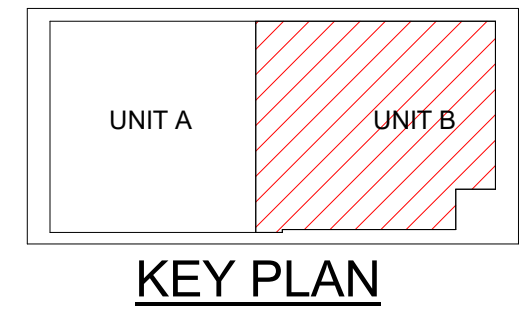
AC HOTELS

JOB NO: 18-007  
 DATE: 2018.03.07  
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 NEW 2019.06.14

2ND FLOOR FRAMING PLAN PART A  
**S-202.2A**



TOP OF CONCRETE (T.O.C.) = VARIES  
 14\"/>



- NOTES:
- 1.) SEE DRAWING S-101 & S-102 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST.
  - 2.) REFERENCE DRAWING S-601 THRU S-606 FOR TYPICAL FRAMING DETAILS.
  - 3.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL WALL OPENINGS.
  - 4.) MASONRY CMU WALLS ARE 8\"/>

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1 2ND FLOOR FRAMING PLAN PART B  
 1/8" = 1'-0"



PURE DESIGN



6/13/2019 3:07:00 PM

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 11264 N. MADISON AVE.  
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**DUAL HOTEL DURANGO**  
 - AC & HIS



AC HOTELS

JOB NO: 18-007  
 DATE: 2018.03.07  
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2ND FLOOR FRAMING PLAN PART B

**S-202.2B**



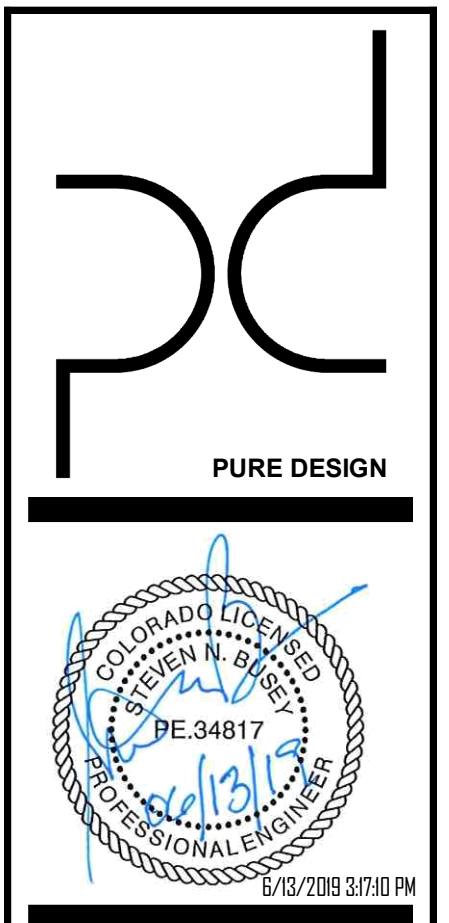
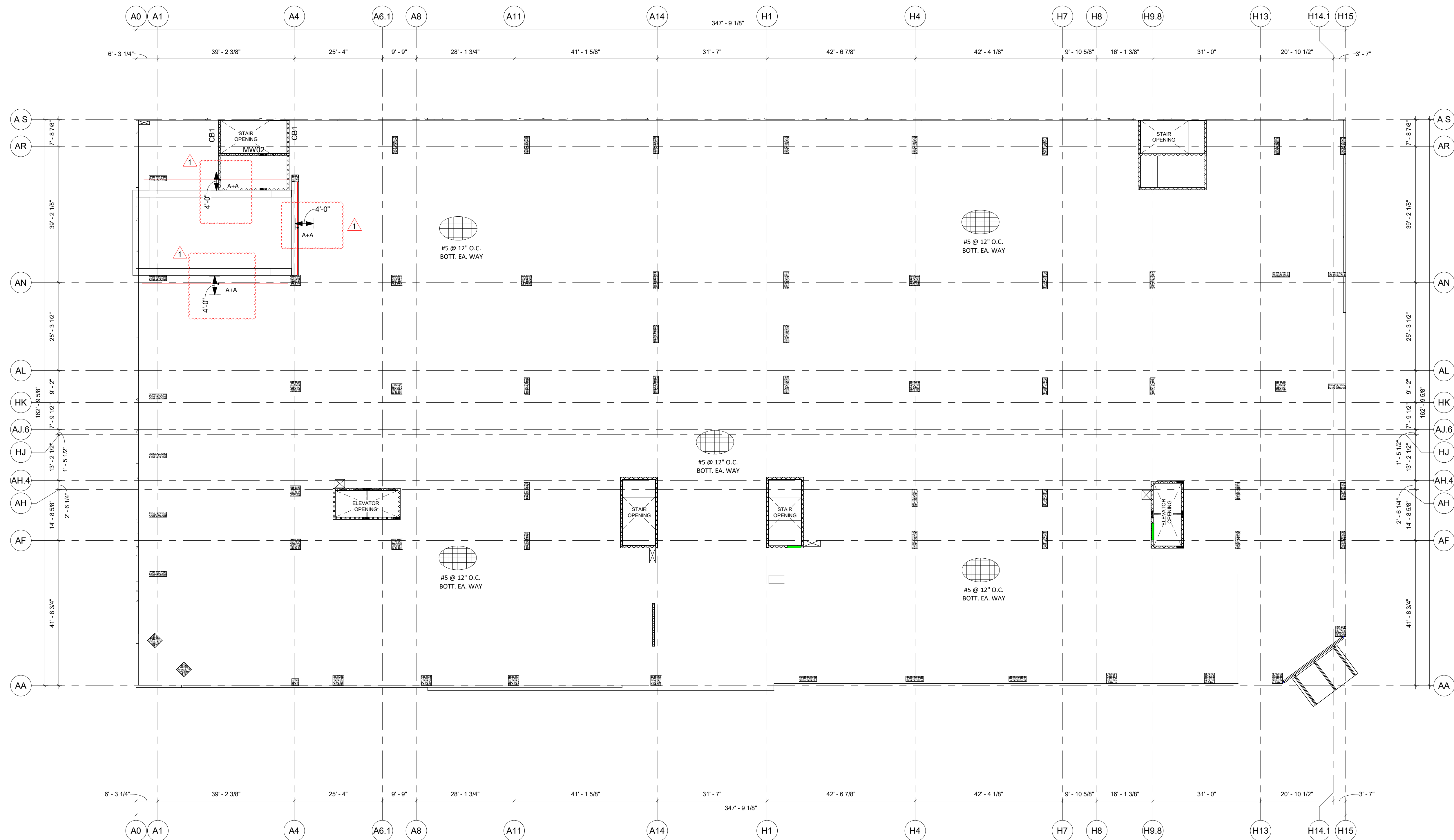


**LEGENDS OF BARS**

BAR MARKED	DIA. & SPACING
A	#5 @ 12" O.C.

**REBAR LAYOUT NOTES**

1. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
2. ELEVATION OF SECOND FLOOR LEVEL FINISH IS +12'-0" U.N.O.
3. CONCRETE SLAB THICKNESS IS 14" U.N.O.



**ARCHITECT**  
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**DUAL HOTEL DURANGO**  
 - AC & HIS

AC HOTELS

JOB NO: 18-007  
 DATE: 2018.03.07  
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 REVISIONS:  
 1 05/17/2019

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1 2ND FLOOR BOTTOM REBAR LAYOUT PLAN  
 1/16" = 1'-0"

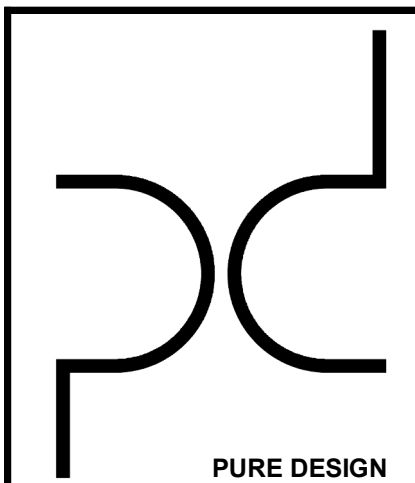
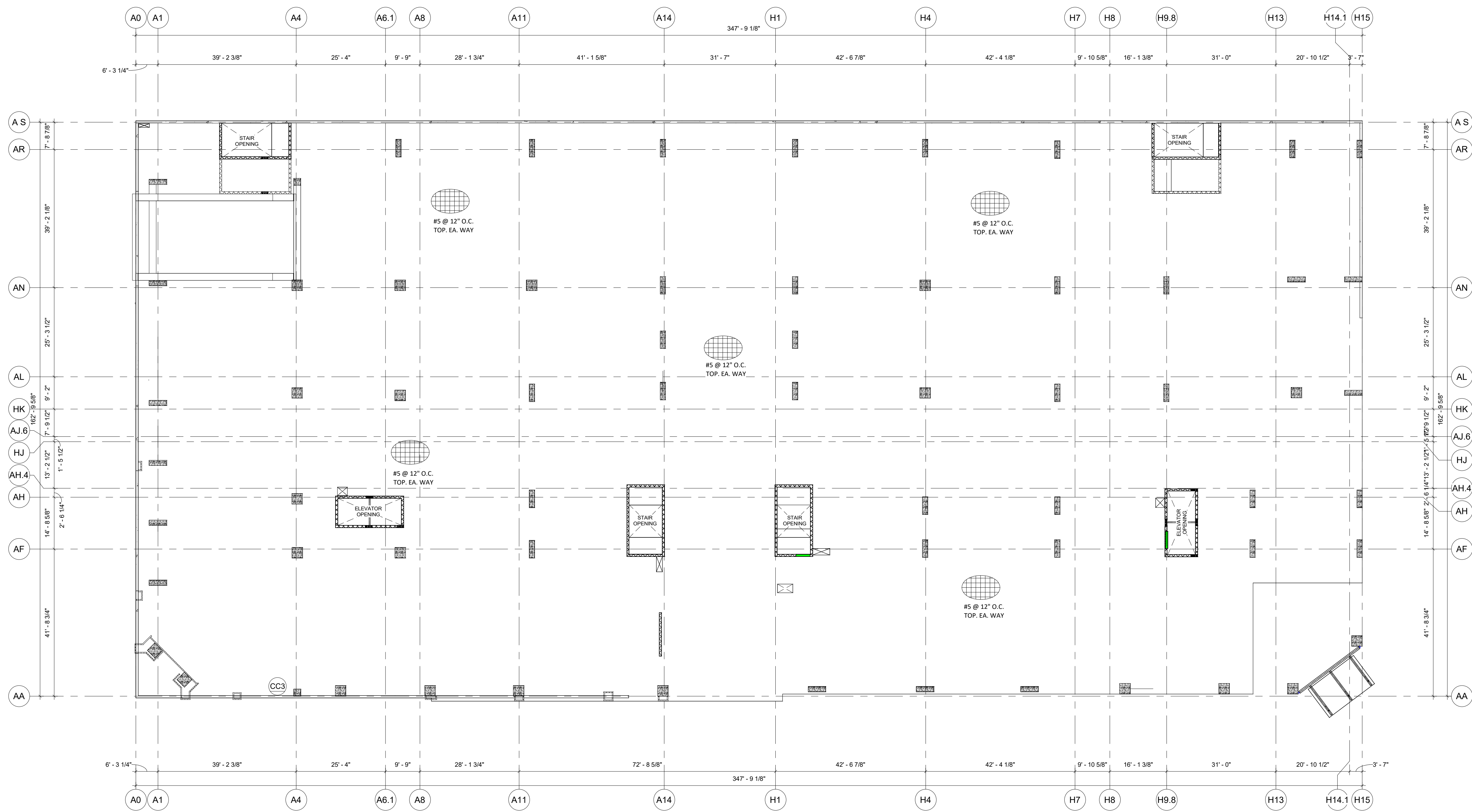
2ND FLOOR BOTTOM REBAR LAYOUT PLAN  
**S-202.4**

**LEGENDS OF BARS**

BAR MARKED	DIA. & SPACING
A	#5 @ 12" O.C.
B	#6 @ 12" O.C.

**REBAR LAYOUT NOTES**

1. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
2. ELEVATION OF SECOND FLOOR LEVEL FINISH IS +12'-0" U.N.O.
3. CONCRETE SLAB THICKNESS IS 14" U.N.O.



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**DUAL HOTEL DURANGO**  
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JOB NO: 18-007  
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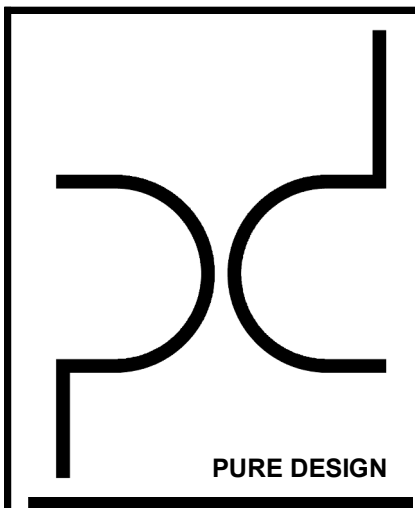
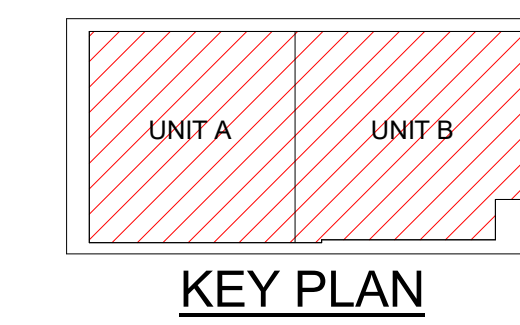
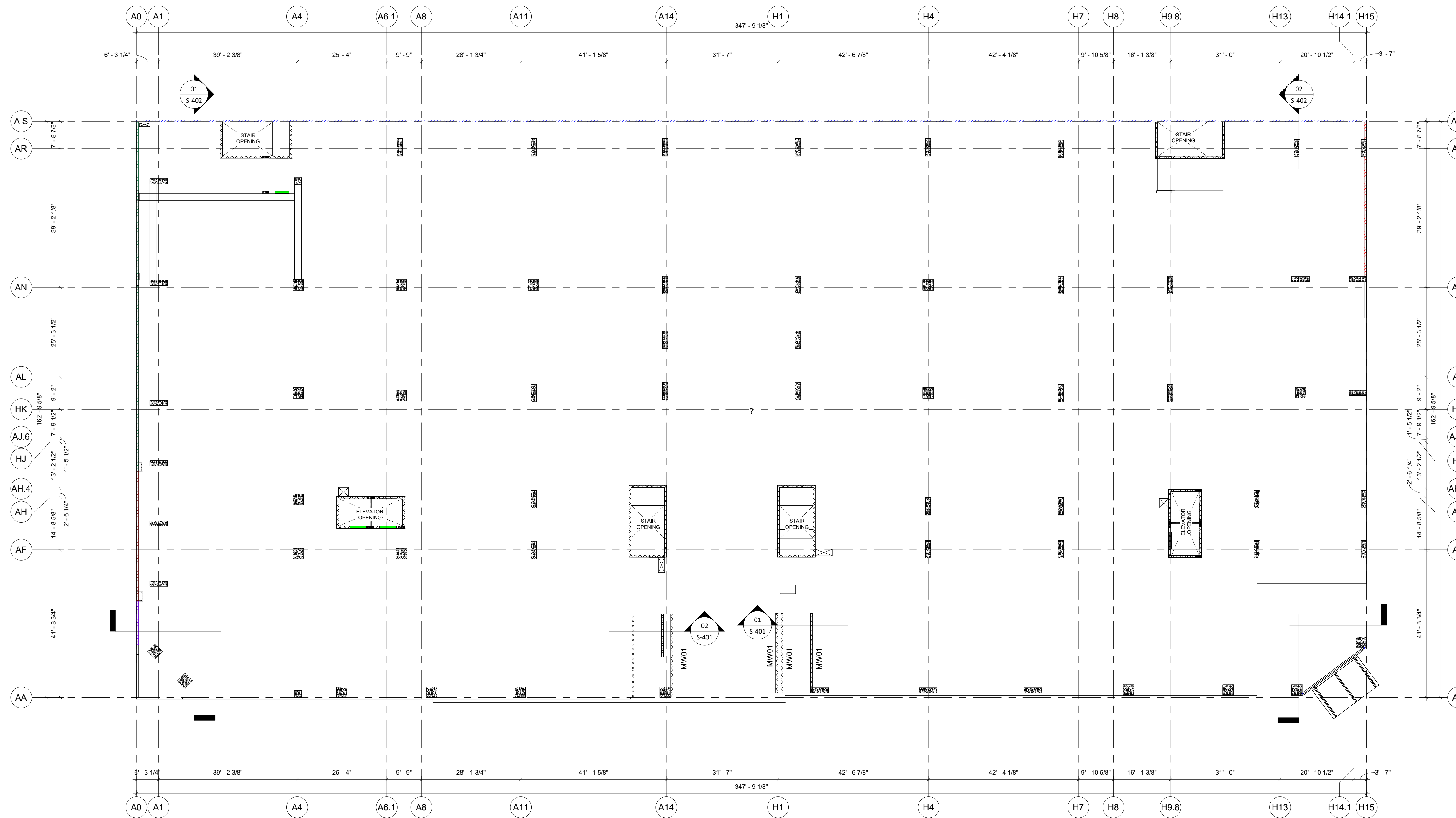
**2ND FLOOR ADDITIONAL TOP REBAR LAYOUT PLAN**  
**S-202.5**

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**2ND FLOOR ADDITIONAL TOP REBAR LAYOUT PLAN**  
 1/16" = 1'-0"

**WALL AND COLUMN NOTES**

1. COLUMNS & SHEAR BEARING WALLS AT THIS PLAN ELEVATION FROM 12'-0"-23'-8"
2. SEE SCHEDULES ON S103 & S106 FOR COLUMN AND WALL TYPE.
3. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
4. G.C. TO COORD. ALL OPENING LOCATIONS W/ ARCHITECTURAL DRAWINGS, M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONC.



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**- AC & HIS**

JOB NO: 18-007  
 DATE: 2018.03.07

ISSUE RECORD:  
 REVISIONS:

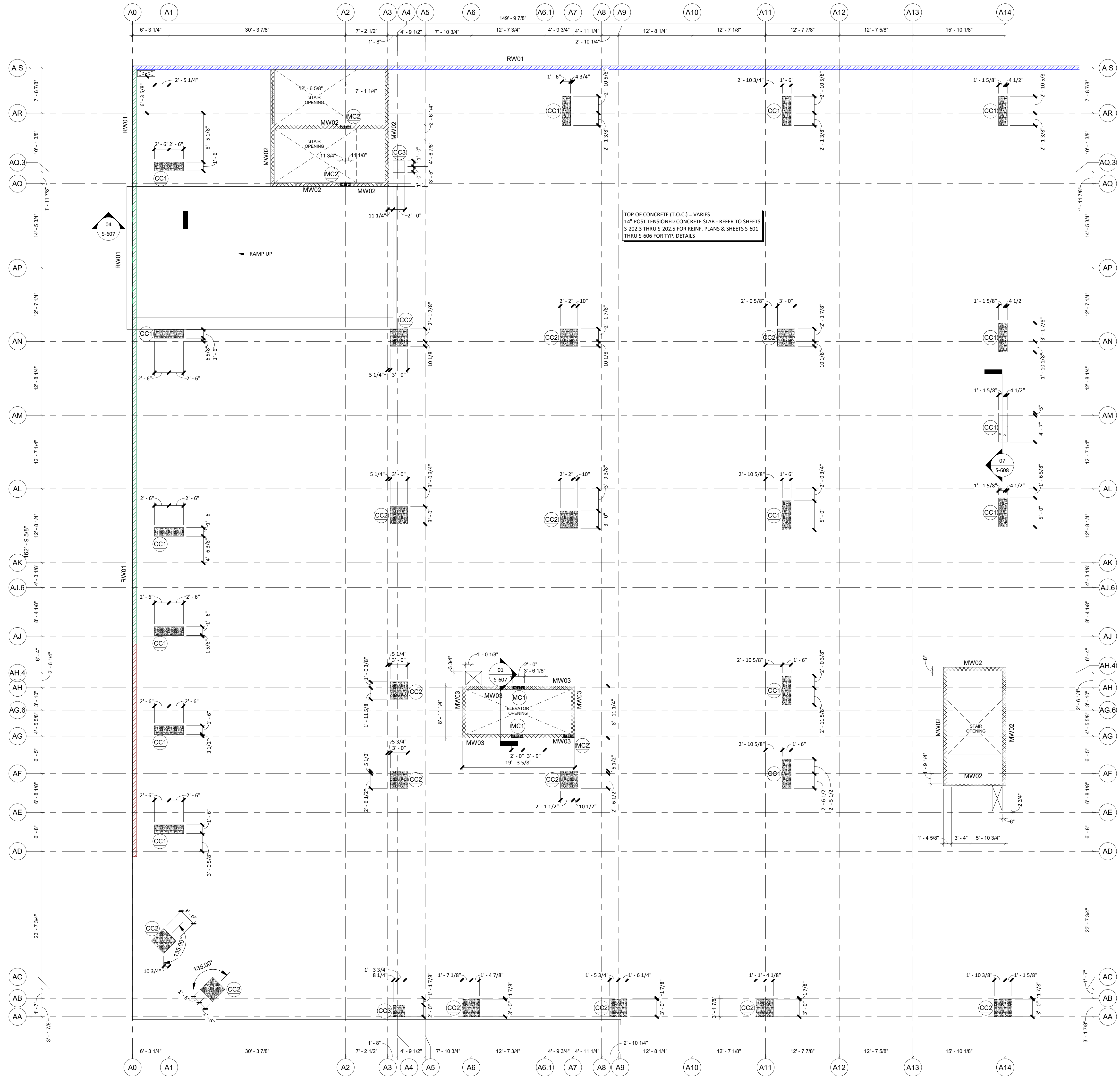
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2ND FLOOR WALL AND COLUMN PLAN

**S-202.6**

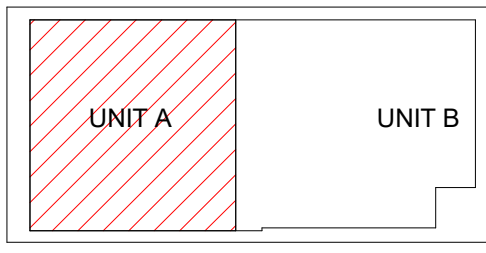
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 Project Number 18-550

① 2ND FLOOR WALL AND COLUMN PLAN  
 1/16" = 1'-0"



**WALL AND COLUMN NOTES**

1. COLUMNS & SHEAR/BEARING WALLS AT THIS PLAN ELEVATION FROM 12'-0"-23'-8"
2. SEE SCHEDULES ON S103 & S106 FOR COLUMN AND WALL TYPE
3. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
4. G.C. TO COORD. ALL OPENING LOCATIONS W/ ARCHITECTURAL DRAWINGS, M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONC



1 2ND FLOOR WALL AND COLUMN PLAN PART A  
1/8" = 1'-0"

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**DUAL HOTEL DURANGO**  
- AC & HIS



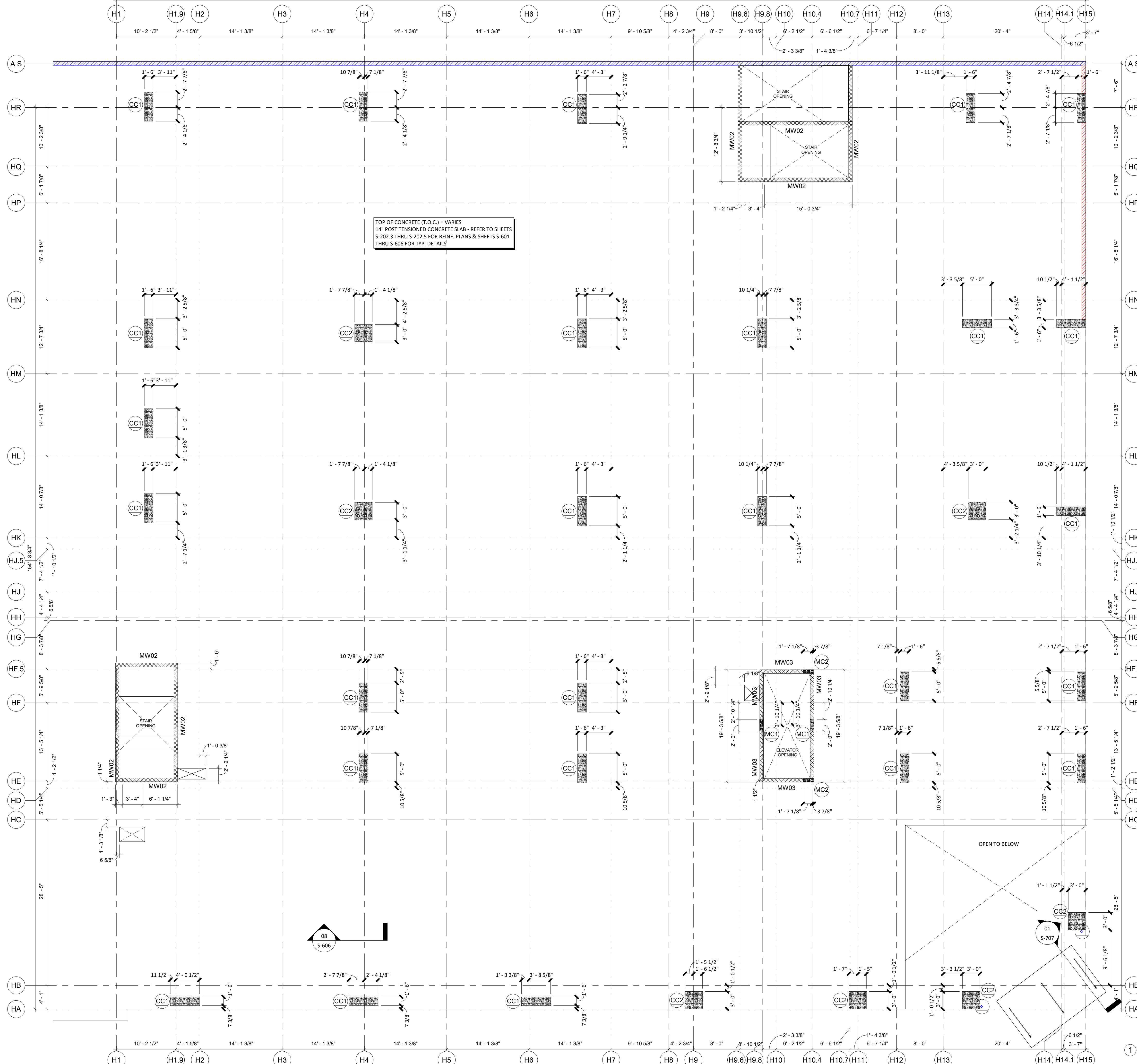
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REVISIONS:

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2ND FLOOR WALL AND COLUMN PLAN PART A

**S-202.6A**

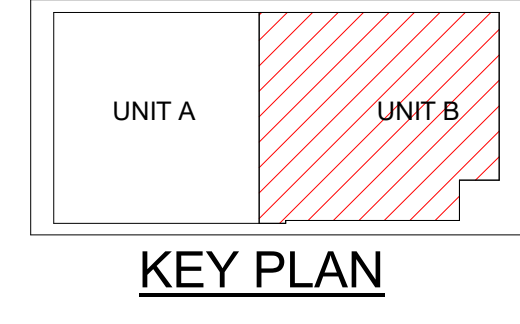
166' - 4 3/8"



TOP OF CONCRETE (T.O.C.) = VARIES  
 14" POST TENSIONED CONCRETE SLAB - REFER TO SHEETS  
 S-202.3 THRU S-202.5 FOR REIN. PLANS & SHEETS S-601  
 THRU S-606 FOR TYP. DETAILS

**WALL AND COLUMN NOTES**

1. COLUMNS & SHEARBEARING WALLS AT THIS PLAN ELEVATION FROM 12'-0" TO 23'-8"
2. SEE SCHEDULES ON S103 & S106 FOR COLUMN AND WALL TYPE
3. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
4. G.C. TO COORD. ALL OPENING LOCATIONS W/ ARCHITECTURAL DRAWINGS, M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONC



1 2ND FLOOR WALL AND COLUMN PLAN PART B  
 1/8" = 1'-0"

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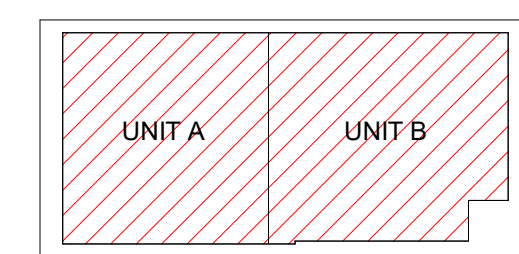
NEW 2019.06.14

2ND FLOOR WALL AND COLUMN PLAN  
 PART B

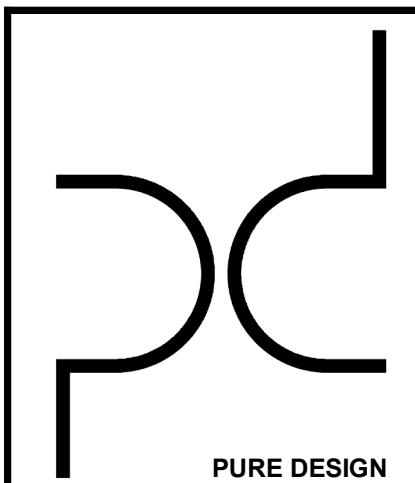
**S-202.6B**

**FLOOR FRAMING PLAN NOTES**

1. REFER TO ARCH. & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
2. ELEVATION OF THIRD FINISH FLOOR IS +25'-0" U.N.O.
3. G.C. TO COORD. W/ ARCH. FOR REQUIRED SLOPE OF CONG. TOPPING SLAB.



**KEY PLAN**



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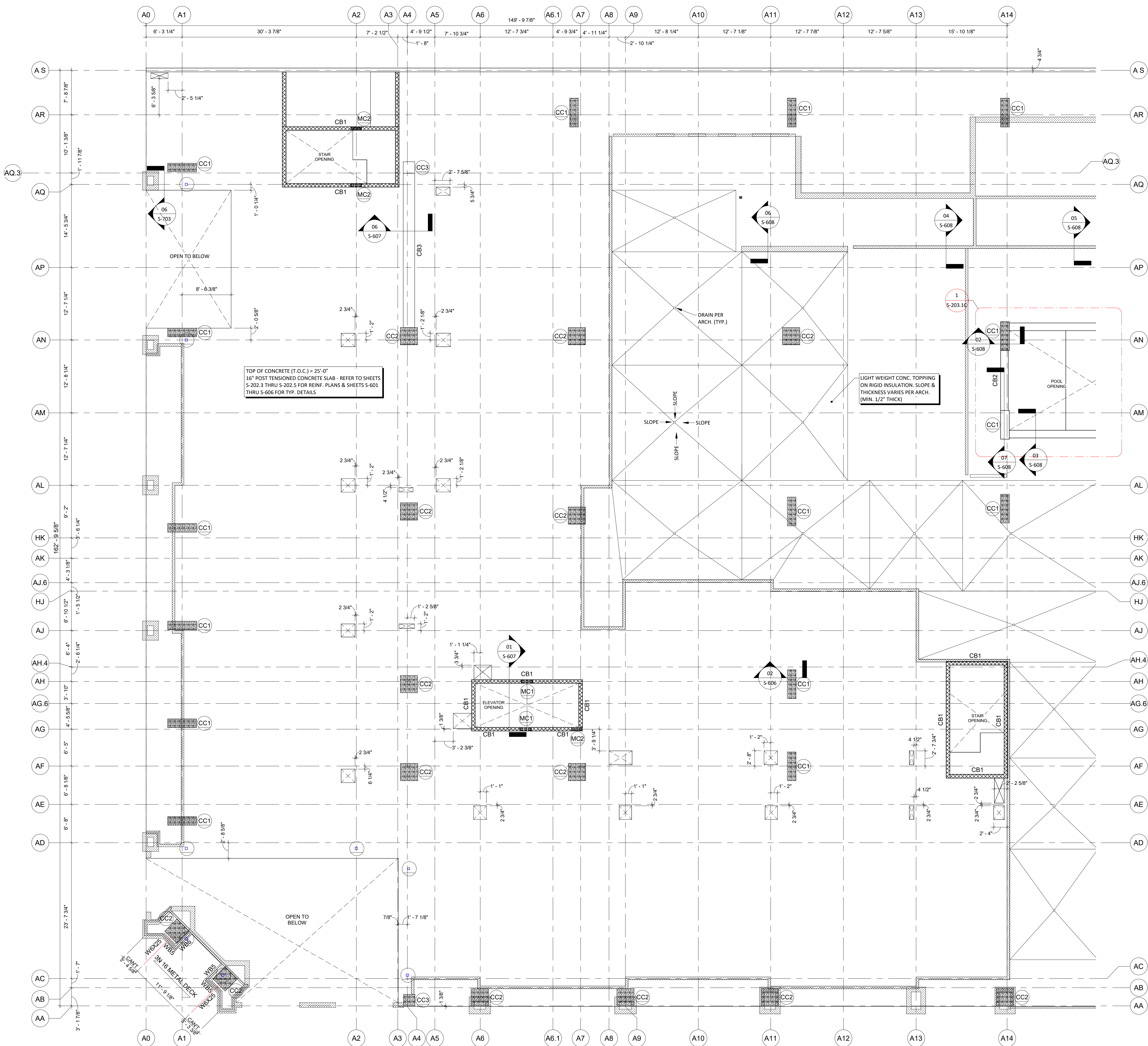
3RD FLOOR FRAMING PLAN

**S-203.1**



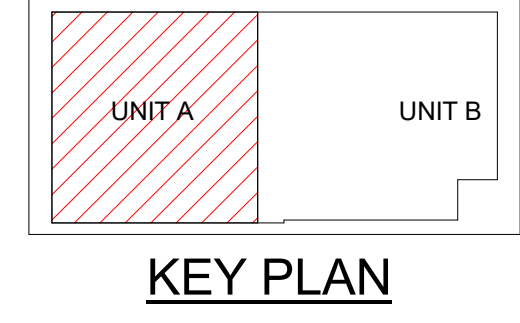
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① 3RD FLOOR FRAMING PLAN  
 1/16" = 1'-0"



TOP OF CONCRETE (T.O.C.) = 25'-0"  
 16" POST TENSIONED CONCRETE SLAB - REFER TO SHEETS  
 S-202.3 THRU S-202.5 FOR REINF. PLANS & SHEETS S-601  
 THRU S-606 FOR TYP. DETAILS

LIGHT WEIGHT CONC. TOPPING  
 ON RIGID INSULATION. SLOPE &  
 THICKNESS VARIES PER ARCH.  
 (MIN. 1/2" THICK)



- NOTES:**
- 1.) REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
  - 2.) ELEVATION OF THIRD FINISH FLOOR IS +25'-0" U.N.O.
  - 3.) G.C. TO COORD. W/ ARCH. FOR REQUIRED SLOPE OF CONC. TOPPING SLAB.
  - 4.) SEE DRAWING S-101 & S-102 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST.
  - 5.) REFERENCE DRAWING S-601 THRU S-606 FOR TYPICAL FRAMING DETAILS.
  - 6.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL WALL OPENINGS.
  - 7.) MASONRY CMU WALLS ARE 8" U.N.O. REFER PER SCHEDULE ON S-106
  - 8.) REFERENCE DRAWING S-609 & S-106 FOR TYPICAL MASONRY LINTEL DETAILS.
  - 9.) ALL EXTERIOR LINTELS SUPPORTING STONE OR BRICK SHALL BE GALVANIZED U.N.O. ON ARCHITECTURAL DRAWINGS.
  - 10.) G.C. TO COORD. ALL OPENING LOCATIONS W/ ARCHITECTURAL DRAWINGS, M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONC.

1) 3RD FLOOR FRAMING PLAN PART A  
 1/8" = 1'-0"

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 - AC & HIS

AC HOTELS

JOB NO: 18-007  
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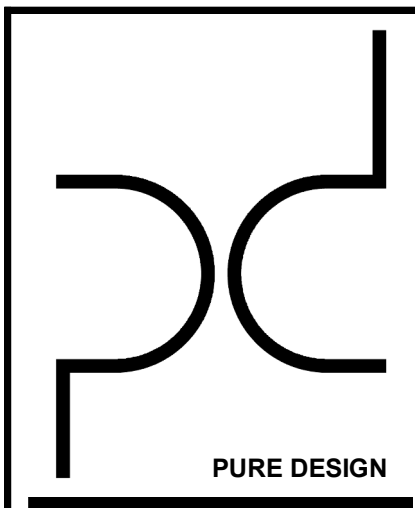
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3RD FLOOR FRAMING PLAN PART A

**S-203.1A**







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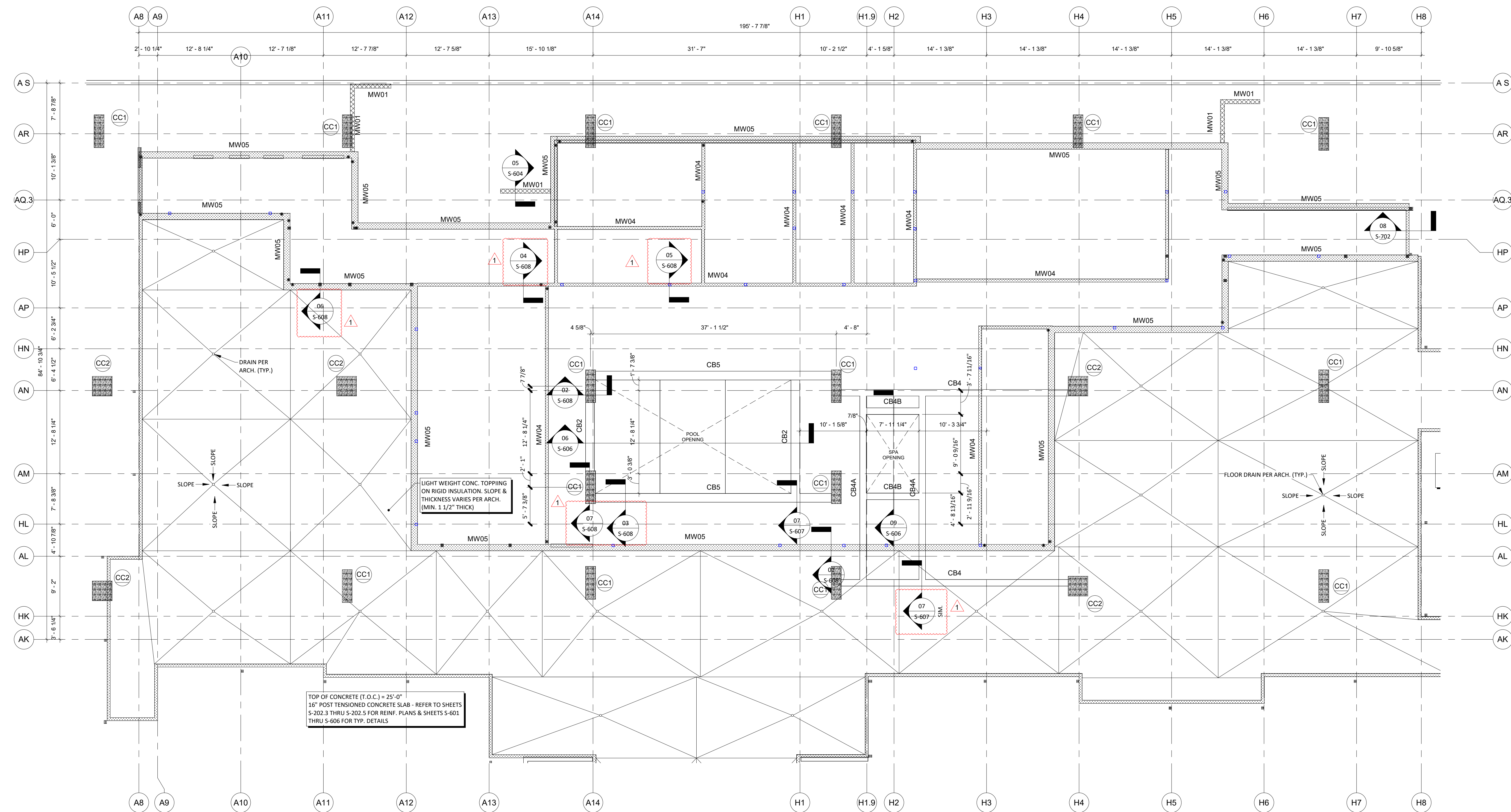


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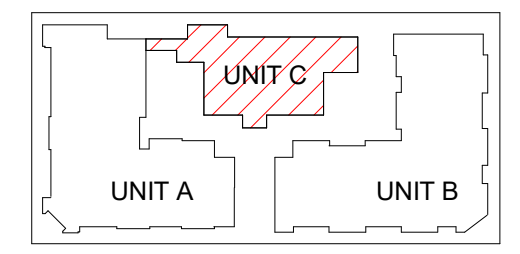
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3RD FLOOR FRAMING PLAN PART C

**S-203.1C**

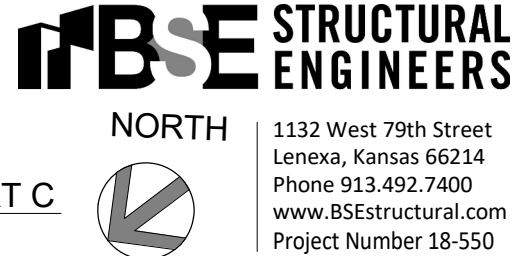


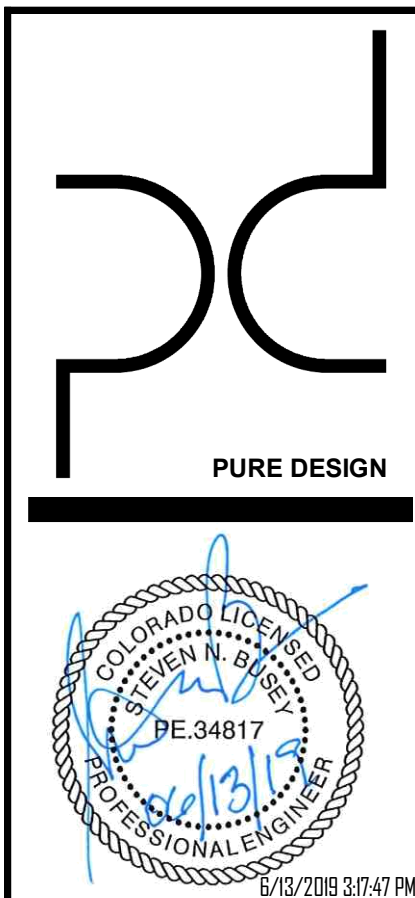
- NOTES:**
- 1.) REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
  - 2.) ELEVATION OF THIRD FINISH FLOOR IS +25'-0" U.N.O.
  - 3.) G.C. TO COORD. W/ ARCH. FOR REQUIRED SLOPE OF CONC. TOPPING SLAB.
  - 4.) SEE DRAWING S-101 & S-102 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST.
  - 5.) REFERENCE DRAWING S-601 THRU S-606 FOR TYPICAL FRAMING DETAILS.
  - 6.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL WALL OPENINGS.
  - 7.) REFER TO MASONRY WALL SCHEDULE FOR MASONRY WALL SIZES.
  - 8.) REFERENCE DRAWING S-609 & S-106 FOR TYPICAL MASONRY LINTEL DETAILS.
  - 9.) ALL EXTERIOR LINTELS SUPPORTING STONE OR BRICK SHALL BE GALVANIZED U.N.O. ON ARCHITECTURAL DRAWINGS.
  - 10.) G.C. TO COORD. ALL OPENING LOCATIONS W/ ARCHITECTURAL DRAWINGS, M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONC.
  - 11.) POOL DESIGN FOR MAXIMUM GUNITE WALL AND FLOOR THICKNESS OF 3" AND 6", RESPECTIVELY. MINIMUM 12" OF 3/4" CLEAN GRAVEL BELOW POOL SLAB AS REQUIRE PER POOL SUPPLIER



KEY PLAN

1 3RD FLOOR FRAMING PLAN PART C  
1/8" = 1'-0"





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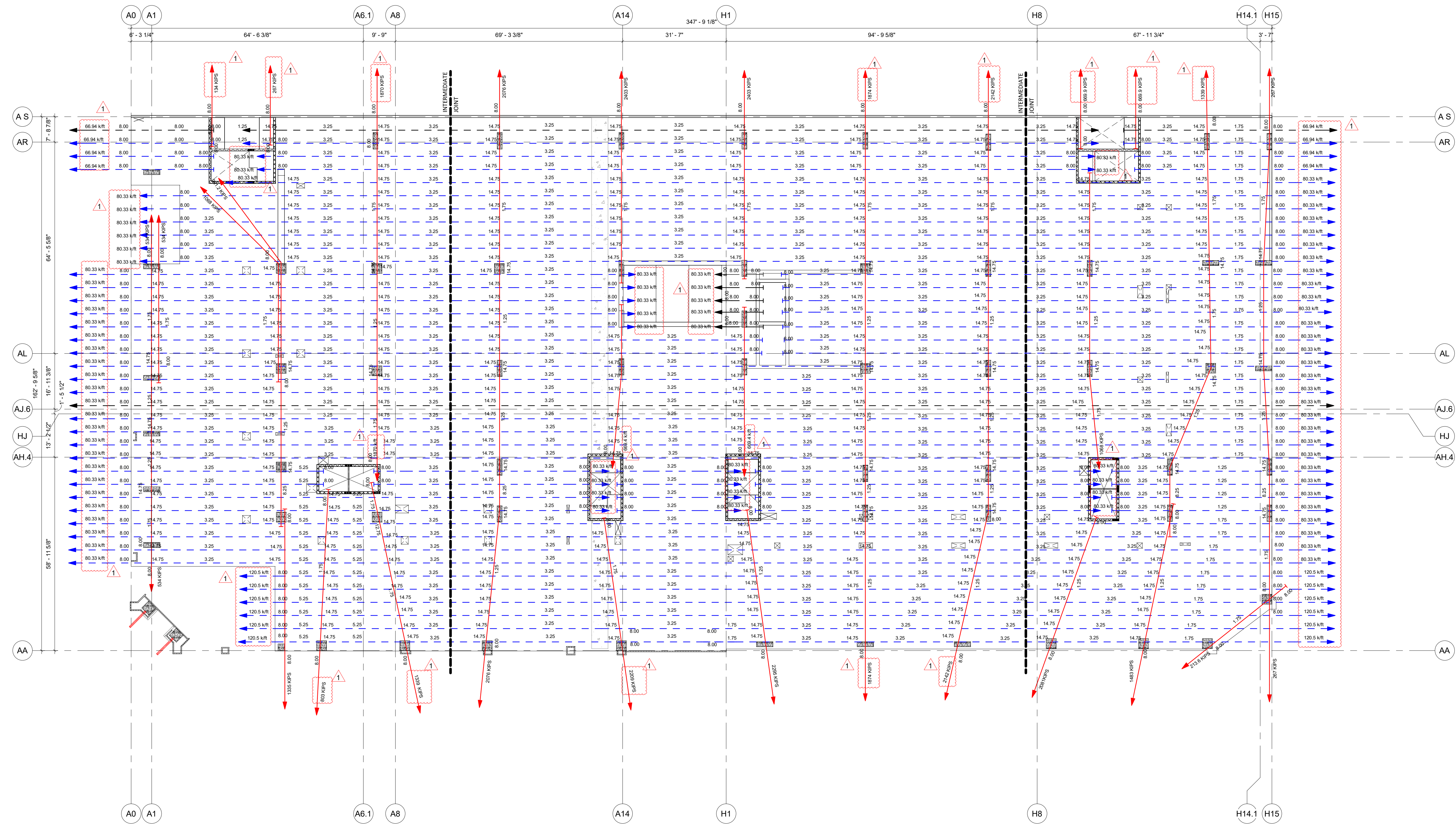
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 - AC & HIS

AC HOTELS

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 1 05/17/2019

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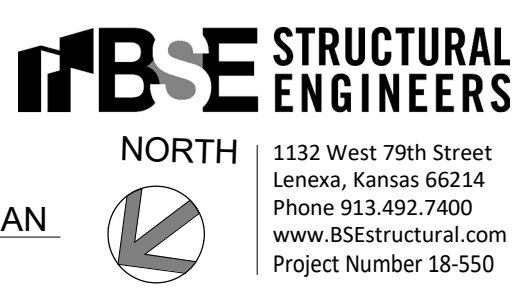
3RD FLOOR TENDON PLAN  
**S-203.2**



- NOTES:
1. THE NUMBERS ON TENDONS ARE THE TENDON HEIGHT FROM CENTROID OF TENDON TO SLAB BOTTOM. THE UNIT IS INCHES.
  2. FOR ALL COLUMNS, PLEASE SEE THE SCHEDULES.
  3. SEE POST-TENSION NOTES ON SHEET S601.
  4. THE FORCES SHOWN ON ALL PT PLAN ARE EFFECTIVE FORCES.
  5. THE THICKNESS OF POST-TENSIONED SLAB FOR THIS FLOOR IS 14"
  6. CONCRETE GROUT STRENGTH SHALL ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS U.N.O.
  7. SPACING 1 =4'-0"  
 SPACING 2 =2'-0"
  8. CABLES HEIGHT IS RELATIVE HEIGHT, RELATIVE TO THE BOTTOM OF ITS SLAB
  9. TENDON FORCES SUCCETIBLE TO CHANGE
  - 10.) TENDON LAYOUTS AND STRESSING SEQUENCE BY OTHERS. TENDONS ARE TO BE DEVELOP THE EFFECTIVE FORCES SHOWN AFTER ALL LOSSES.
  - 11.) ALL ANCHORAGE DEVICES ARE BY OTHERS & ARE TO BE INSTALLED PER THE MANUFACTURER.
  - 12.) G.C. TO COORD. ALL OPENING LOCATIONS & DIMENSIONS W/ ARCHITECTURAL & M.E.P. DRAWINGS & EQUIPMENT MANUF. (AS REQ'D) PRIOR TO PLACEMENT OF PT TENDONS & CONG.
  - 13.) ALL WOOD IN CONTACT W/ SLAB SHALL BE TREATED & ALL SILL PL. ANCHORS EMBEDDED PER DETAILS.
  - 14.) SHORING BY OTHERS.

LEGENDS	
MAIN TENDON	
DISTRIBUTION TENDON	
POST POUR BAND	
INTERMEDIATE JOINT	

② 3RD FLOOR TENDON PLAN  
 1/16" = 1'-0"



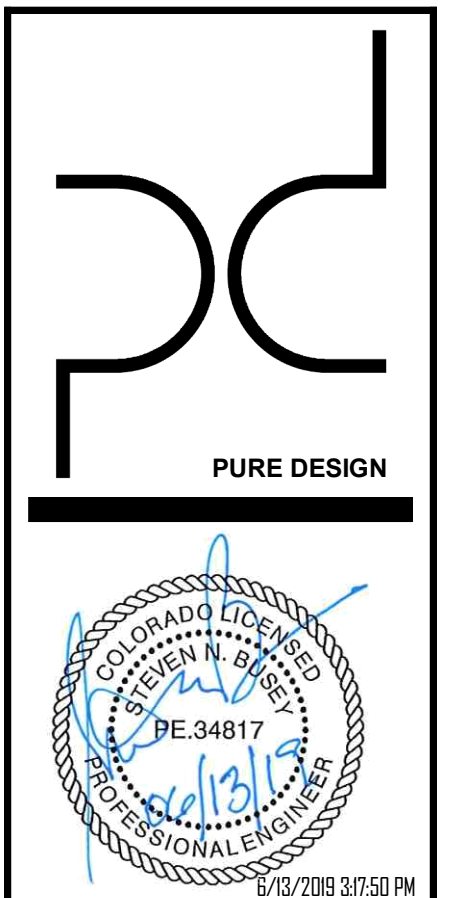
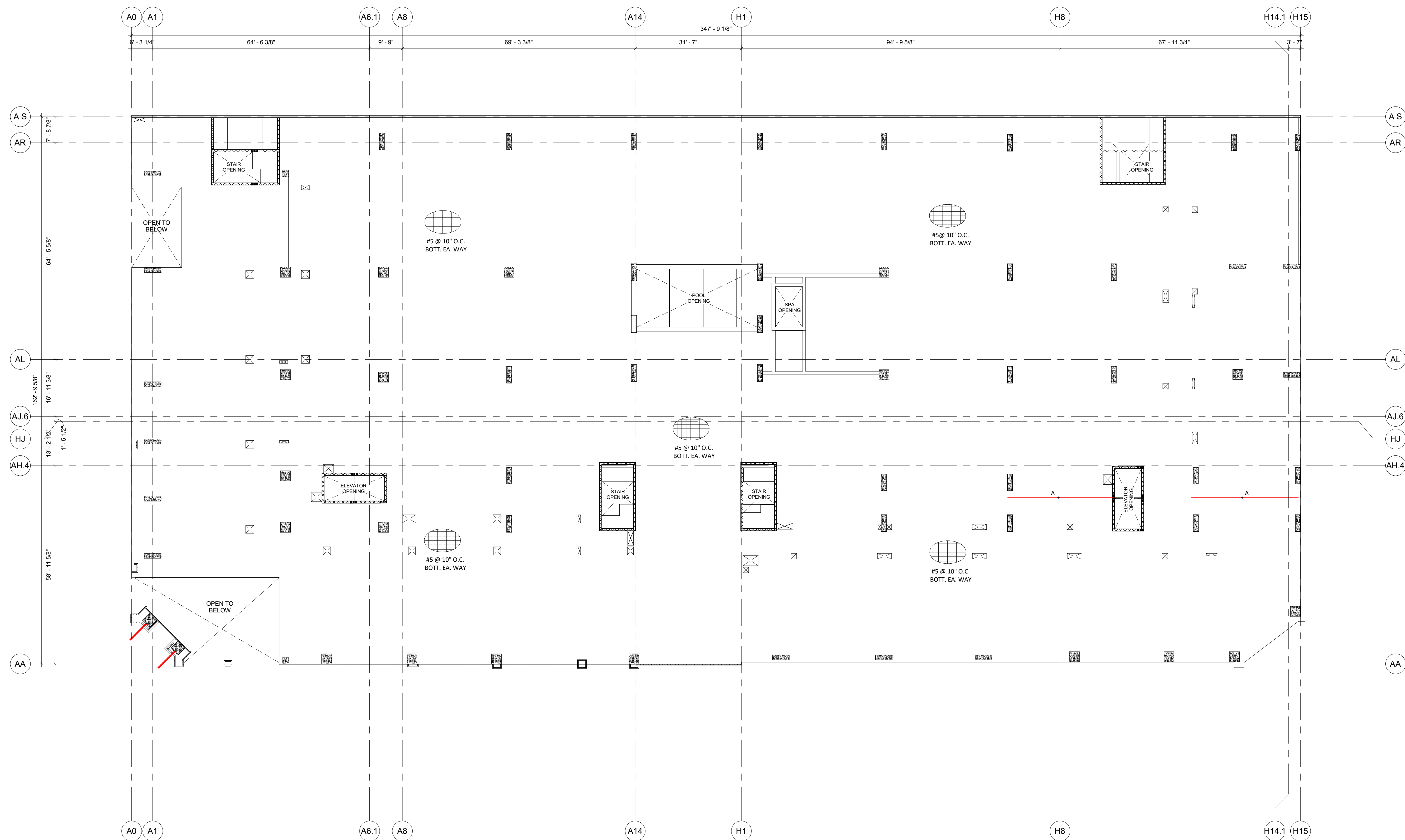
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**LEGENDS OF BARS**

BAR MARKED	DIA. & SPACING
A	#5 @ 12" O.C.

**REBAR LAYOUT NOTES**

1. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
2. ELEVATION OF THIRD FLOOR LEVEL FINISH IS +25'-0" U.N.O.
3. CONCRETE SLAB THICKNESS IS 16" U.N.O.



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**DUAL HOTEL DURANGO**  
 - AC & HIS

AC HOTELS

JOB NO: 18-007  
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 1 05/17/2019

NEW 2019.06.14

3RD FLOOR BOTTOM REBAR LAYOUT PLAN  
**S-203.3**

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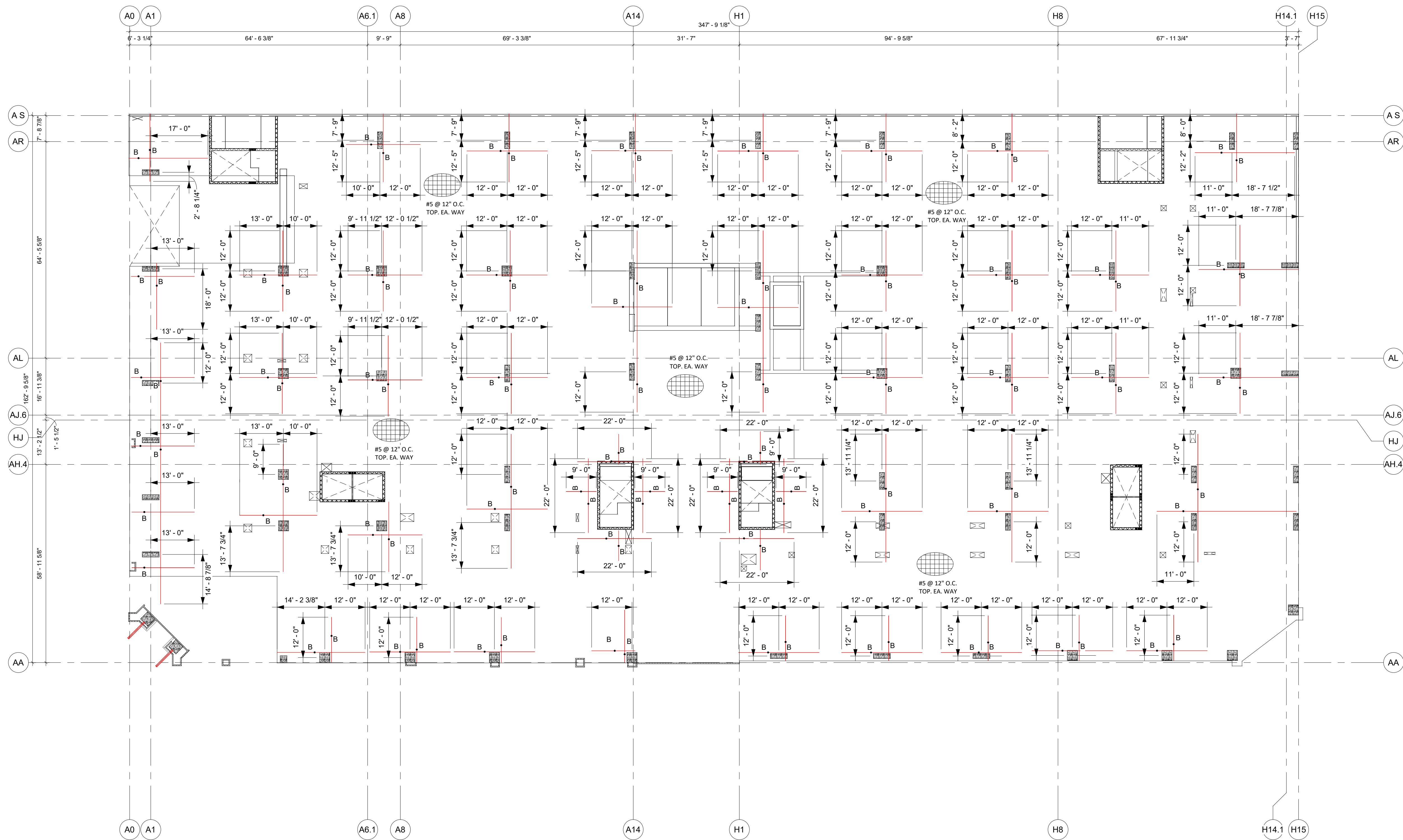
1 3RD FLOOR BOTTOM REBAR LAYOUT PLAN  
 1/16" = 1'-0"

**LEGENDS OF BARS**

BAR MARKED	DIA. & SPACING
A	#5 @ 12" O.C.
B	#6 @ 12" O.C.

**REBAR LAYOUT NOTES**

1. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
2. ELEVATION OF THIRD FLOOR LEVEL FINISH IS +25'-0" U.N.O.
3. CONCRETE SLAB THICKNESS IS 16" U.N.O.



**3RD FLOOR ADDITIONAL TOP REBAR LAYOUT PLAN**  
1/16" = 1'-0"

**BS STRUCTURAL ENGINEERS**  
NORTH  
1132 West 79th Street  
Lenexa, Kansas 66214  
Phone 913.492.7400  
www.BSEStructural.com  
Project Number 18-550

**PURE DESIGN**

**ARCHITECT**  
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KANSAS CITY, MO 64115  
PHONE: 316.617.2052

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LENEXA, KS 66214  
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**MEP ENGINEERS**  
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PHONE: 913.492.2400

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ABERDEEN, SD 57401  
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**DUAL HOTEL DURANGO - AC & HIS**

**AC HOTELS**

JOB NO: 18-007  
DATE: 2018.03.07  
ISSUE RECORD:  
REVISIONS:  
1 05/17/2019

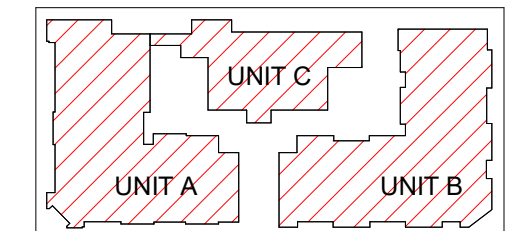
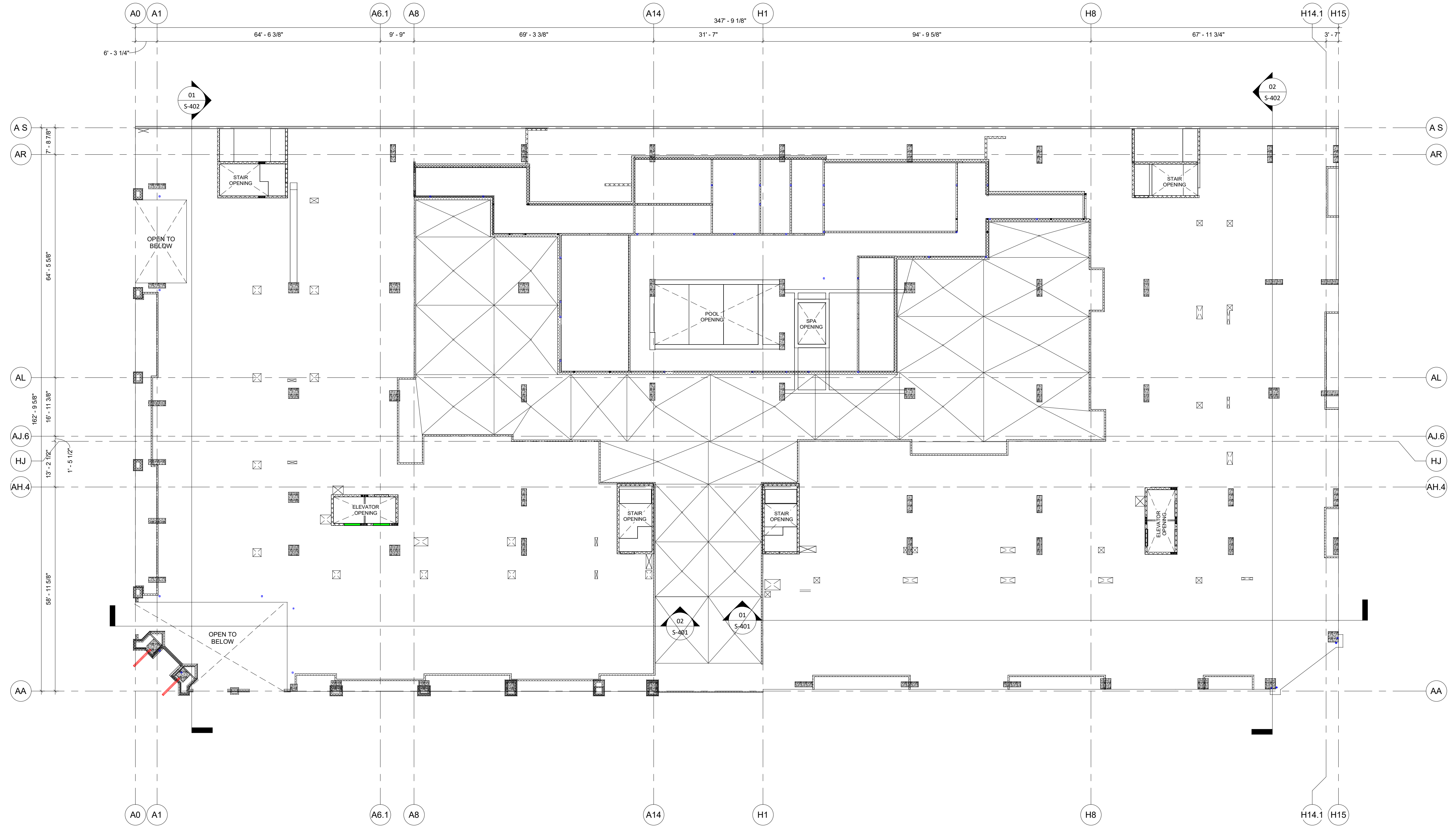
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3RD FLOOR ADDITIONAL TOP REBAR LAYOUT PLAN

**S-203.4**

**WALL AND COLUMN NOTES**

1. SHEAR/BEAR WALL, WHICH ARE BEARING TRUSSES AT THIS PLAN ELEVATED FROM +25'-0" - +33'-4 1/4" U.N.O.
2. COLUMN AT THIS PLAN ELEVATED FROM +25'-0" - +34'-1 1/4" U.N.O.
3. SEE SCHEDULES ON S105 FOR COLUMN AND WALL TYPE.
4. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
5. REFERENCE S-208 & S-209 FOR HOLD-DOWN LOCATIONS.



**KEY PLAN**

**BSE STRUCTURAL ENGINEERS**  
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1 3RD FLOOR WALL AND COLUMN PLAN  
 1/16" = 1'-0"

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**MEP ENGINEERS**  
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**DUAL HOTEL DURANGO**  
 - AC & HIS

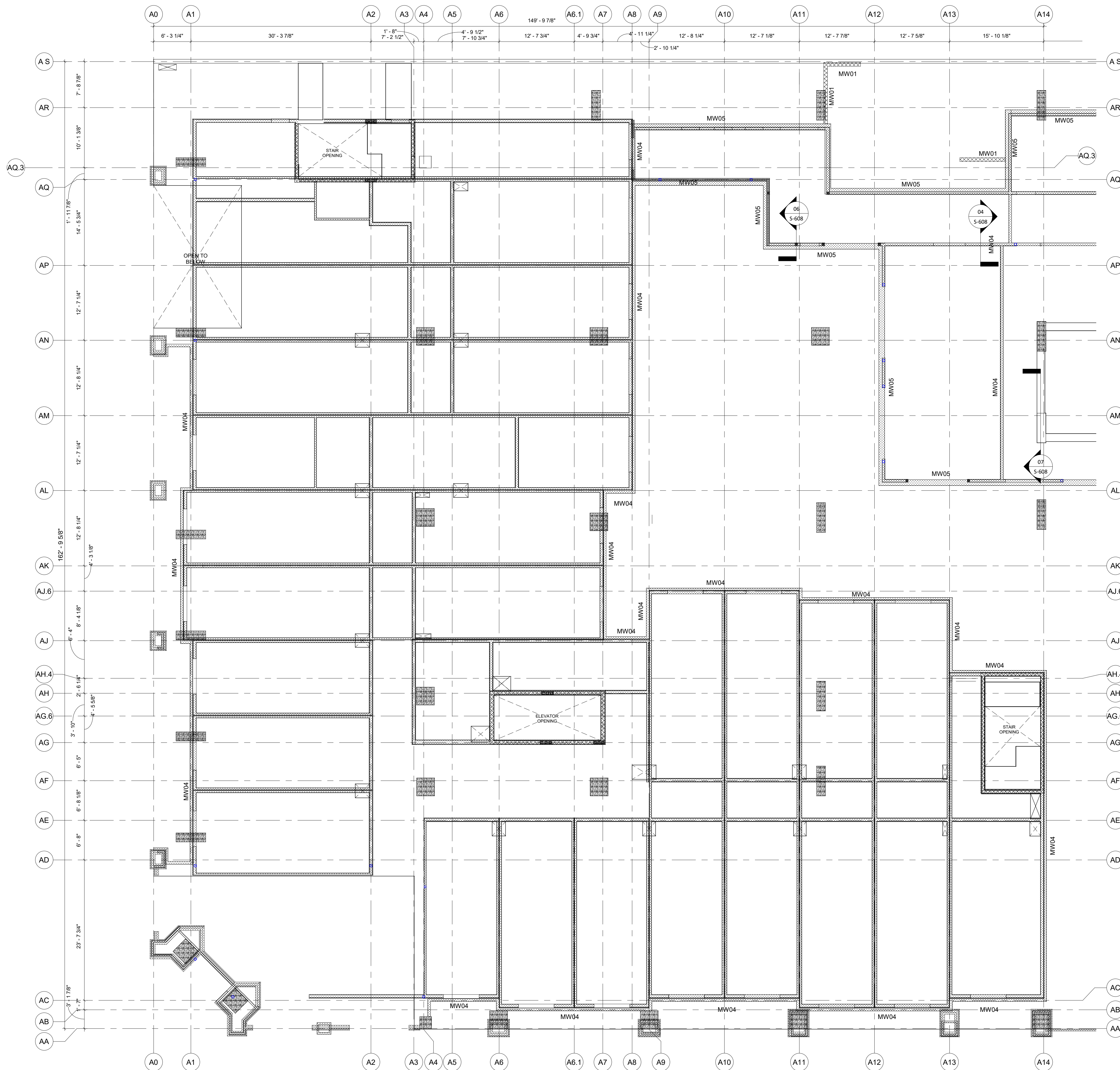
AC HOTELS

JOB NO:	18-007
DATE:	2018.03.07
ISSUE RECORD:	
REVISIONS:	
1	05/17/2019

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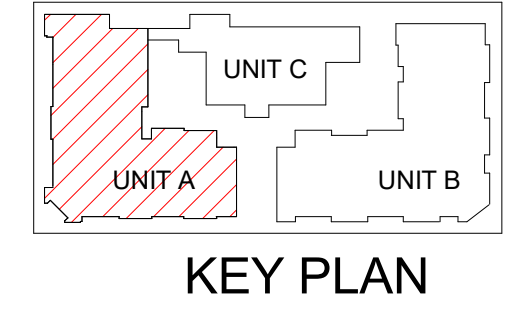
3RD FLOOR WALL AND COLUMN PLAN

**S-203.5**



**WALL AND COLUMN NOTES**

1. SHEAR/BEAR WALL, WHICH ARE BEARING TRUSSES AT THIS PLAN ELEVATED FROM +25'-0" - +33'-10 1/4" U.N.O.
2. COLUMN AT THIS PLAN ELEVATED FROM +25'-0" - +34'-7 1/4" U.N.O.
3. SEE SCHEDULES ON S105 FOR COLUMN AND WALL TYPE.
4. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
5. REFERENCE S208 FOR HOLD-DOWN LOCATIONS.



**ABSE STRUCTURAL ENGINEERS**  
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1 3RD FLOOR WALL AND COLUMN PLAN PART A  
 1/8" = 1'-0"



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**ARCHITECT**  
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**MEP ENGINEERS**  
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**DUAL HOTEL DURANGO**  
**- AC & HIS**

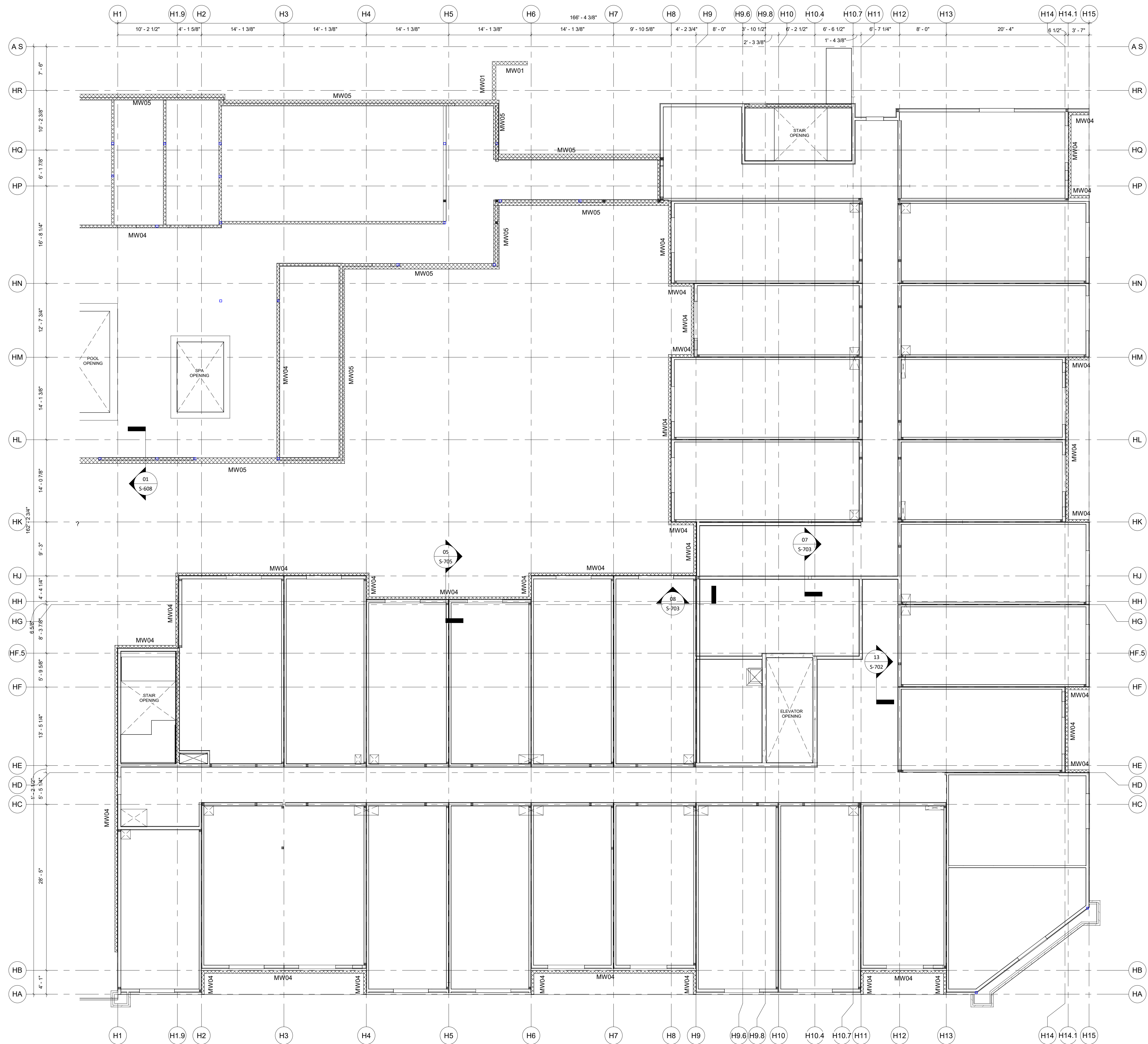


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 1 05/17/2019

**NEW 2019.06.14**

3RD FLOOR WALL AND COLUMN PLAN PART A

**S-203.5A**



- WALL AND COLUMN NOTES**
1. SHEAR/BEAR WALL, WHICH ARE BEARING TRUSSES AT THIS PLAN ELEVATED FROM +25'-0" ~ +33'-4 1/4" U.N.O.
  2. COLUMN AT THIS PLAN ELEVATED FROM +25'-0" ~ +34'-1 1/4" U.N.O.
  3. SEE SCHEDULES ON S105 FOR COLUMN AND WALL TYPE.
  4. REFER TO ARCH & MEP DWGS FOR OPENING DIMENSIONS ON THIS FLOOR.
  5. REFERENCE S-208 & S-209 FOR HOLD-DOWN LOCATIONS.

**PURE DESIGN**

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 PHONE: 913.492.7400

**MEP ENGINEERS**  
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 13300 W. 98TH ST.  
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 311 4th Ave South  
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 1-320-259-6557

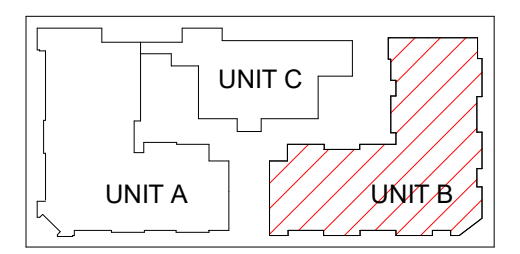
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**DUAL HOTEL DURANGO**  
 - AC & HIS

AC HOTELS

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KEY PLAN

**BSE STRUCTURAL ENGINEERS**

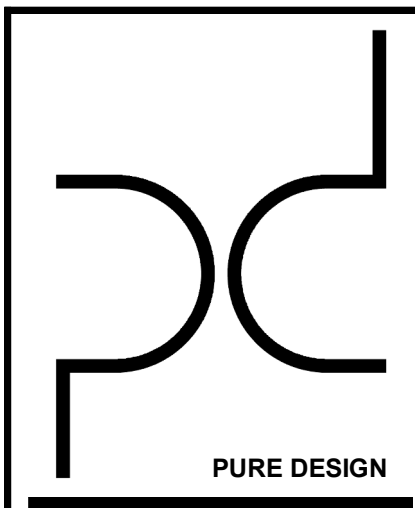
NORTH

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 Project Number 18-550

1 3RD FLOOR WALL AND COLUMN PLAN PART B  
 1/8" = 1'-0"

3RD FLOOR WALL AND COLUMN PLAN PART B

**S-203.5B**



**ARCHITECT**  
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**DUAL HOTEL DURANGO**  
 - AC & HIS



JOB NO: 18-007  
 DATE: 2018.03.07

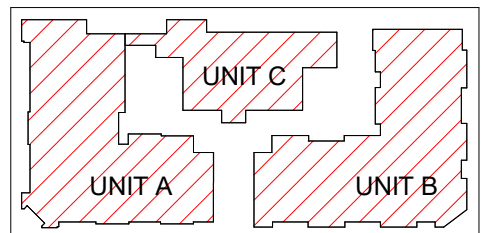
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REVISIONS:

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3RD FLOOR MODULAR FRAMING PLAN

**S-203.5C**



KEY PLAN

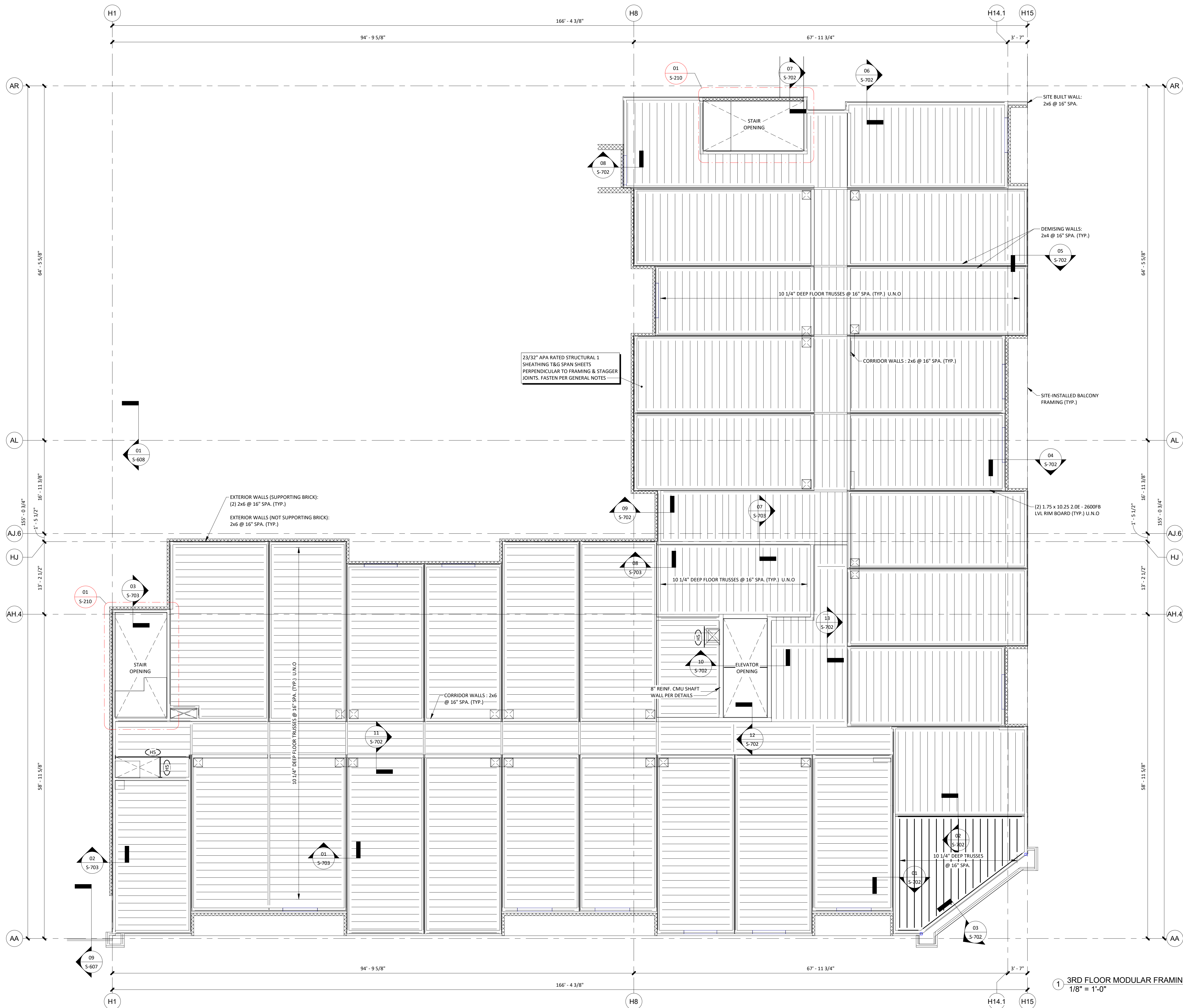
- NOTES:
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-203.5F FOR 3RD LEVEL CEILING FRAMING PLAN
  - 6.) ○ = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE

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 Project Number 18-550

① 3RD FLOOR MODULAR FRAMING PLAN  
 1/16" = 1'-0"







23/32\"/>

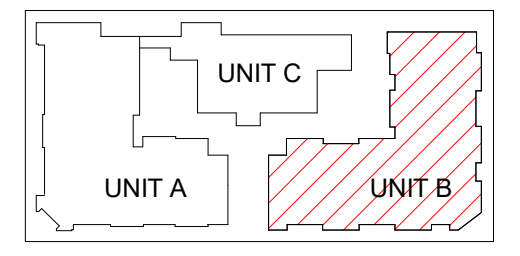
EXTERIOR WALLS (SUPPORTING BRICK):  
 (2) 2x6 @ 16\"/>

EXTERIOR WALLS (NOT SUPPORTING BRICK):  
 2x6 @ 16\"/>

CORRIDOR WALLS: 2x6 @ 16\"/>

8\"/>

10 1/4\"/>



KEY PLAN

- NOTES:
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-203.5H FOR 3RD LEVEL CEILING FRAMING PLAN
  - 6.) ○ = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE

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1 3RD FLOOR MODULAR FRAMING PLAN PART B  
 1/8\"/>

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**STRUCTURAL ENGINEER**  
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 PHONE: 913.492.2400

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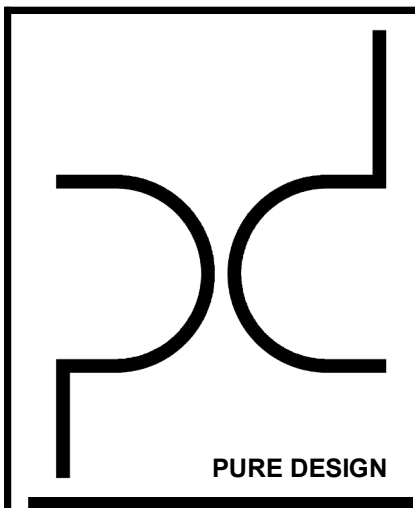
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3RD FLOOR MODULAR FRAMING PLAN PART B

**S-203.5E**



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**DUAL HOTEL DURANGO**  
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 DATE: 2018.03.07

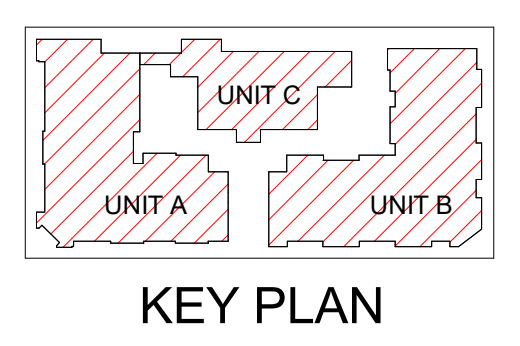
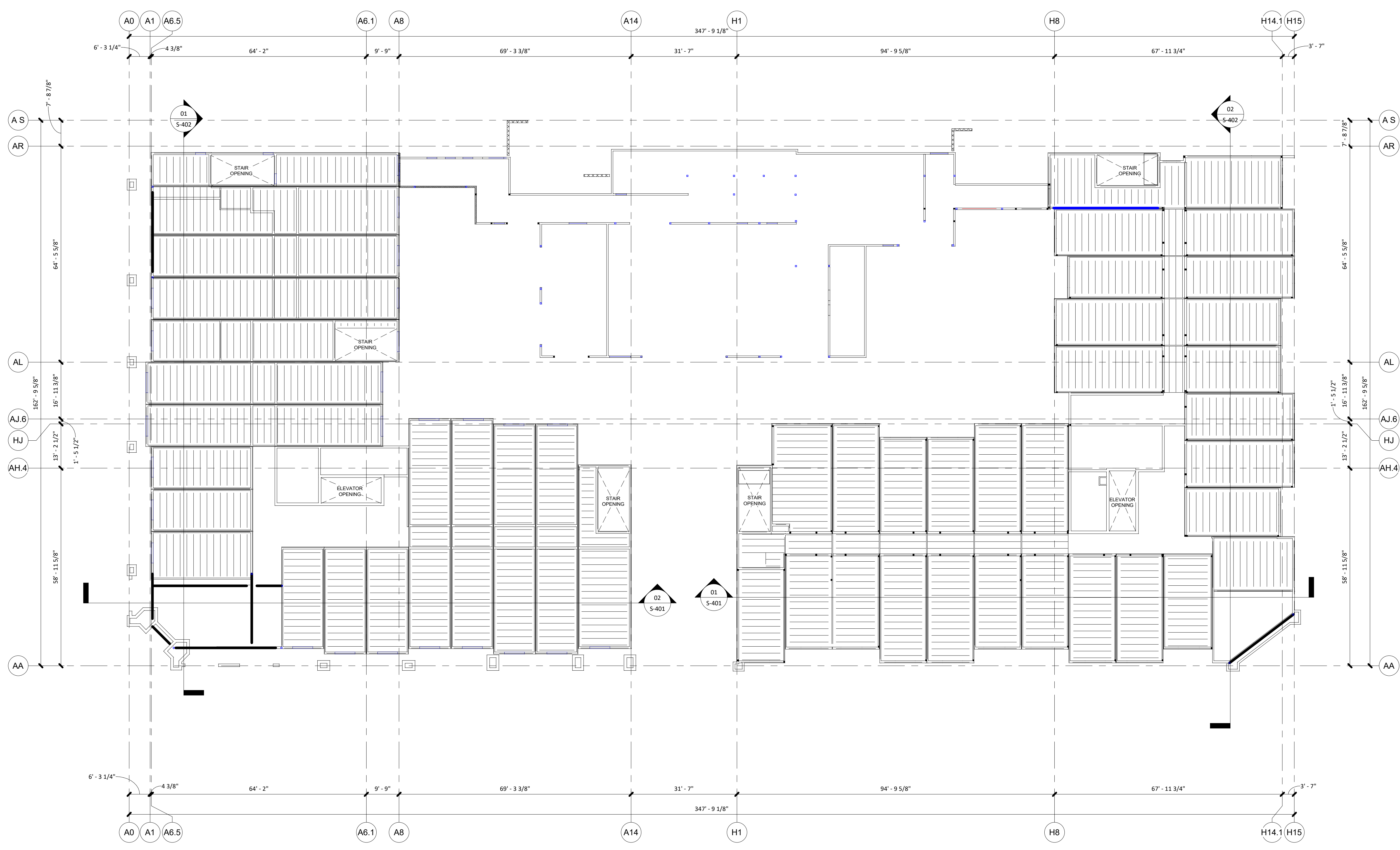
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REVISIONS:

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3RD FLOOR CEILING FRAMING PLAN

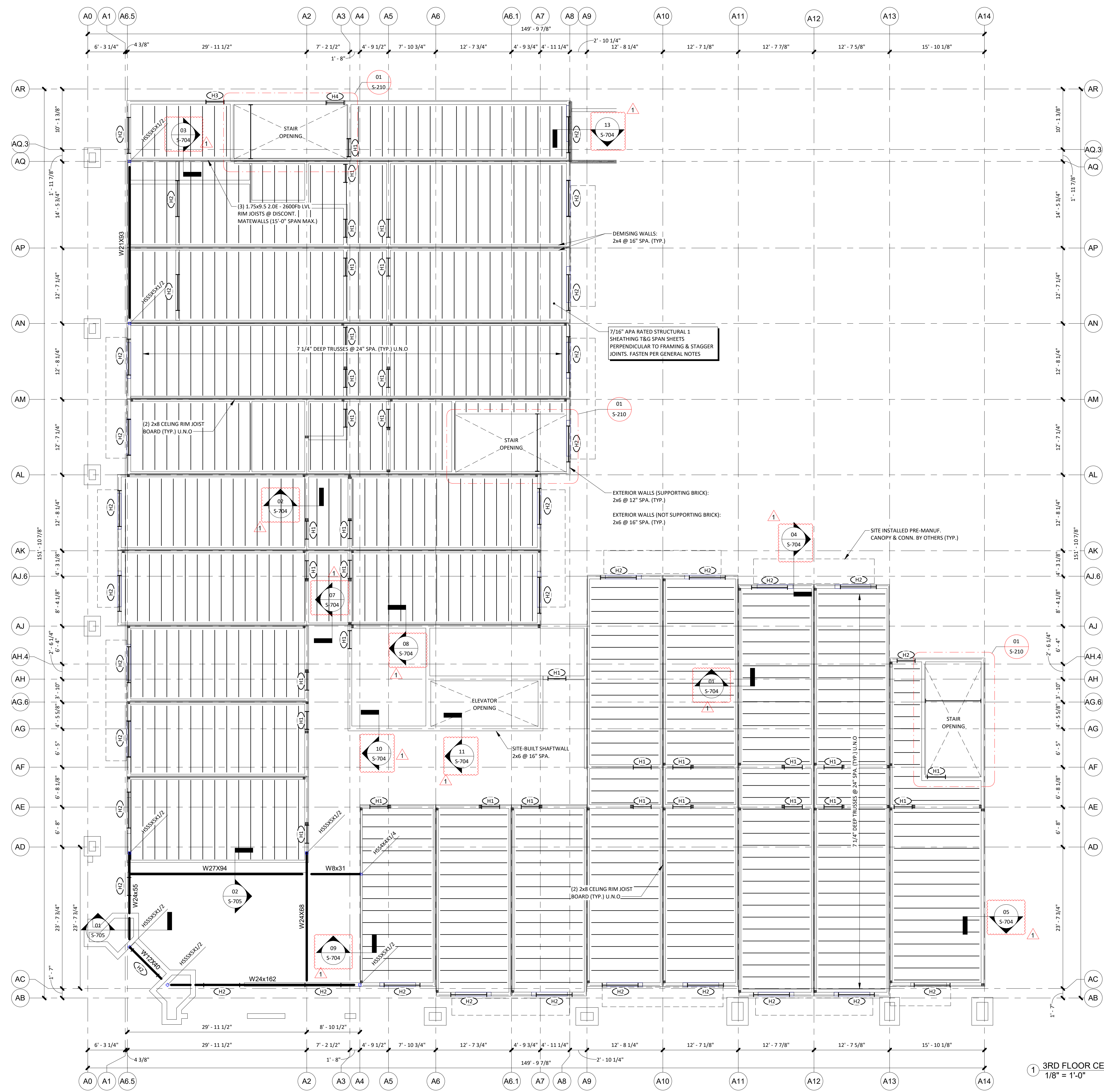
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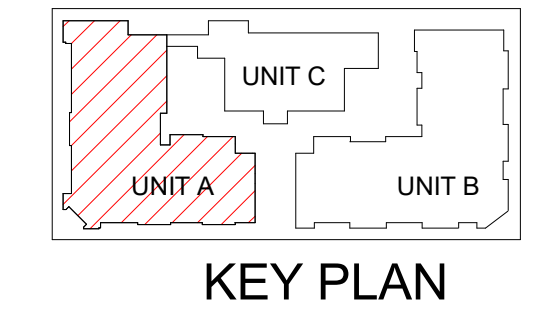
- NOTES:**
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-203.SC FOR 3RD LEVEL FLOOR FRAMING PLAN

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1 3RD FLOOR CEILING FRAMING PLAN  
 1/16" = 1'-0"



- NOTES:
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-203.5D FOR 3RD LEVEL FLOOR FRAMING PLAN
  - 6.)  $\bigcirc$  = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



1 3RD FLOOR CEILING FRAMING PLAN PART A  
1/8" = 1'-0"

**BSE STRUCTURAL ENGINEERS**  
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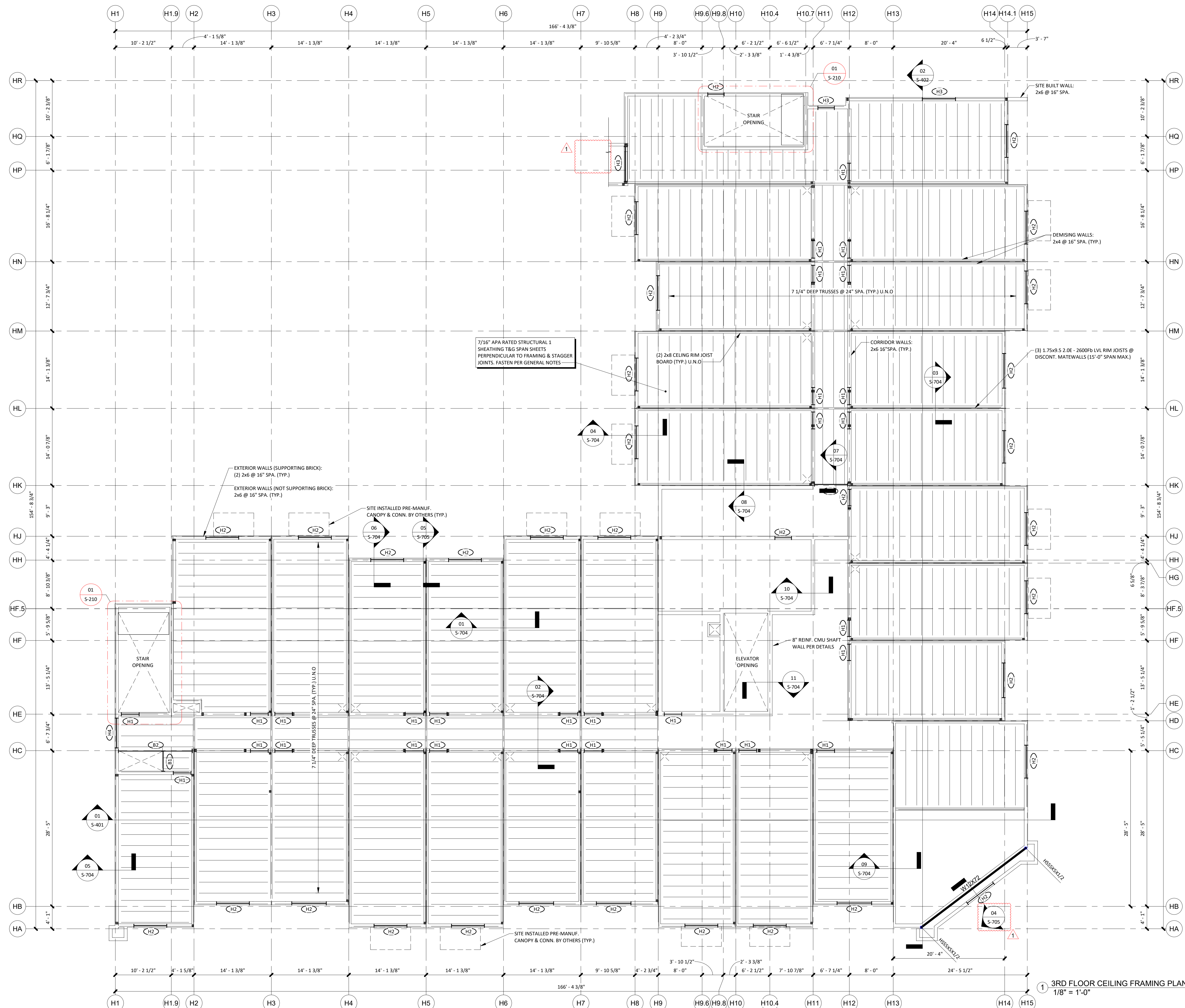
AC HOTELS

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 DATE: 2018.03.07  
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3RD FLOOR CEILING FRAMING PLAN PART A

**S-203.5G**



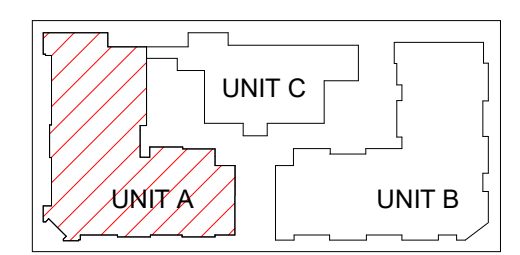
7/16" APA RATED STRUCTURAL 1 SHEATHING T&G SPAN SHEETS PERPENDICULAR TO FRAMING & STAGGER JOINTS. FASTEN PER GENERAL NOTES

EXTERIOR WALLS (SUPPORTING BRICK): (2) 2x6 @ 16" SPA. (TYP.)  
EXTERIOR WALLS (NOT SUPPORTING BRICK): 2x6 @ 16" SPA. (TYP.)

SITE INSTALLED PRE-MANUF. CANOPY & CONN. BY OTHERS (TYP.)

SITE INSTALLED PRE-MANUF. CANOPY & CONN. BY OTHERS (TYP.)

- NOTES:
- 1) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEDGEN, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5) REFERENCE DRAWINGS S-203.5E FOR GROUND LEVEL FLOOR FRAMING PLAN
  - 6) ○ = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



KEY PLAN

**BSE STRUCTURAL ENGINEERS**  
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1 3RD FLOOR CEILING FRAMING PLAN PART B  
1/8" = 1'-0"

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- AC & HIS

AC HOTELS

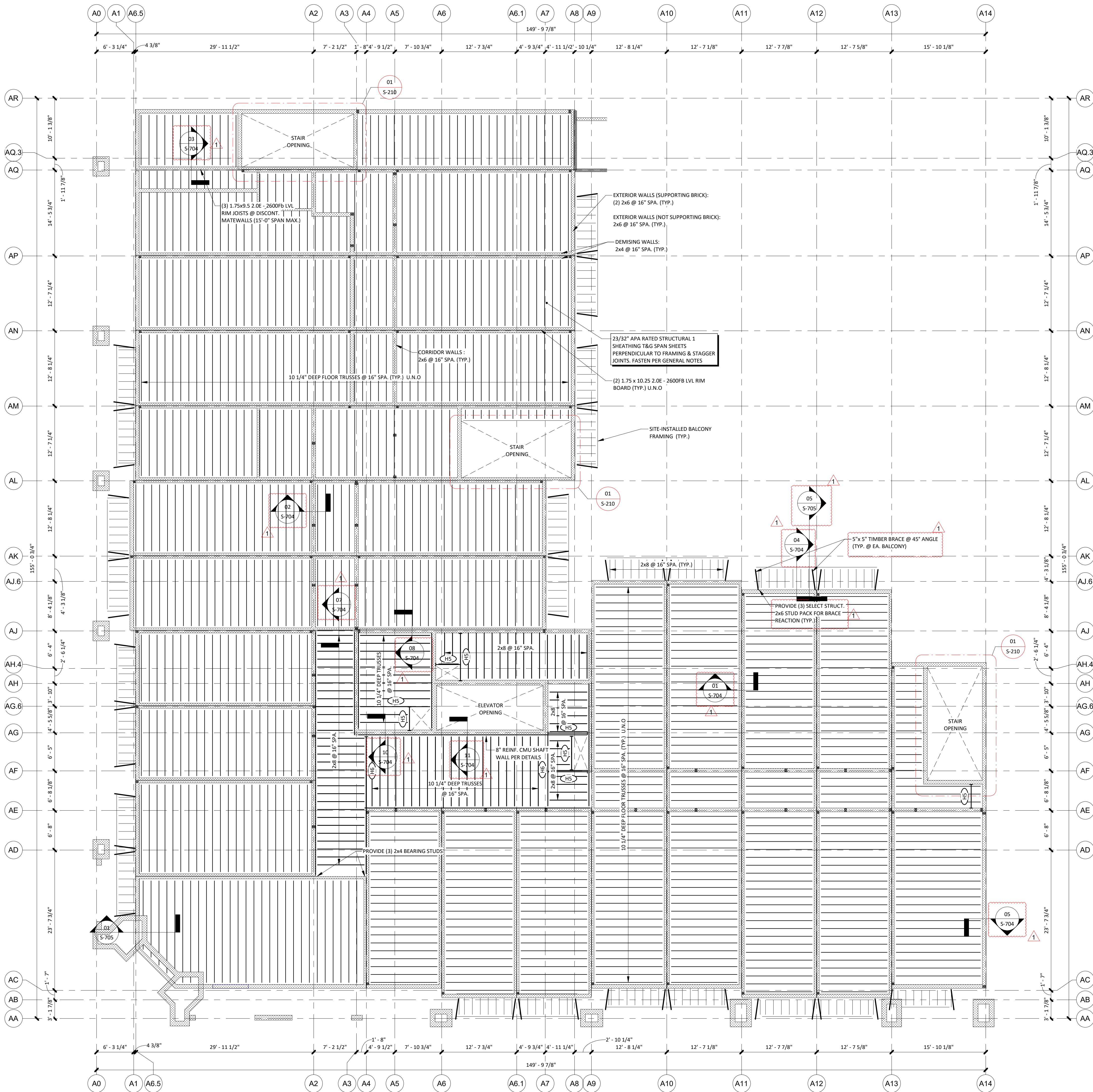
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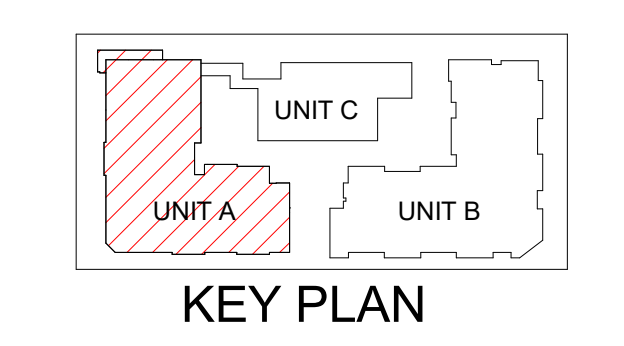
3RD FLOOR CEILING FRAMING PLAN PART B

S-203.5H



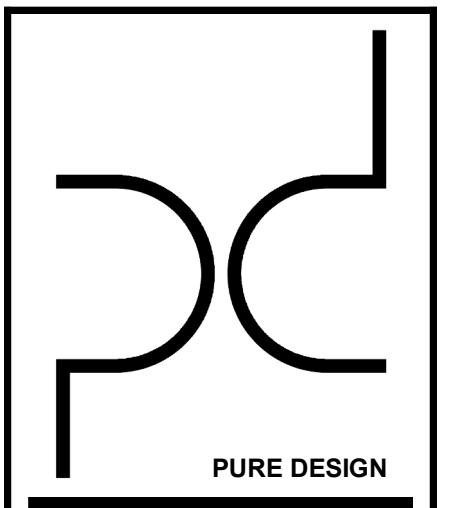


- NOTES:**
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEDGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-204.2A FOR 4TH LEVEL CEILING FRAMING PLAN
  - 6.) DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



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① 4TH FLOOR FRAMING PLAN PART A  
 1/8" = 1'-0"



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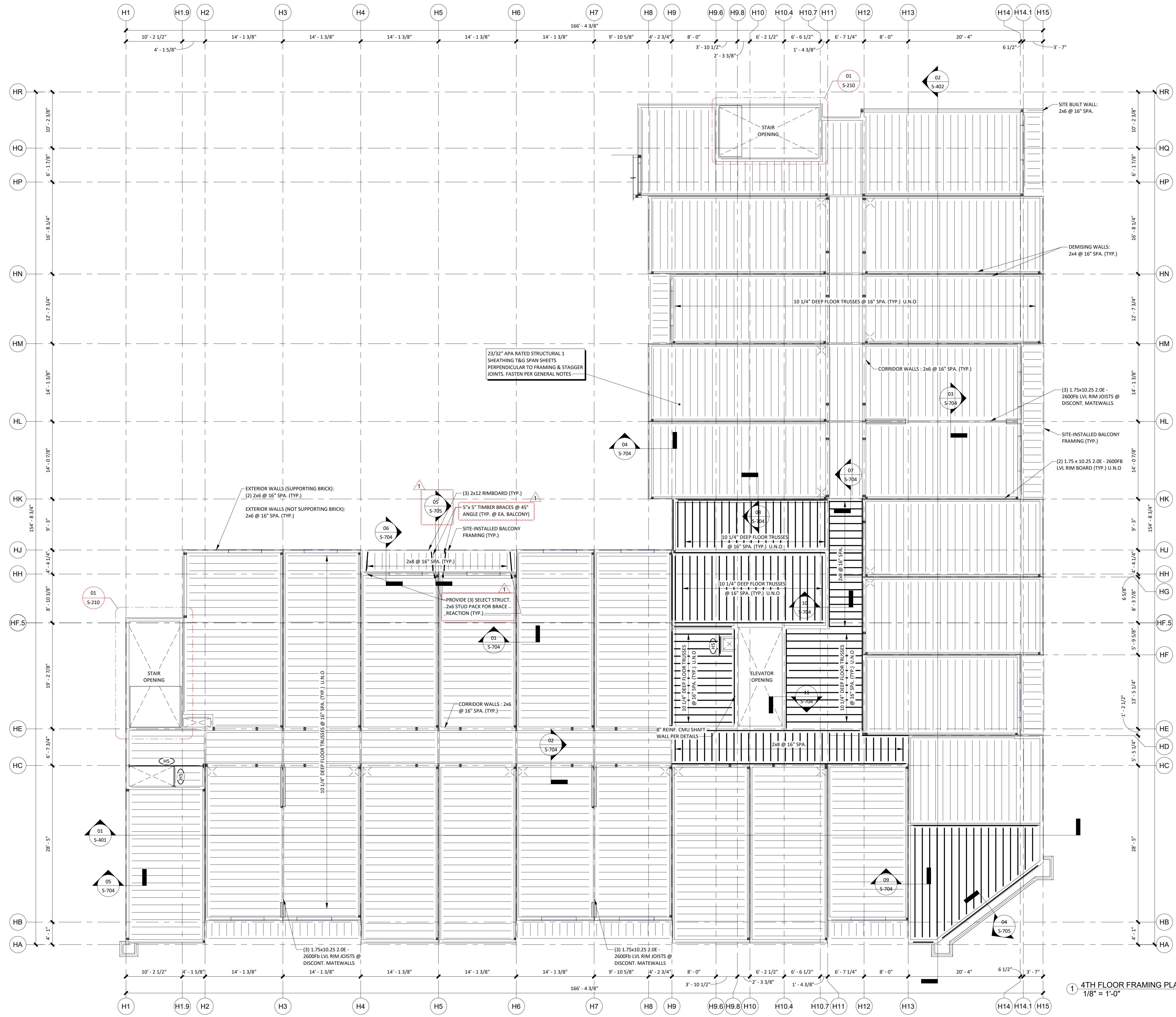
AC HOTELS

JOB NO: 18-007  
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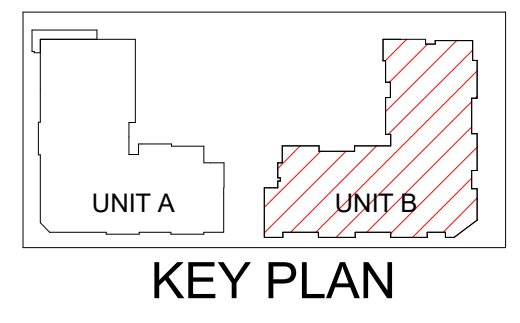
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4TH FLOOR FRAMING PLAN PART A

**S-204.1A**



- NOTES:
- 1) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEDGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5) REFERENCE DRAWINGS S-204.2B FOR 4TH LEVEL CEILING FRAMING PLAN
  - 6) ○ = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



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1 4TH FLOOR FRAMING PLAN PART B  
 1/8" = 1'-0"

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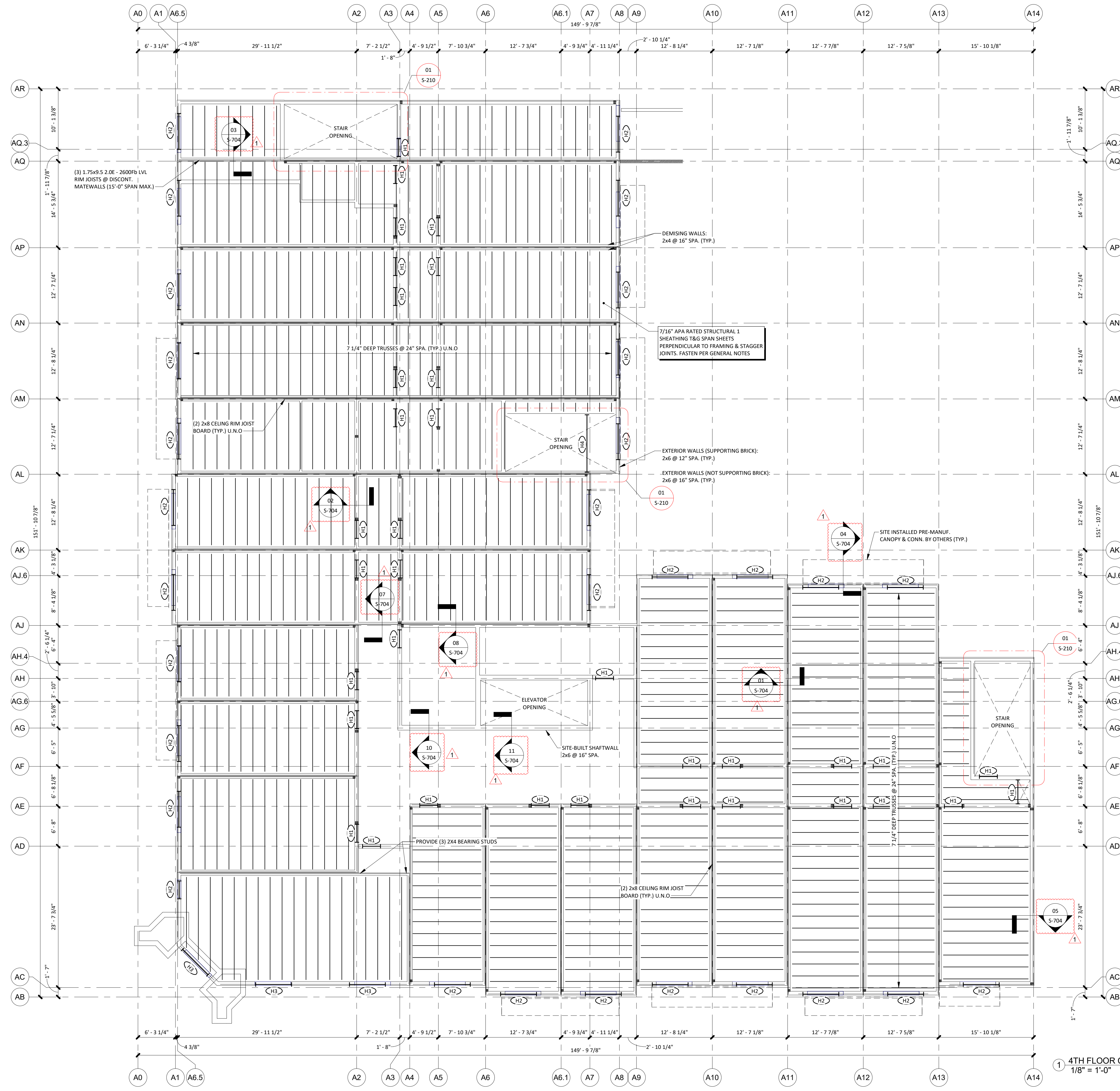
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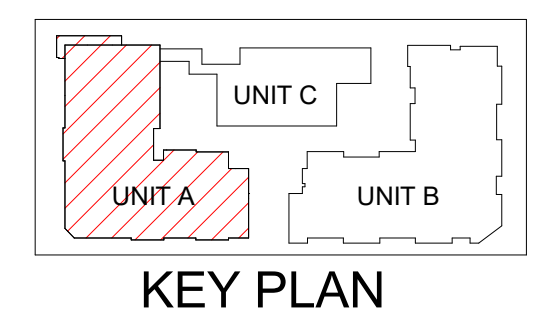
**NEW 2019.06.14**

4TH FLOOR FRAMING PLAN PART B  
**S-204.1B**





- NOTES:
- SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
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  - REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - REFERENCE DRAWINGS S-204.1A FOR 4TH LEVEL FLOOR FRAMING PLAN
  - DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



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1 4TH FLOOR CEILING FRAMING PLAN PART A  
 1/8" = 1'-0"

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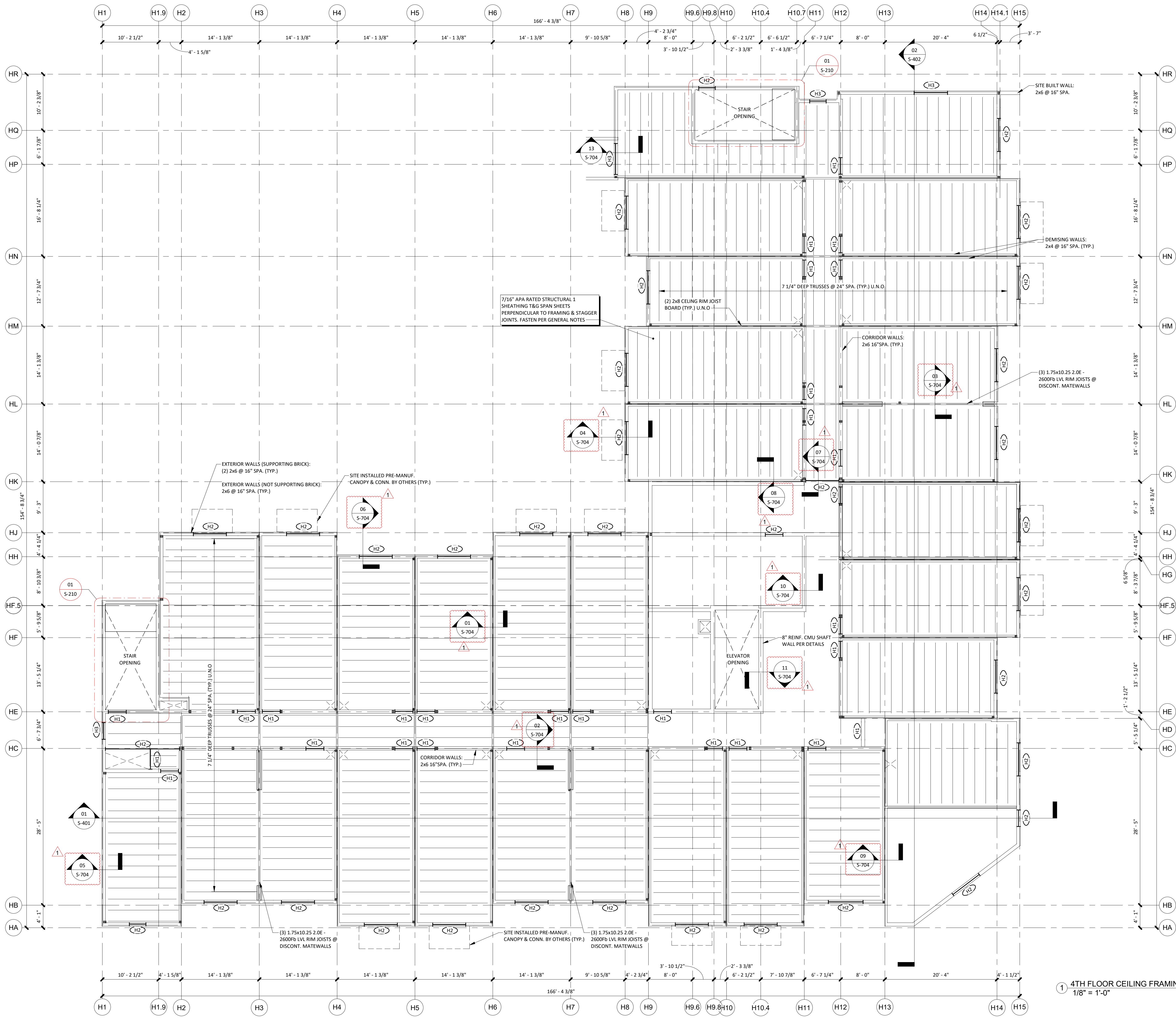
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4TH FLOOR CEILING FRAMING PLAN  
 PART A

**S-204.2A**



7/16" APA RATED STRUCTURAL I SHEATHING T&G SPAN SHEETS PERPENDICULAR TO FRAMING & STAGGER JOINTS. FASTEN PER GENERAL NOTES

EXTERIOR WALLS (SUPPORTING BRICK): (2) 2x6 @ 16" SPA. (TYP.)  
 EXTERIOR WALLS (NOT SUPPORTING BRICK): 2x6 @ 16" SPA. (TYP.)  
 SITE INSTALLED PRE-MANUF. CANOPY & CONN. BY OTHERS (TYP.)

CORRIDOR WALLS: 2x6 16" SPA. (TYP.)

8" REINF. CMU SHAFT WALL PER DETAILS

SITE BUILT WALL: 2x6 @ 16" SPA.

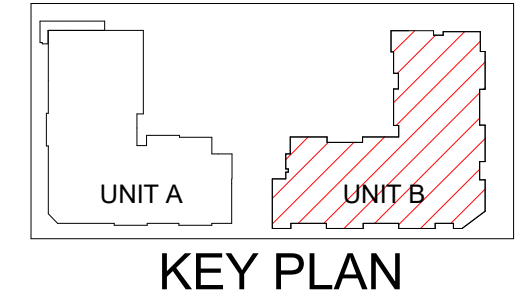
DEMISING WALLS: 2x4 @ 16" SPA. (TYP.)

CORRIDOR WALLS: 2x6 16" SPA. (TYP.)

(3) 1.75x10.25 2.0E - 2600Fb LVL RIM JOISTS @ DISCONT. MATEWALLS

(3) 1.75x10.25 2.0E - 2600Fb LVL RIM JOISTS @ DISCONT. MATEWALLS  
 SITE INSTALLED PRE-MANUF. CANOPY & CONN. BY OTHERS (TYP.)  
 (3) 1.75x10.25 2.0E - 2600Fb LVL RIM JOISTS @ DISCONT. MATEWALLS

- NOTES:
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-204.1B FOR FLOOR LEVEL FRAMING PLAN
  - 6.) ○ = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



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1 4TH FLOOR CEILING FRAMING PLAN PART B  
 1/8" = 1'-0"

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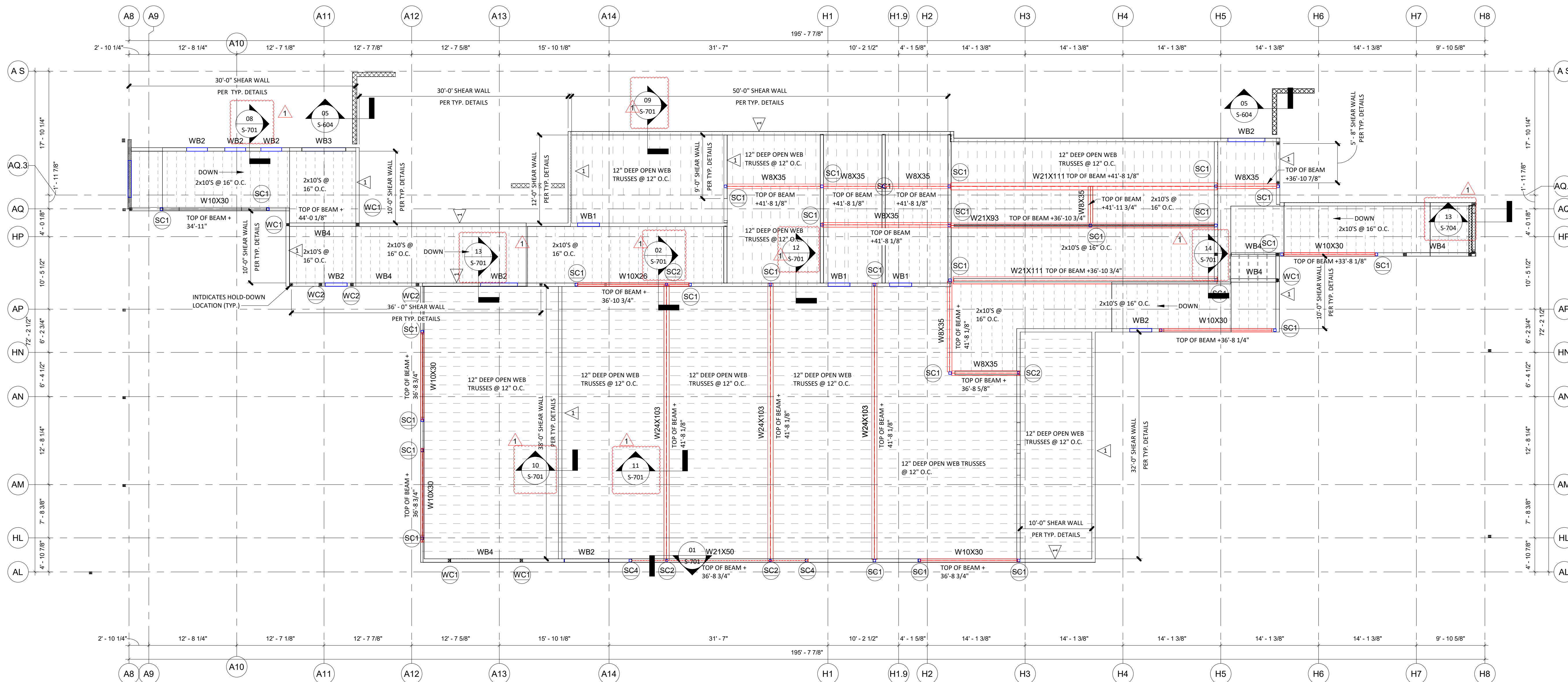
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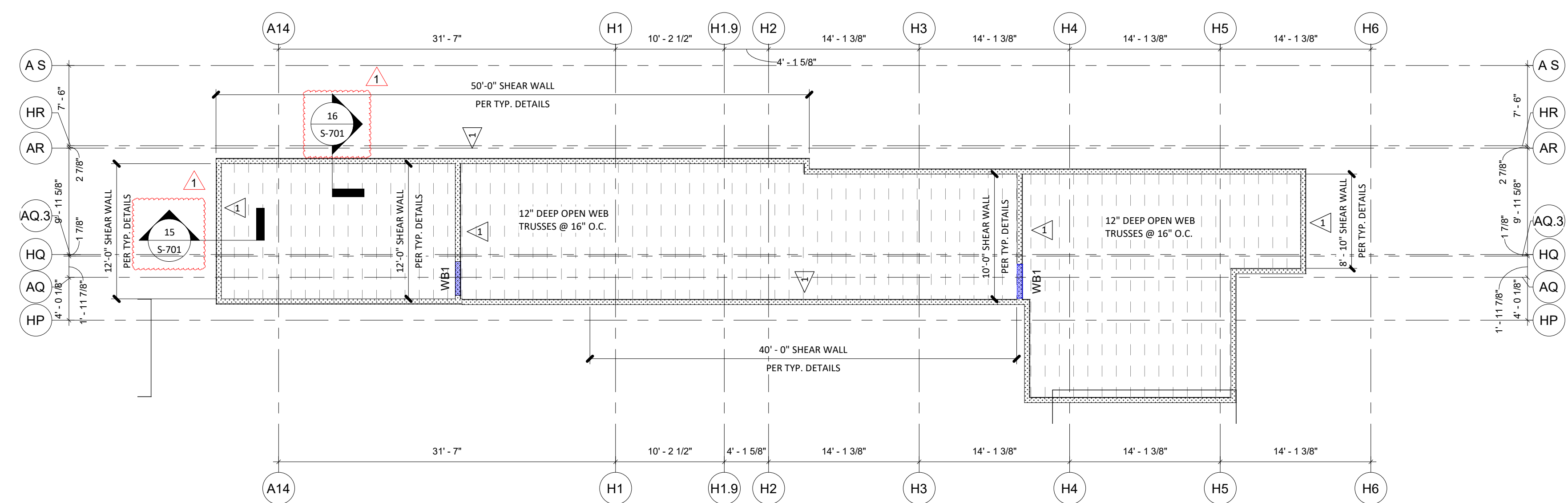
4TH FLOOR CEILING FRAMING PLAN PART B

**S-204.2B**

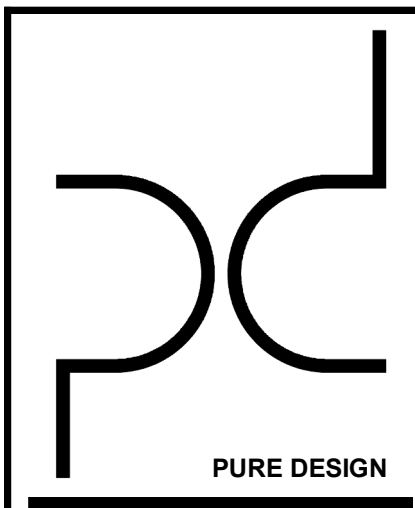


1 POOL ROOF FRAMING PLAN  
1/8" = 1'-0"

SHEAR WALL MARK	PLYWOOD SHEATHING	PANEL EDGE BLOCKING	FASTENING	FLOOR TO FLOOR CONNECTION ENDS	HOLD DOWN AT FOUNDATION	HOLD DOWN ANCHORS	SILL PLATE ANCHORS
1	15/32" OSB APA RATED SHEATHING (1) SIDE	BLOCKED	10d NAILS @ 6" SPA. PERIMETER @ 12" SPA. FIELD	(1) SIMPSON HDU2-SDS2.5 W/ (3) 2x POST	(1) HDU2-SDS2.5	1/2" Ø THREADED ROD ANCHORS - ANCHOR USING ADHESIVE ANCHORING SYSTEM - EMBED 10"	1/2" Ø CONC. SCREW ANCHOR @ 32" SPA. - EMBED 3 1/2" MIN.



2 AMENITIES ROOF FRAMING PLAN  
1/8" = 1'-0"



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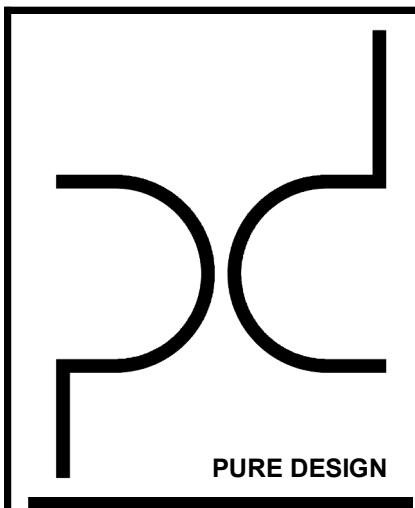
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4TH FLOOR STEEL FRAMING PLAN  
PART C  
**S-204.2C**



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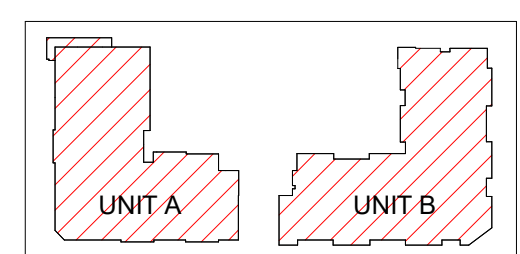
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5TH FLOOR FRAMING PLAN

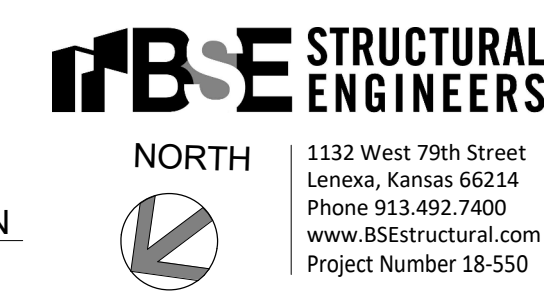
**S-205.1**



- NOTES:**
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
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  - 5.) REFERENCE DRAWINGS S-205.2A & S-205.2B FOR 4TH LEVEL CEILING FRAMING PLAN

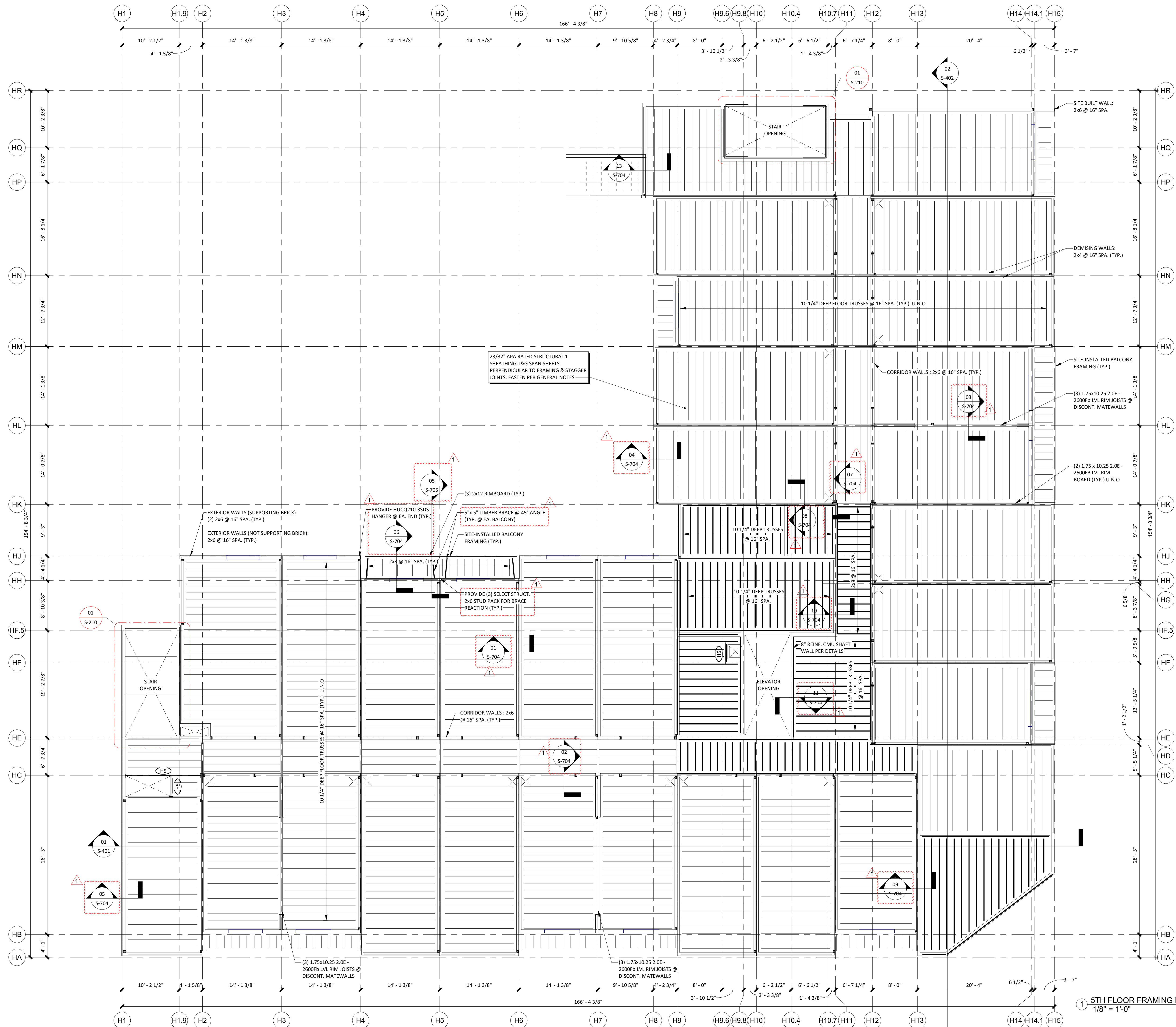


KEY PLAN

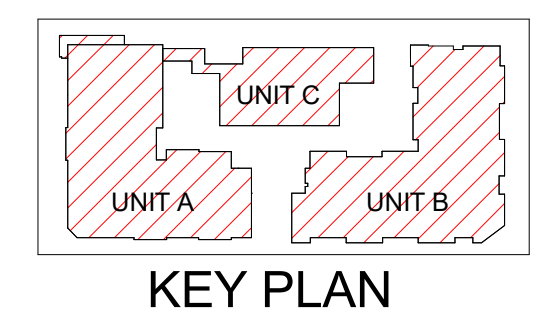


1 OVERALL 5TH FLOOR FRAMING PLAN  
1/16" = 1'-0"





- NOTES:**
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  - REFERENCE DRAWINGS S-205.2B FOR 4TH LEVEL CEILING FRAMING PLAN
  - = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



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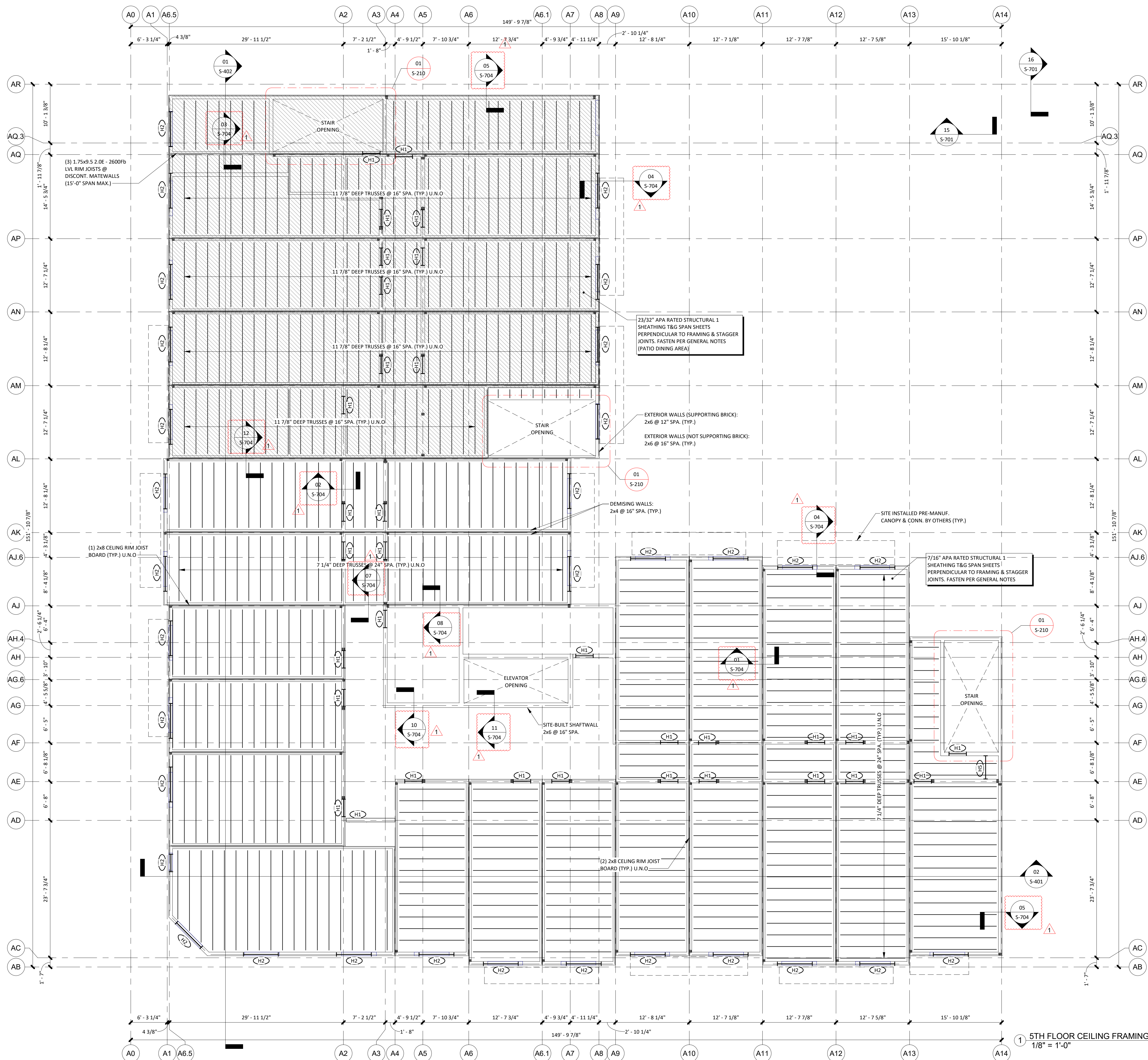


AC HOTELS

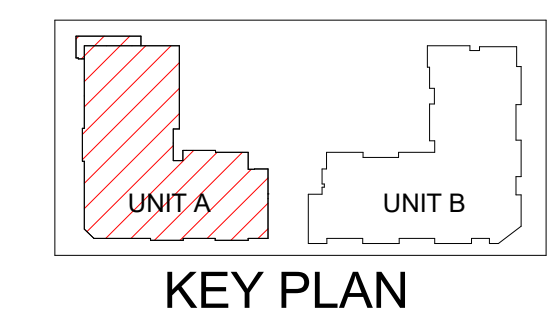
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**5TH FLOOR FRAMING PLAN PART B**  
 1/8" = 1'-0"

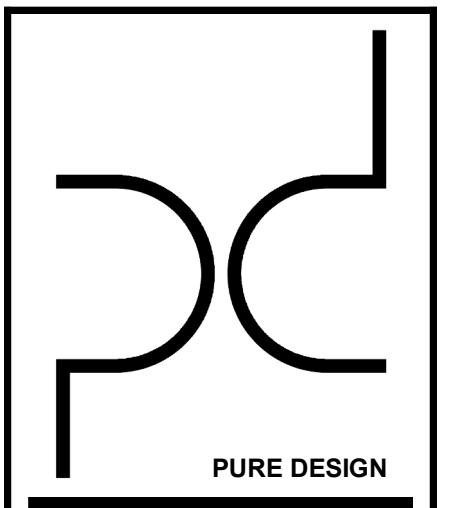


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**5TH FLOOR CEILING FRAMING PLAN PART A**  
 1/8" = 1'-0"



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**DUAL HOTEL DURANGO**  
**- AC & HIS**

**AC HOTELS**

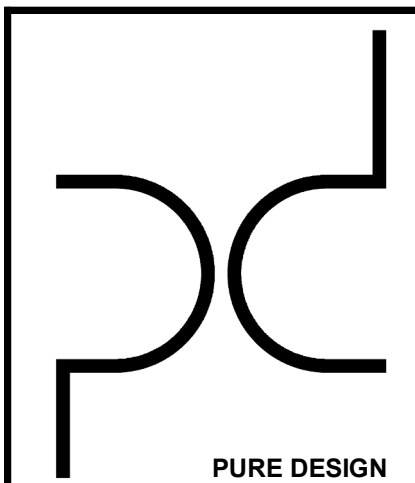
JOB NO: 18-007  
 DATE: 2019.03.07  
 ISSUE RECORD:  
 REVISIONS:  
 1 05/17/2019

**NEW 2019.06.14**

**S-205.2A**







**ARCHITECT**  
 PURE DESIGN  
 11254 N. MADISON AVE.  
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 PHONE: 913.492.7400

**MEP ENGINEERS**  
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 LENEXA, KS 66215  
 PHONE: 913.492.2400

**CIVIL ENGINEERS**  
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 311 4th Ave South  
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**OWNER**  
 LAMONT COMPANIES  
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**DUAL HOTEL DURANGO**  
 - AC & HIS

AC HOTELS

JOB NO: 18-007  
 DATE: 2018.03.07

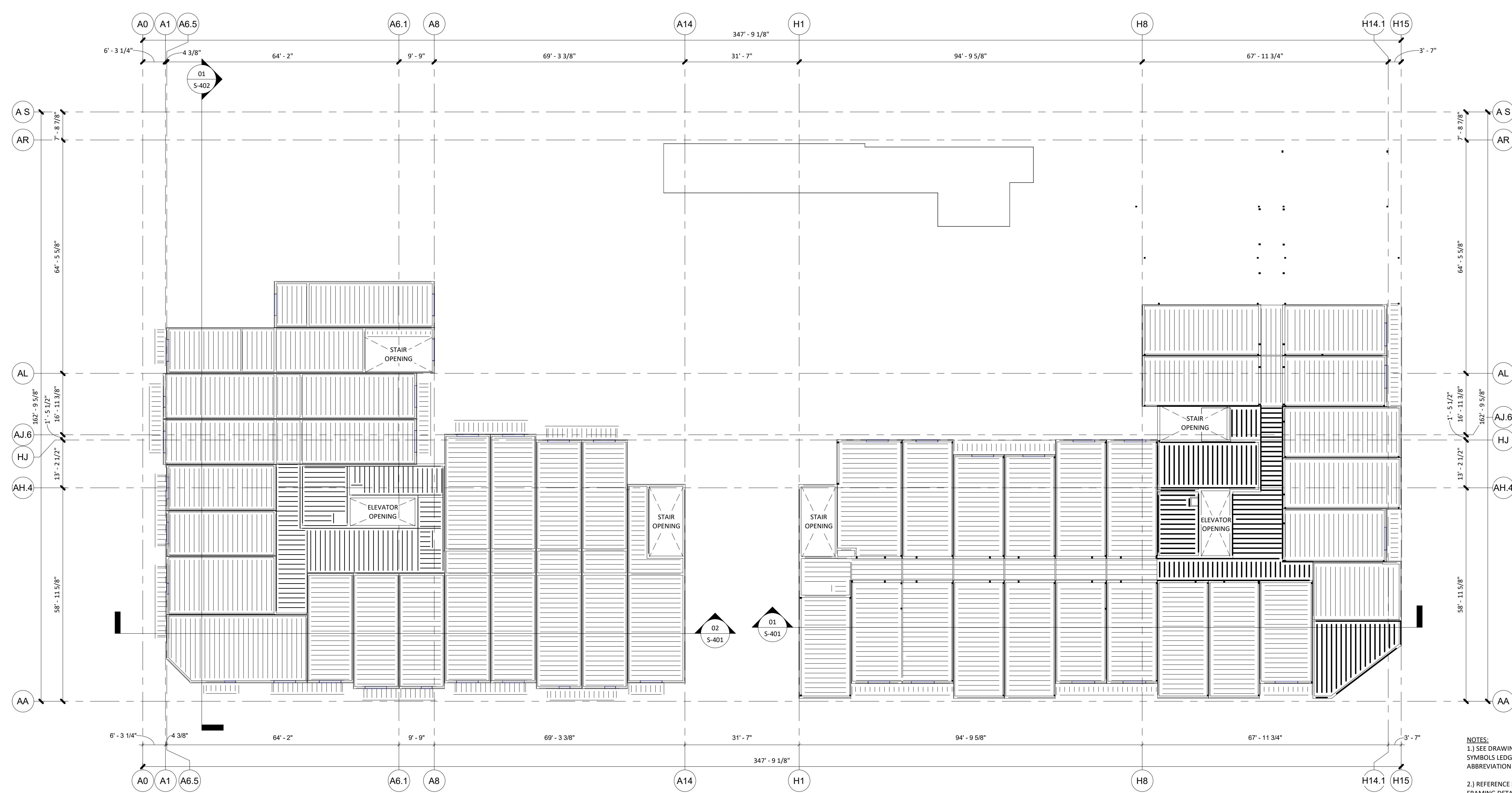
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REVISIONS:

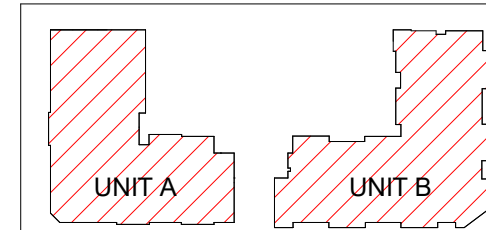
NEW 2019.06.14

6TH FLOOR FRAMING PLAN

**S-206.1**



- NOTES:**
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEDGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-207.1A & S-207.1B FOR ROOF FRAMING PLANS



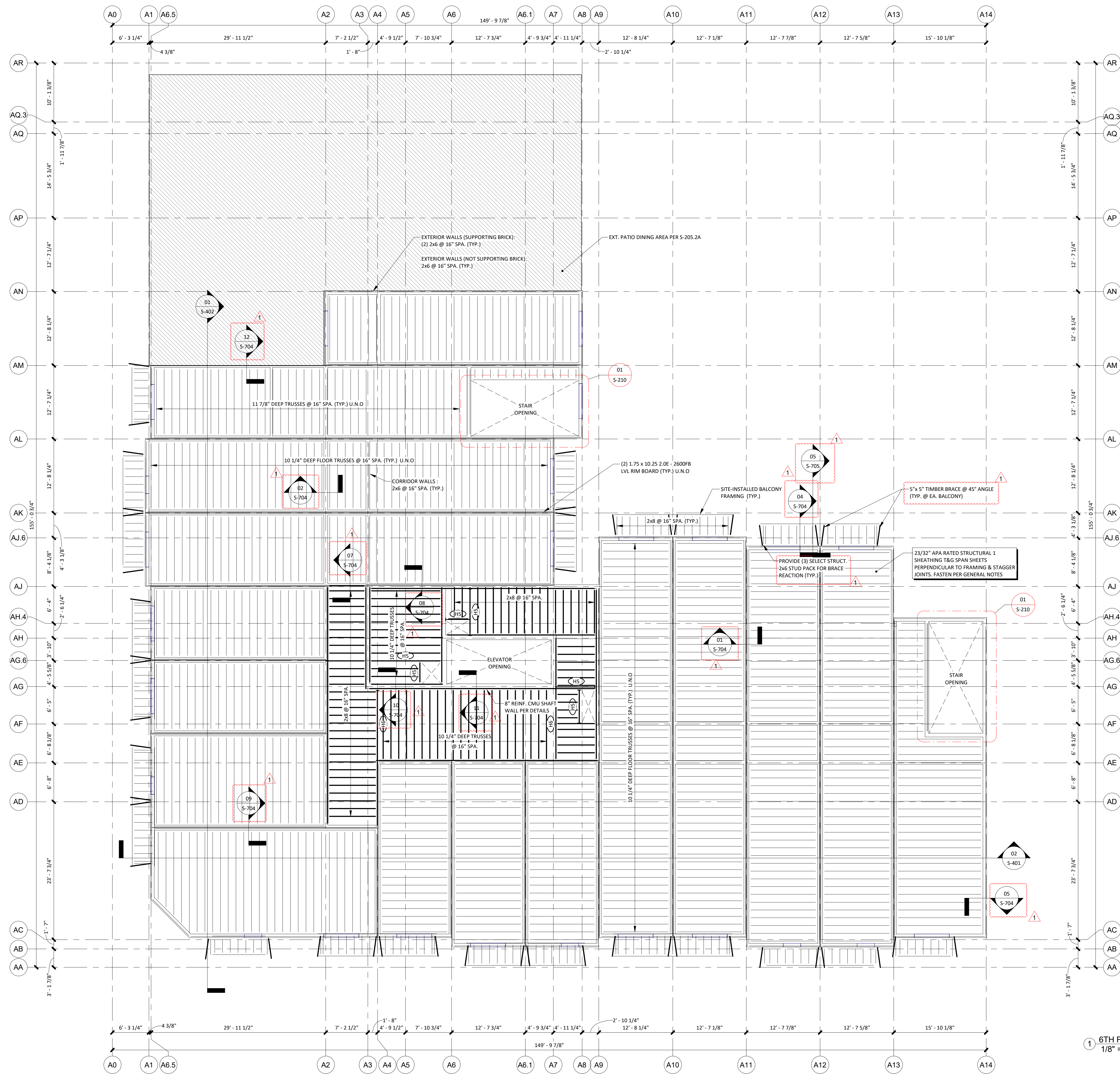
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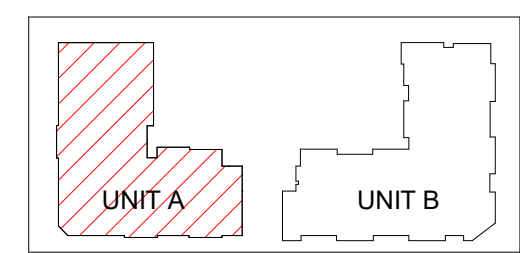
**NORTH**

1132 West 79th Street  
 Lenexa, Kansas 66214  
 Phone 913.492.7400  
 www.BSEstructural.com  
 Project Number 18-550

1 OVERALL 6TH FLOOR FRAMING PLAN  
 1/16" = 1'-0"



- NOTES:**
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-207.1A FOR 4TH LEVEL CEILING FRAMING PLAN
  - 6.) DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



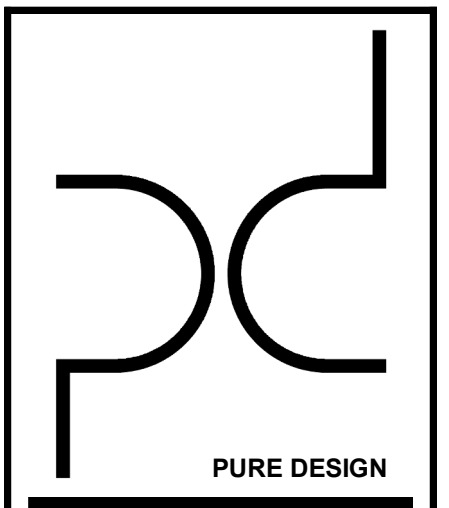
**KEY PLAN**

**STRUCTURAL ENGINEERS**

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Lenexa, Kansas 66214  
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www.BSEstructural.com  
Project Number 18-550

1 6TH FLOOR FRAMING PLAN PART A  
1/8" = 1'-0"



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**DUAL HOTEL DURANGO**  
**- AC & HIS**

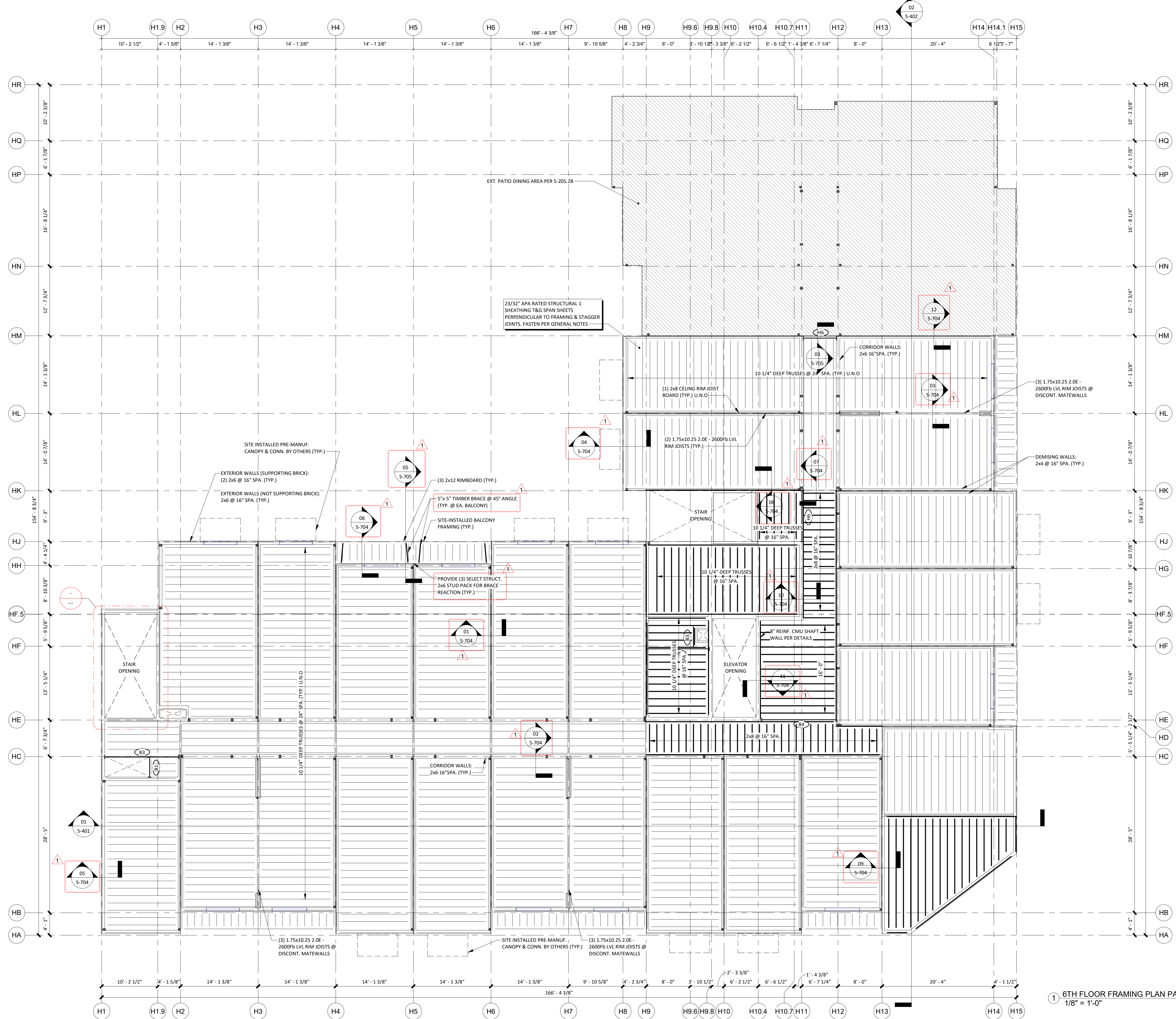
AC HOTELS

JOB NO: 18-007  
DATE: 2018.03.07  
ISSUE RECORD:  
REVISIONS:  
1 05/17/2019

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6TH FLOOR FRAMING PLAN PART A

**S-206.1A**



EXT. PATIO DINING AREA PER S-205.2B

23/32\"/>

SITE INSTALLED PRE-MANUF. CANOPY & CONN. BY OTHERS (TYP.)

EXTERIOR WALLS (SUPPORTING BRICK): 2x6 @ 16\"/>

EXTERIOR WALLS (NOT SUPPORTING BRICK): 2x6 @ 16\"/>

(3) 2x12 RIMBOARD (TYP.)

5\"/>

SITE-INSTALLED BALCONY FRAMING (TYP.)

PROVIDE (3) SELECT STRUCT. 2x6 STUD PACK FOR BRACE REACTION (TYP.)

CORRIDOR WALLS: 2x6 16\"/>

10 1/4\"/>

(1) 2x8 CEILING RIM JOIST BOARD (TYP.) U.N.O.

(2) 1.75x10.25 2.0E - 2600Fb LVL RIM JOISTS (TYP.)

(3) 1.75x10.25 2.0E - 2600Fb LVL RIM JOISTS @ DISCONT. MATEWALLS

DEMISING WALLS: 2x4 @ 16\"/>

STAIR OPENING

10 1/4\"/>

10 1/4\"/>

ELEVATOR OPENING

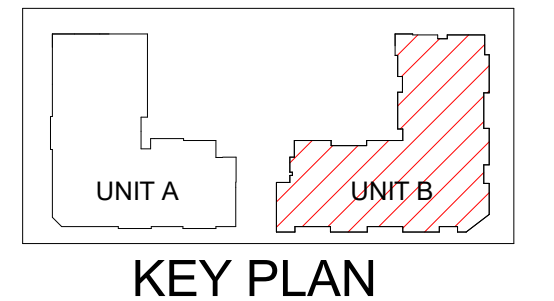
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CORRIDOR WALLS: 2x6 16\"/>

SITE INSTALLED PRE-MANUF. CANOPY & CONN. BY OTHERS (TYP.)

(3) 1.75x10.25 2.0E - 2600Fb LVL RIM JOISTS @ DISCONT. MATEWALLS

- NOTES:
- SEE DRAWINGS S-101 FOR GENERAL NOTES, SYMBOLS, LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - REFERENCE DRAWINGS S-207.1B FOR CEILING LEVEL FRAMING PLAN
  - DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



1 6TH FLOOR FRAMING PLAN PART B  
1/8\"/>

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AC HOTELS

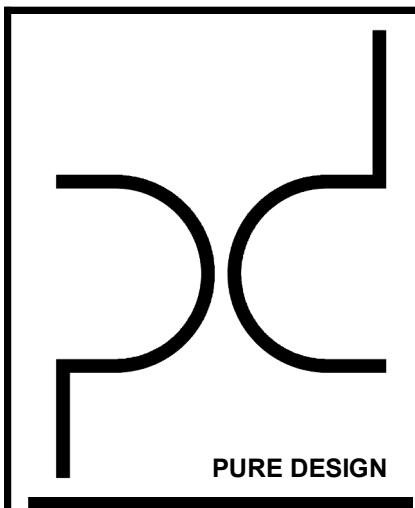
**DUAL HOTEL DURANGO**  
**- AC & HIS**

JOB NO: 18-007  
DATE: 2019.03.07  
ISSUE RECORD:  
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6TH FLOOR FRAMING PLAN PART B

**S-206.1B**



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JOB NO: 18-007  
 DATE: 2018.03.07

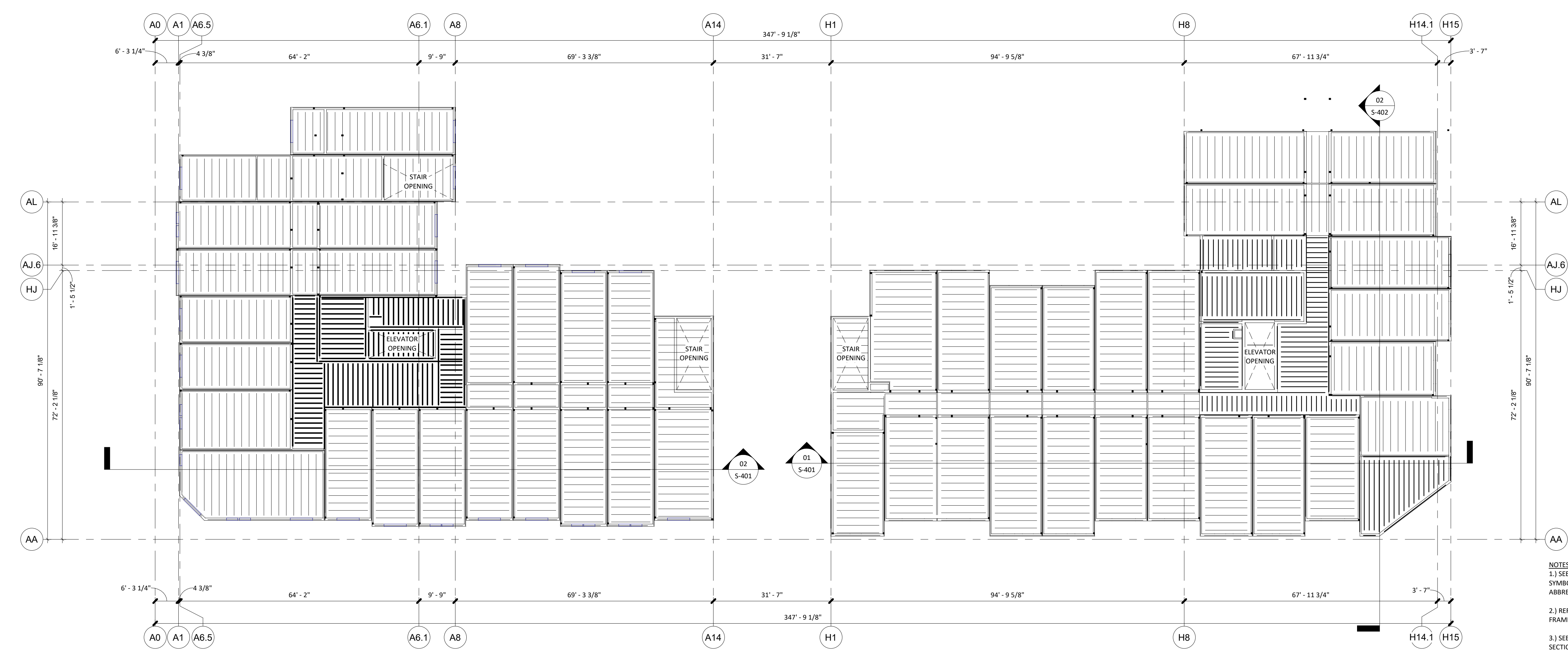
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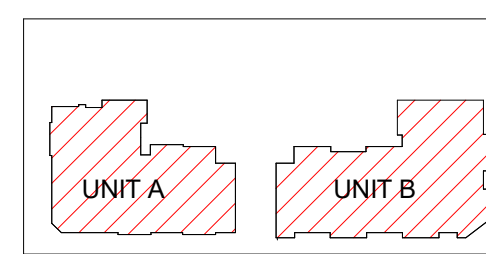
NEW 2019.06.14

ROOF FRAMING PLAN

**S-207.1**



- NOTES:**
- 1.) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2.) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3.) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5.) REFERENCE DRAWINGS S-206.1 FOR 6TH FLOOR LEVEL FRAMING PLAN

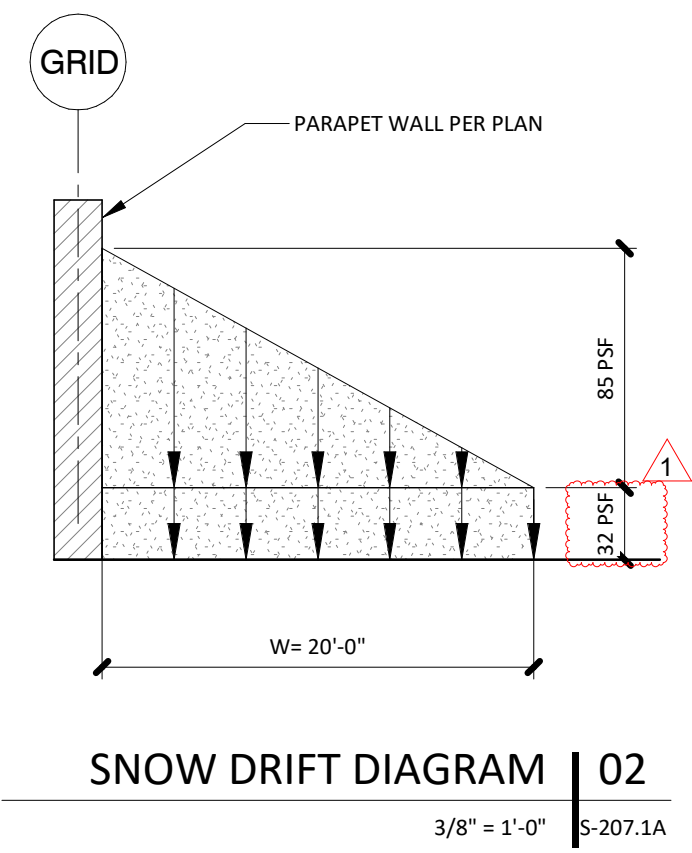
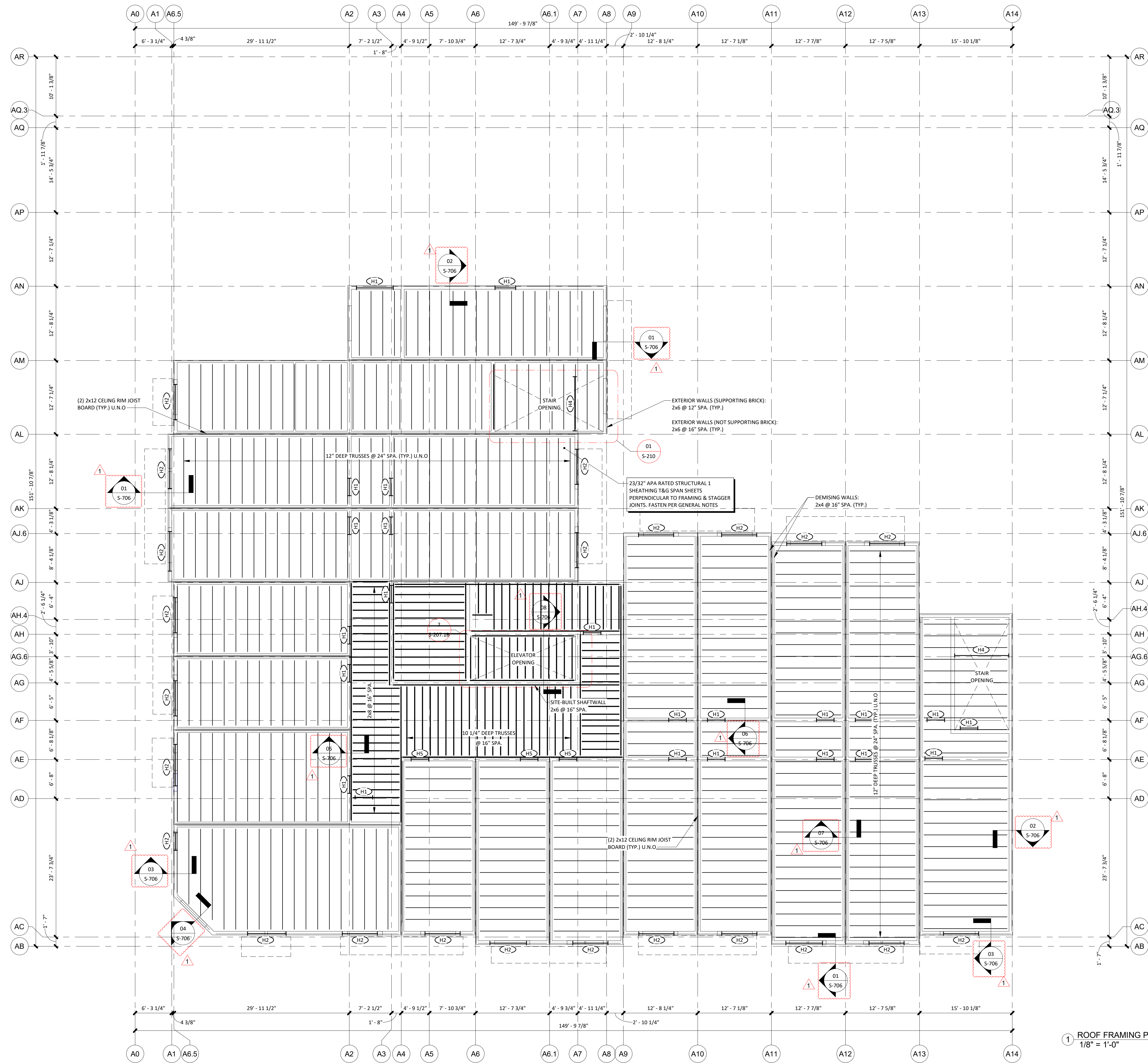


**KEY PLAN**

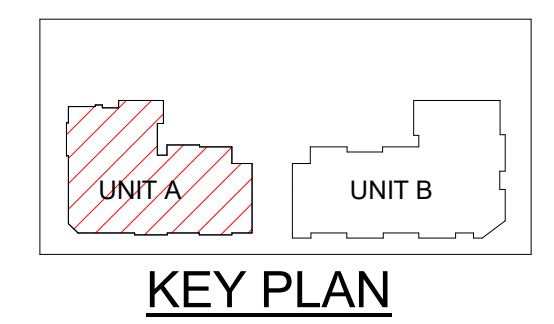


**NORTH**  
 1132 West 79th Street  
 Lenexa, Kansas 66214  
 Phone 913.492.7400  
 www.BSEstructural.com  
 Project Number 18-550

**1 OVERALL ROOF FRAMING PLAN**  
 1/16" = 1'-0"



- NOTES:
- SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - REFERENCE DRAWINGS S-206.1A FOR FLOOR LEVEL FRAMING PLAN
  - = DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



**BSE STRUCTURAL ENGINEERS**  
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 www.BSEStructural.com  
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1 ROOF FRAMING PLAN PART A  
 1/8" = 1'-0"

PURE DESIGN

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 11264 N. MADISON AVE.  
 KANSAS CITY, MO 64155  
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 - AC & HIS

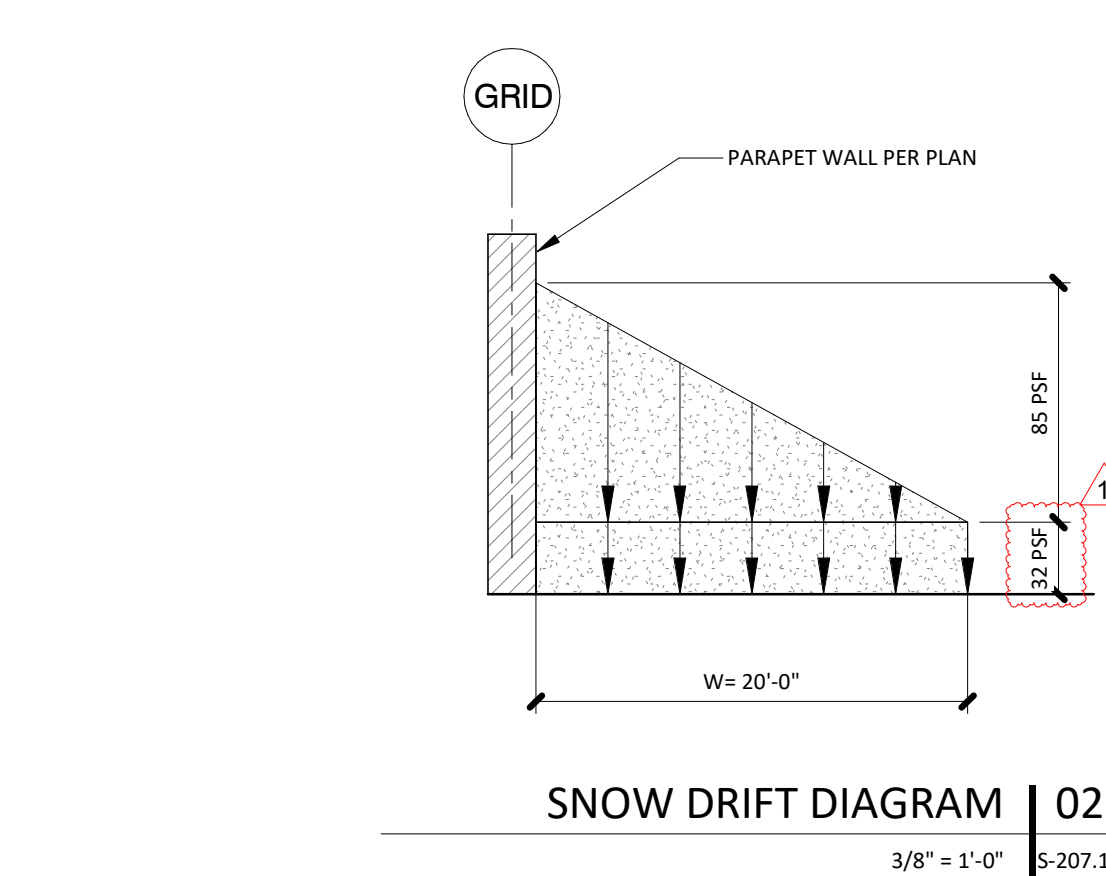
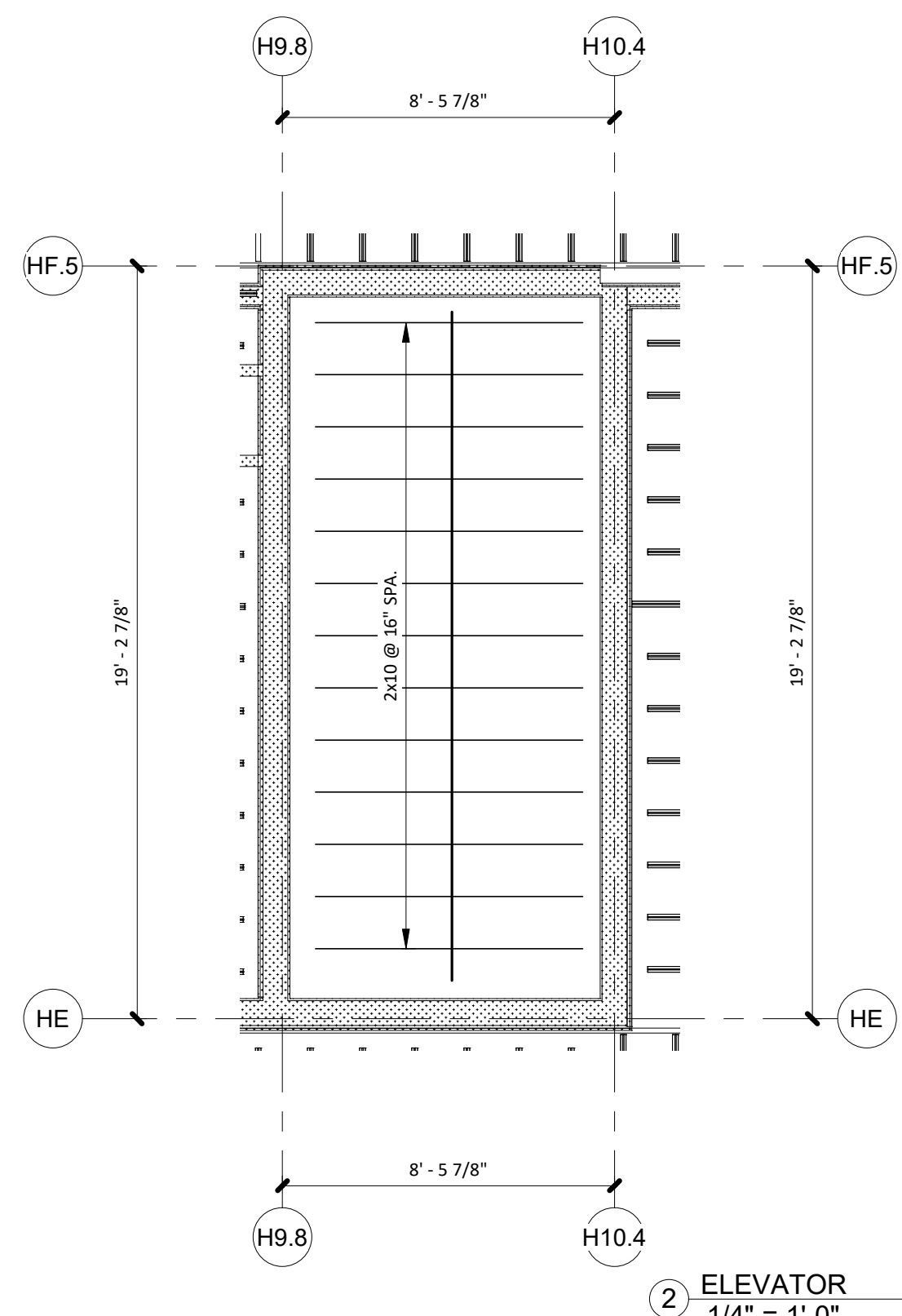
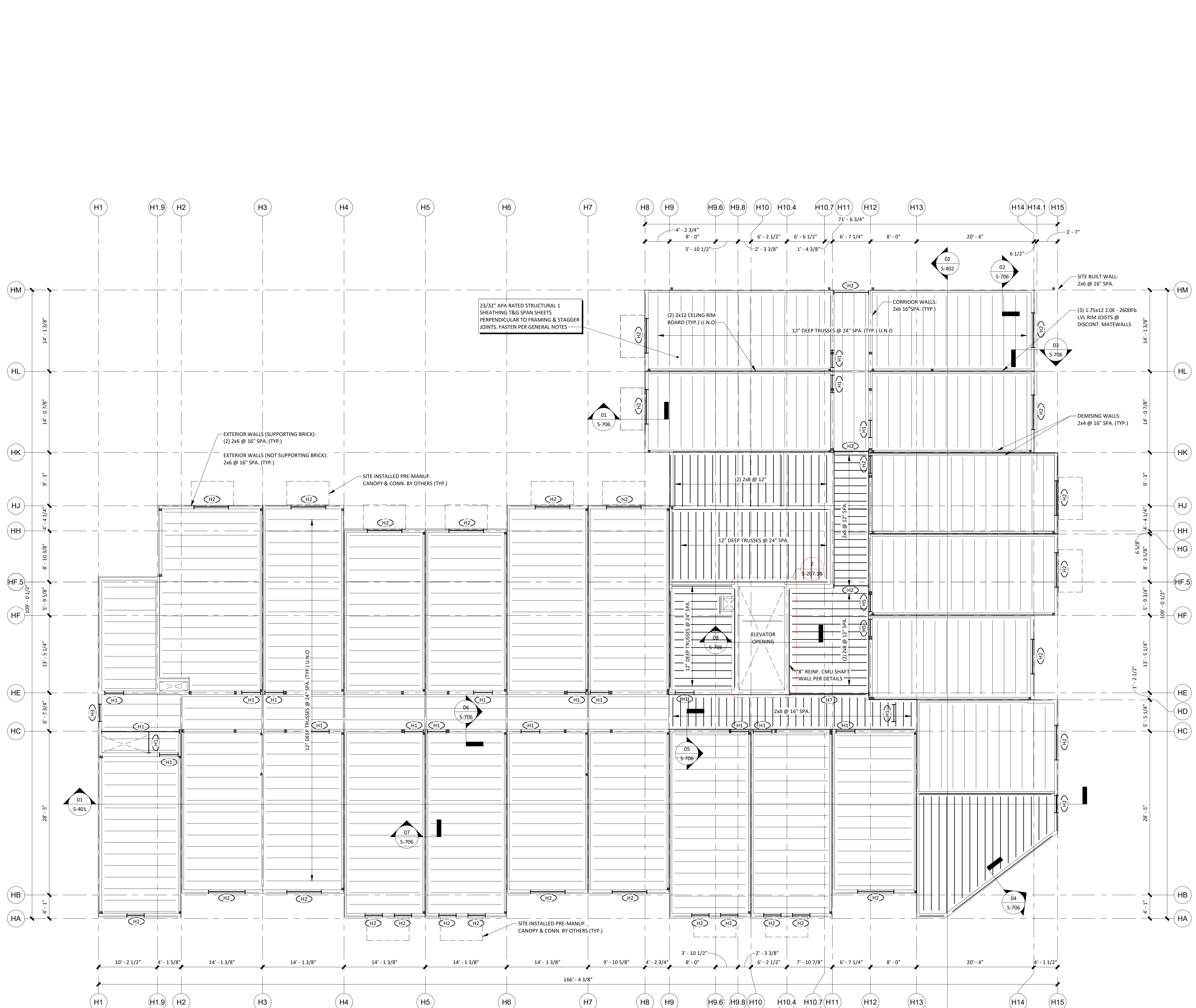
AC HOTELS

JOB NO: 18-007  
 DATE: 2019.03.07  
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 1 05/17/2019

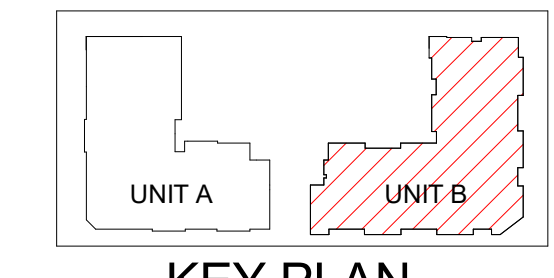
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ROOF FRAMING PLAN PART A

**S-207.1A**



- NOTES:
- 1) SEE DRAWING S-101 FOR GENERAL NOTES, SYMBOLS LEDGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - 2) REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - 3) SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - 4) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - 5) REFERENCE DRAWINGS S-206.1B FOR FLOOR LEVEL FRAMING PLAN
  - 6) ○ DENOTES HEADER - REFER TO SCHEDULE & DETAILS. REFERENCE SHEET S-610 FOR HEADER SCHEDULE



**STRUCTURAL ENGINEERS**  
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 Project Number 18-550

1 ROOF FRAMING PLAN PART B  
 1/8" = 1'-0"

PURE DESIGN

ARCHITECT  
 PURE DESIGN  
 11254 N. MADISON AVE.  
 KANSAS CITY, MO 64155  
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**DUAL HOTEL DURANGO**  
 - AC & HIS

AC HOTELS

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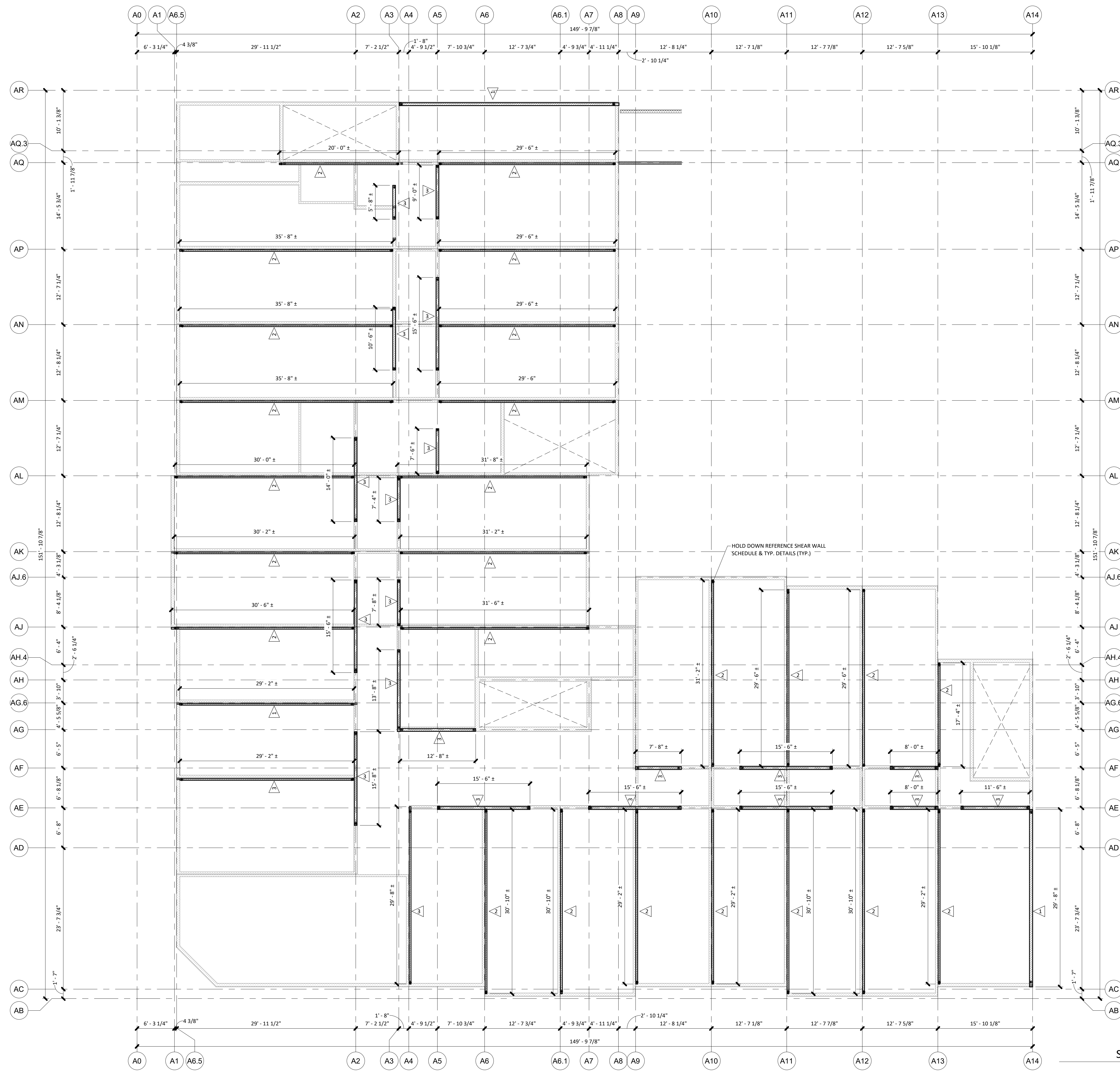
ISSUE RECORD:

REVISIONS:	DATE:
1	05/17/2019

**NEW 2019.06.14**

ROOF FRAMING PLAN PART B

**S-207.1B**



LATERAL STABILITY SCHEDULE

MARK	LEVEL	SHEARWALL 02/S-609	HOLD-DOWN 03/S-609
1	6	A	A
	5	A	A
	4	A	A
	3	A	A/D
2	6	A	A
	5	A	A
	4	C	C
3	3	C	C/D
	6	A	A
	5	B	A
	4	C	B
	3	C	B/E

- NOTES:
- SEE DRAWINGS S101-1 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
  - REFERENCE S-600 SERIES SHEETS FOR TYPICAL FRAMING DETAILS
  - SEE DRAWINGS S401 & S402 FOR FULL BUILDING SECTIONS
  - REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - =SHEAR WALL REFER TO LATERAL STABILITY SCHEDULE FOR SHEATHING, FASTENING, ANCHORING, & HOLD DOWN REQUIREMENTS.

SHEARWALL PLAN PART "A" | 01  
1/8" = 1'-0" S-208

**BSE STRUCTURAL ENGINEERS**  
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www.BSEStructural.com  
Project Number 18-550

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**DUAL HOTEL DURANGO - AC & HIS**

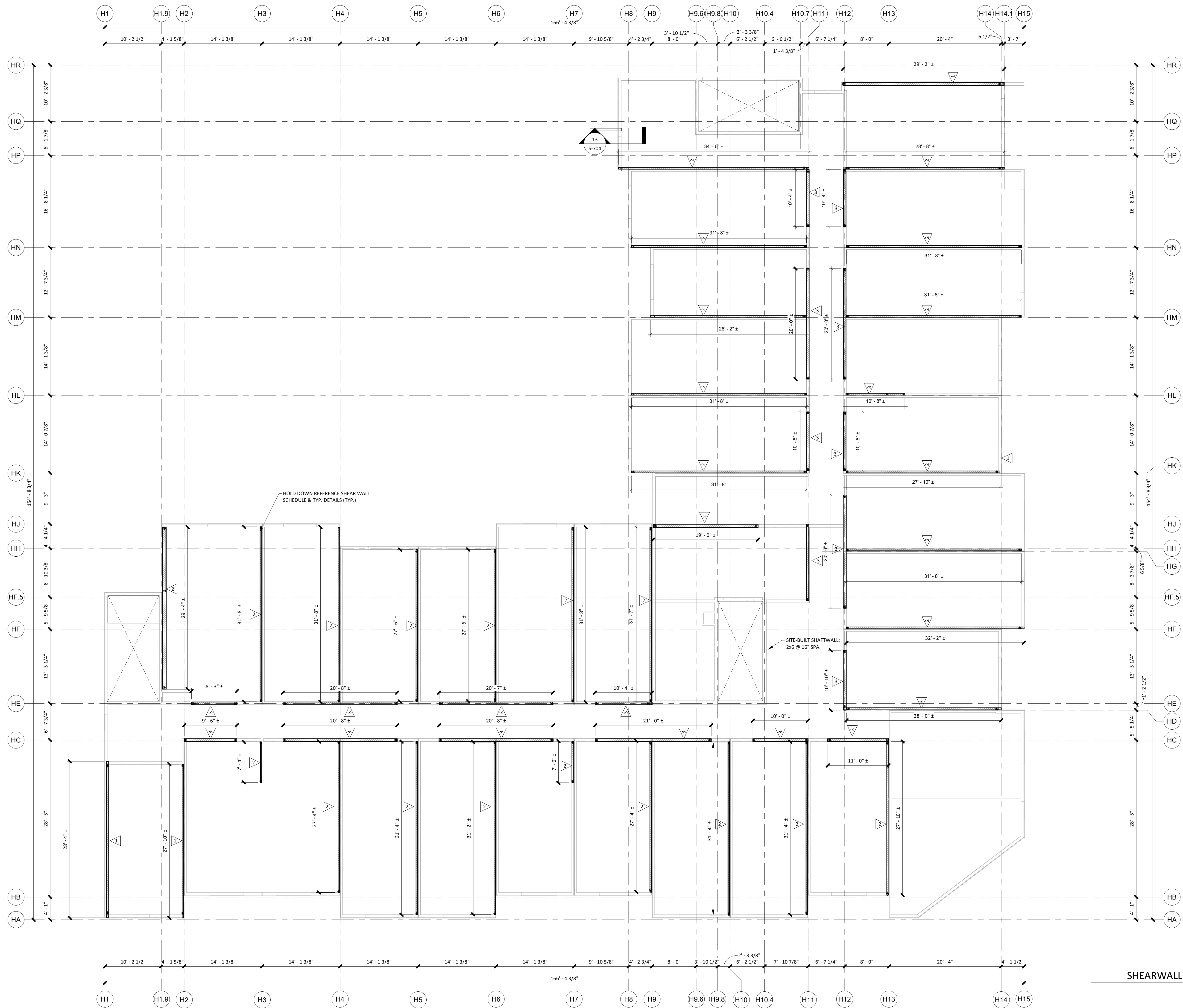
AC HOTELS

JOB NO: 18-007  
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1 05/17/2019

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SHEARWALL PLAN PART A

**S-208**



LATERAL STABILITY SCHEDULE

MARK	LEVEL	SHEARWALL 02/S-609	HOLD-DOWN 03/S-609
1	6	A	A
	5	A	A
	4	A	A
	3	A	A/D
2	6	A	A
	5	A	A
	4	C	C
3	3	C	C/D
	6	A	A
	5	B	A
	4	C	B
	3	C	B/E

- NOTES:
- SEE DRAWINGS S101-1 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST
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  - REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL FLOOR & WALL OPENINGS
  - SHEAR WALL REFER TO LATERAL STABILITY SCHEDULE FOR SHEATHING, FASTENING, ANCHORING, & HOLD DOWN REQUIREMENTS.

SHEARWALL PLAN PART B | 01  
1/8" = 1'-0" | S-209

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205 6TH AVE. SE - SUITE 300  
ABERDEEN, SD 57401  
PHONE: 605.225.1712

**DUAL HOTEL DURANGO - AC & HIS**

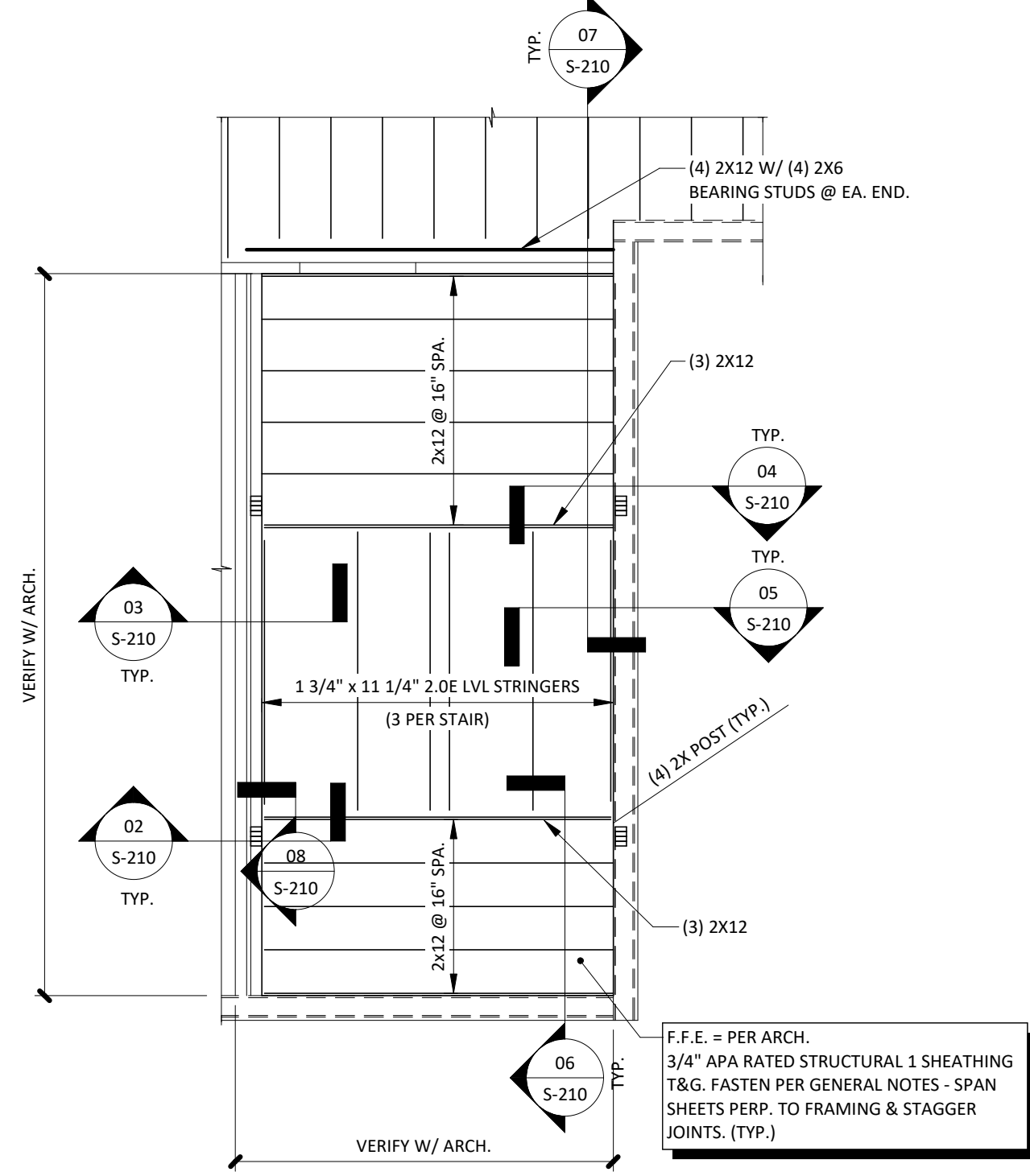
AC HOTELS

JOB NO: 18-007  
DATE: 2019.03.07  
ISSUE RECORD:  
REVISIONS:  
1 05/17/2019

NEW 2019.06.14

SHEARWALL PLAN PART B  
**S-209**



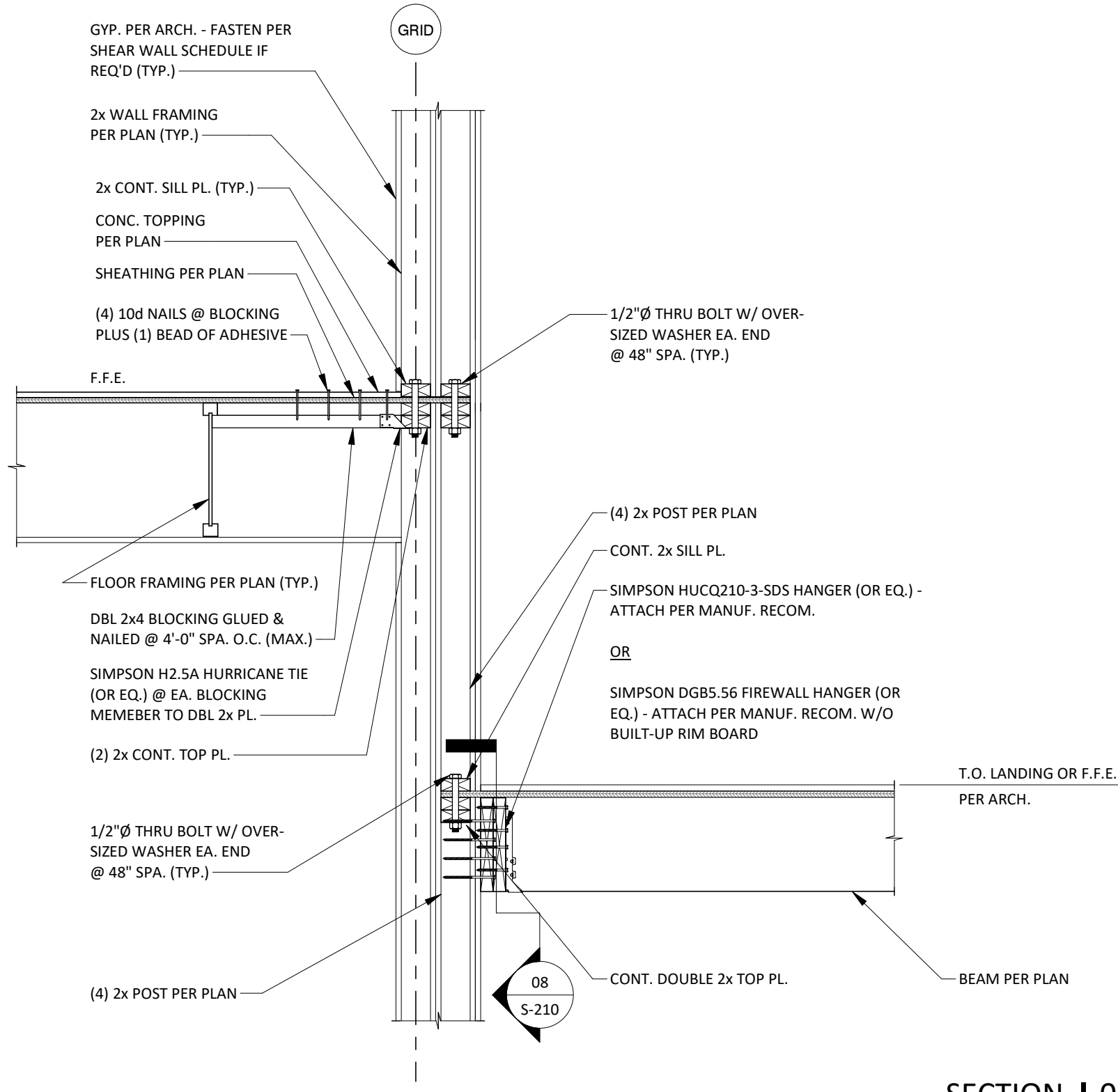


NOTES:

- 1.) SEE DRAWING S0.0 FOR GENERAL NOTES, SYMBOLS LEGEND, MATERIALS LEGEND, & ABBREVIATION LIST.
- 2.) REFERENCE DRAWING S4.1 FOR TYPICAL FRAMING DETAILS.
- 3.) SEE DRAWING S0.1 FOR FULL BUILDING SECTIONS.
- 4.) REFERENCE ARCHITECTURAL DRAWINGS TO VERIFY SIZE & LOCATIONS OF ALL ROOF & WALL OPENINGS.

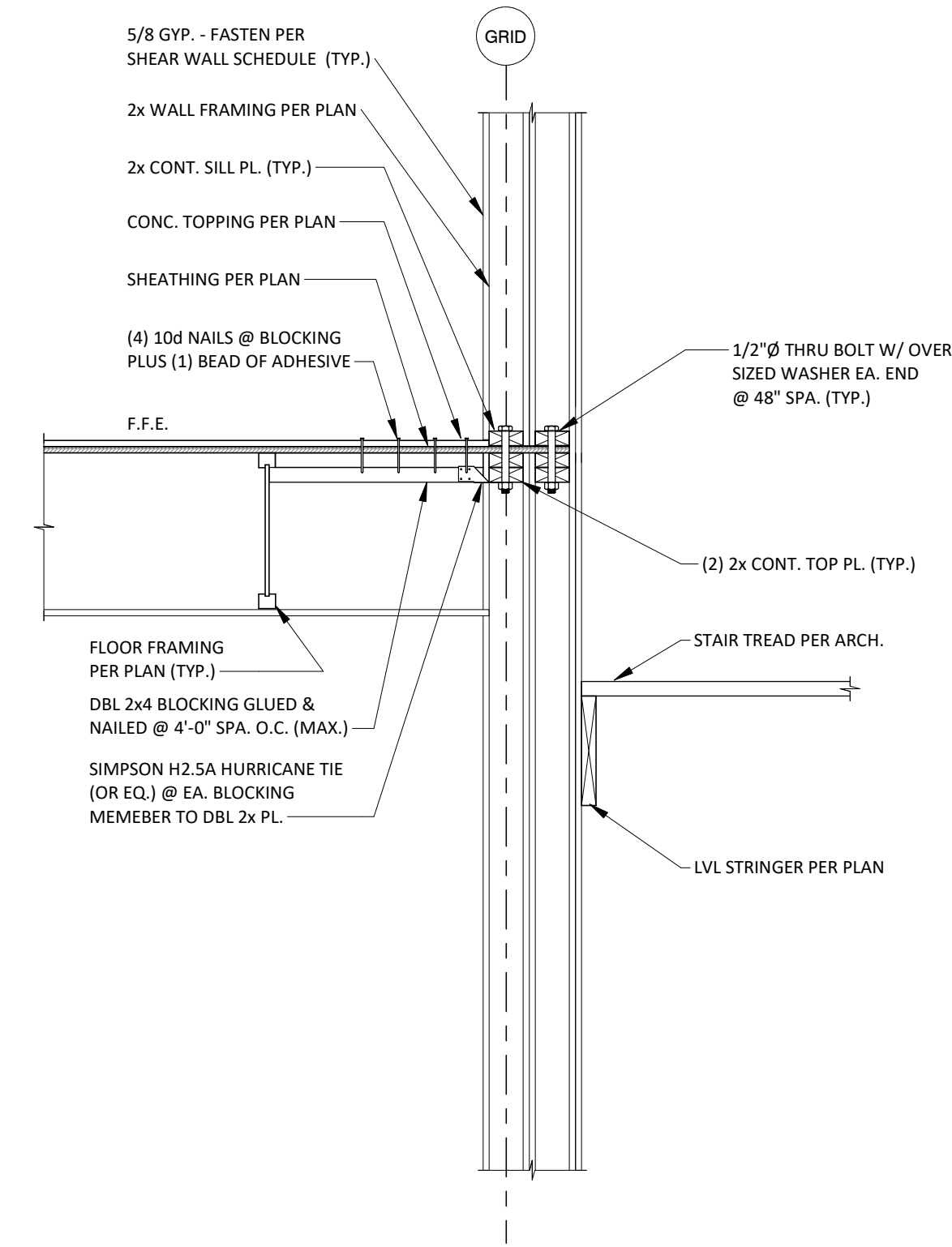
TYP. WOOD STAIR FRAMING PLAN | 01

1/4" = 1'-0" S-210



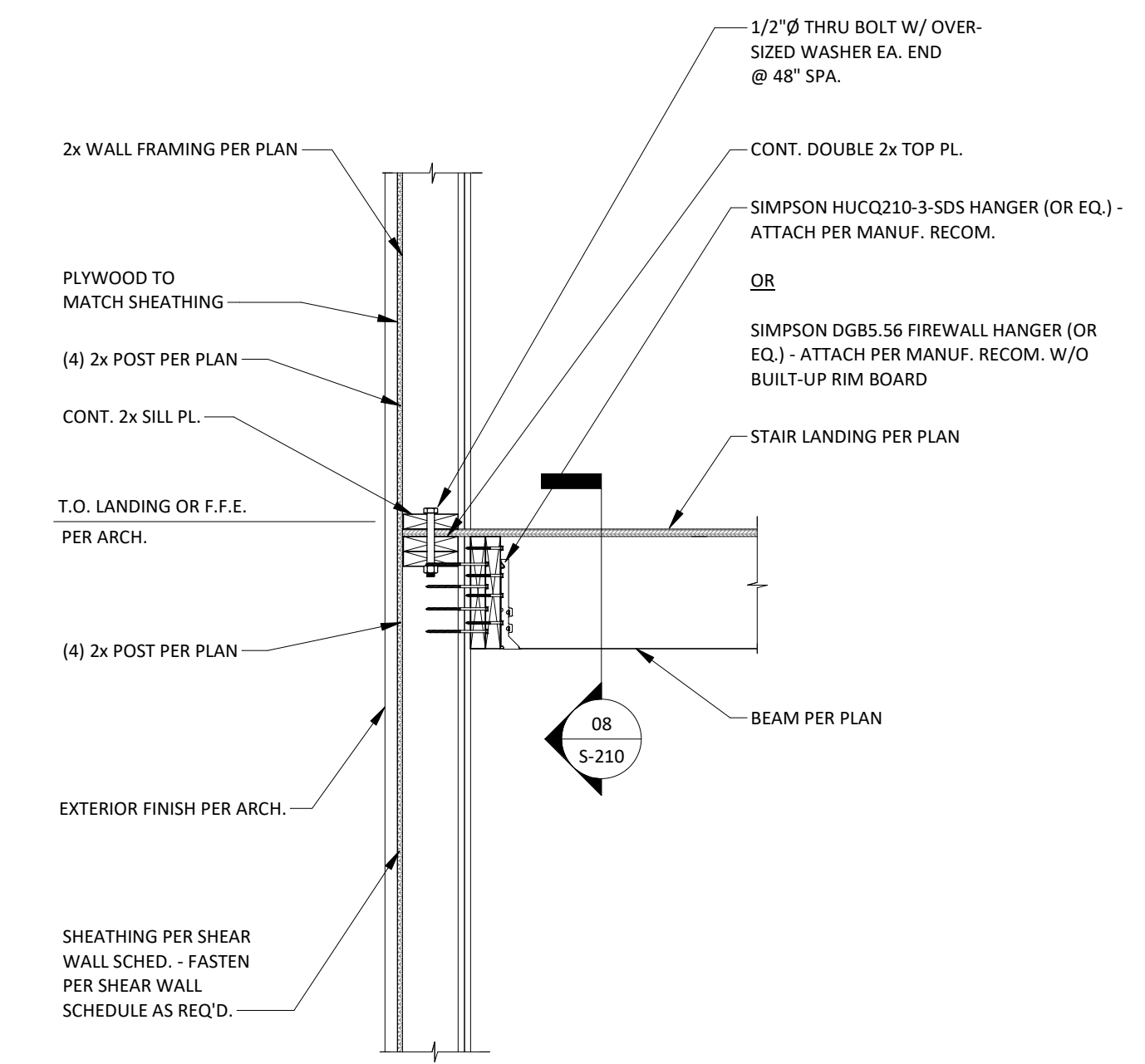
SECTION | 02

3/4" = 1'-0" S-210



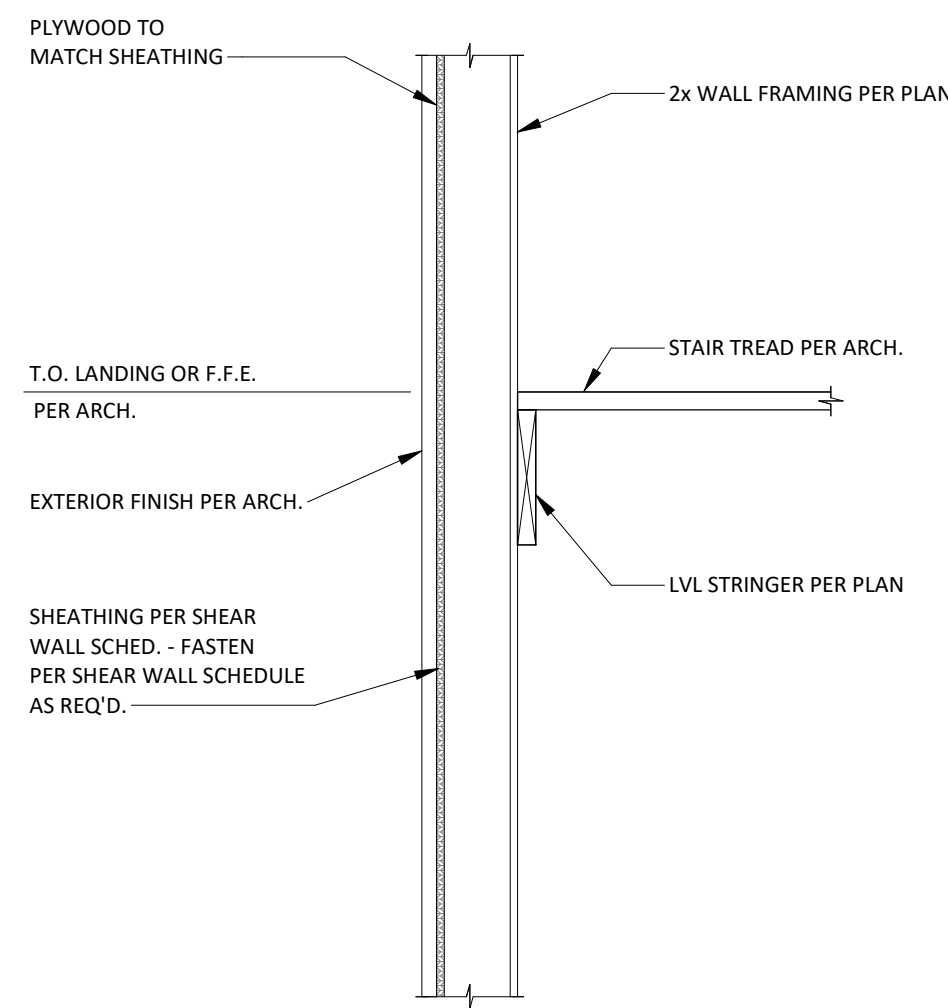
SECTION | 03

3/4" = 1'-0" S-210



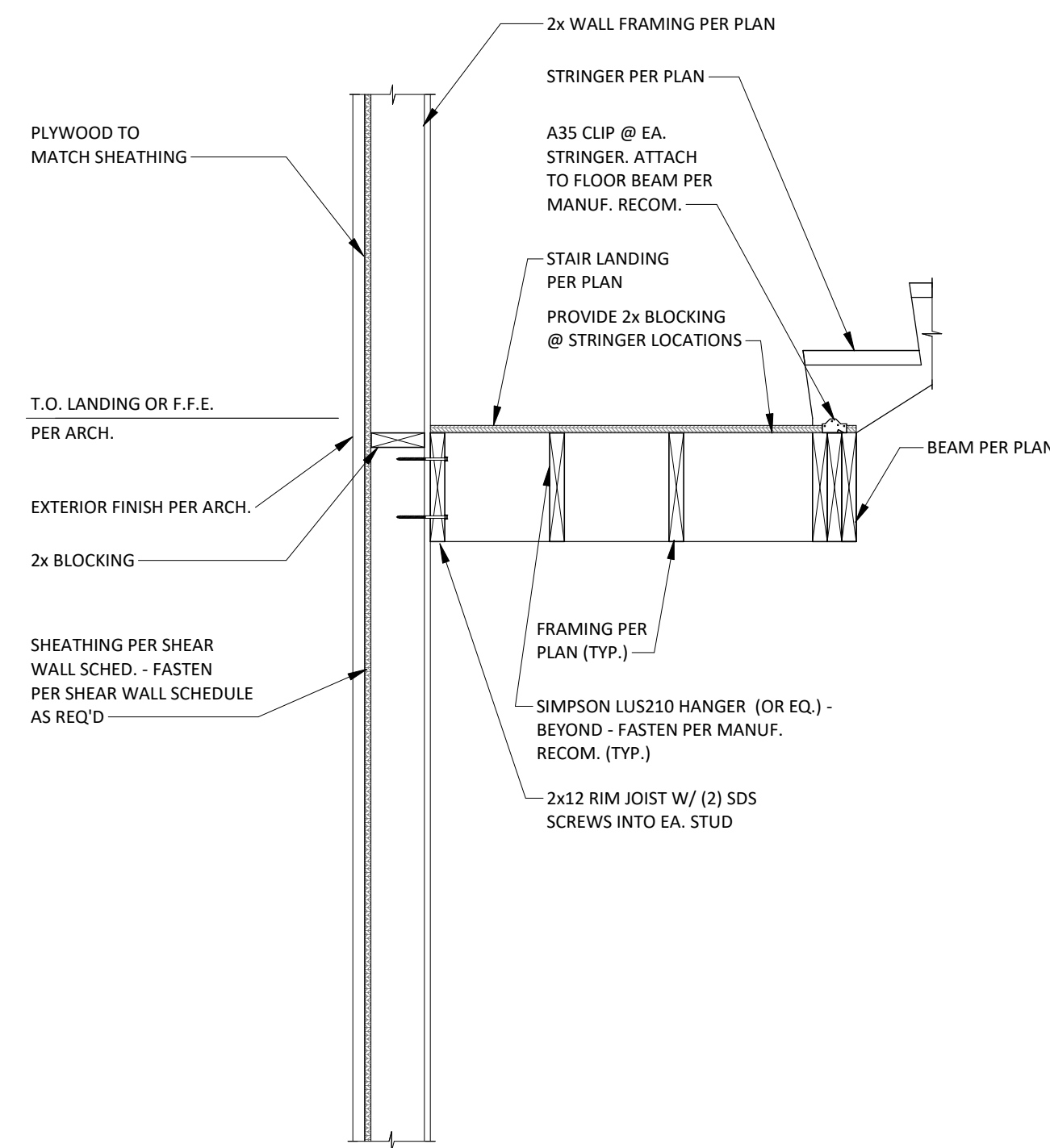
SECTION | 04

3/4" = 1'-0" S-210



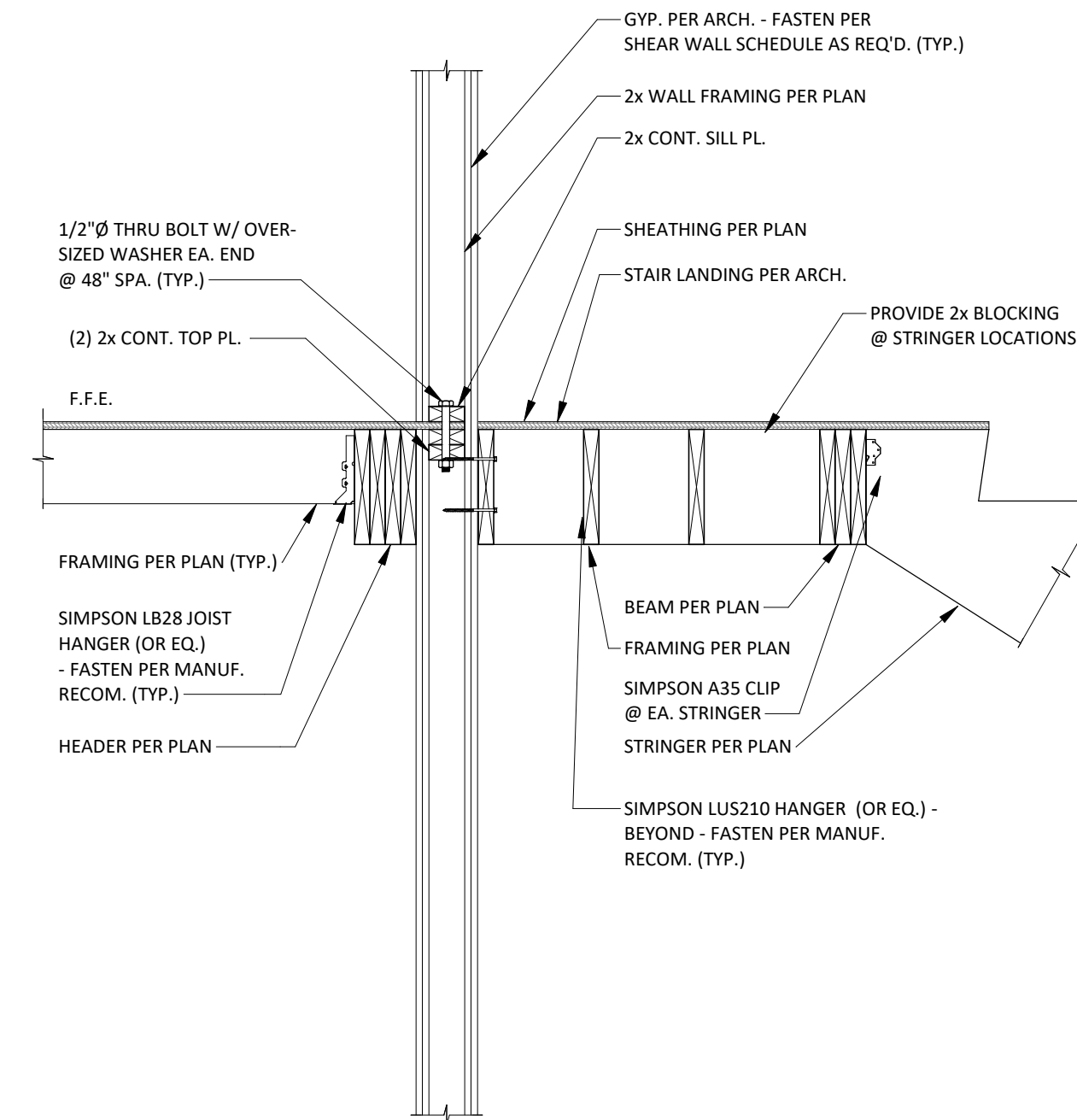
SECTION | 05

3/4" = 1'-0" S-210



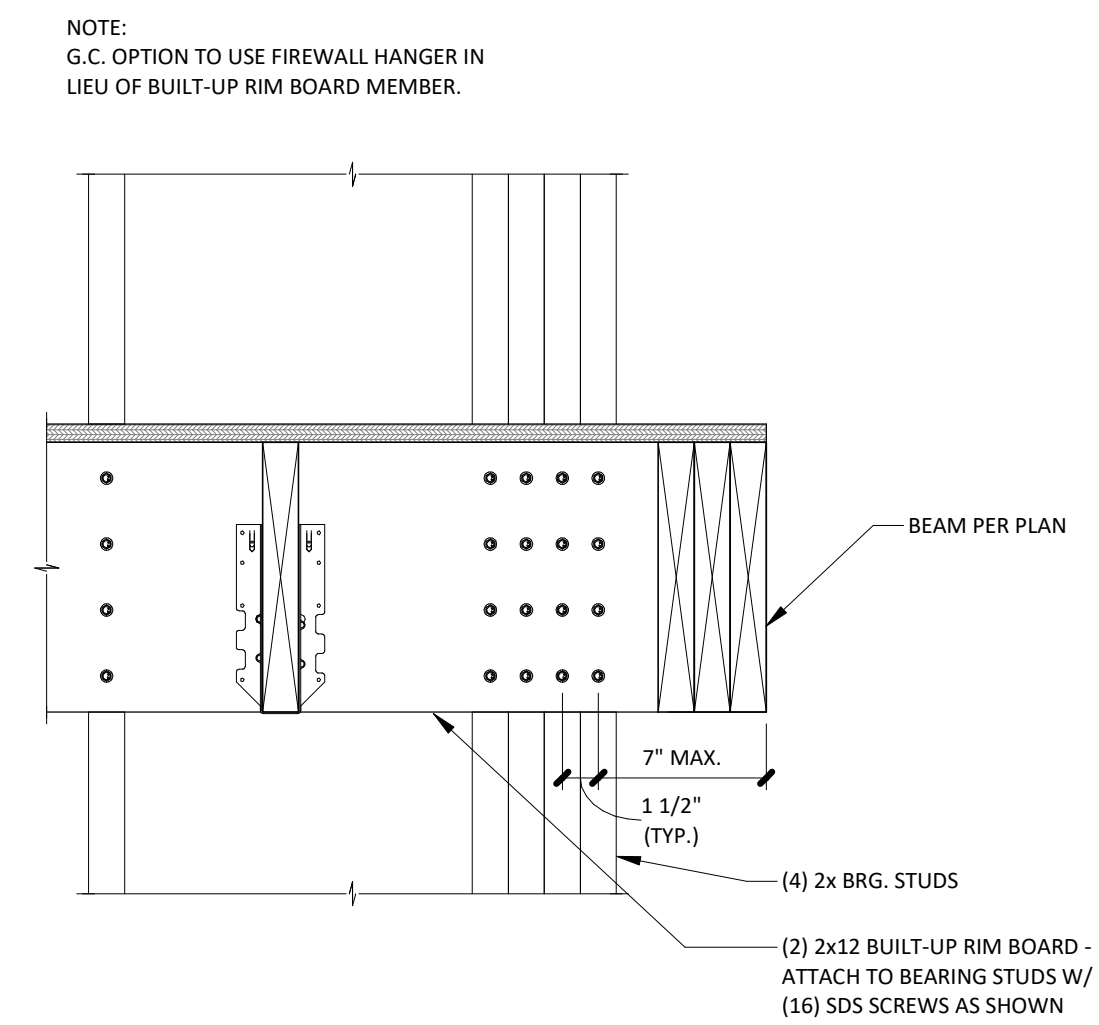
SECTION | 06

3/4" = 1'-0" S-210



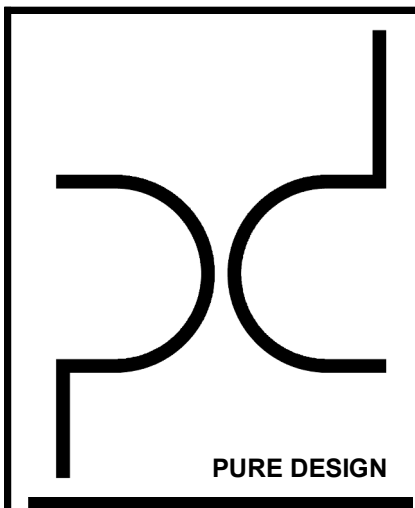
SECTION | 07

3/4" = 1'-0" S-210



SECTION | 08

1 1/2" = 1'-0" S-210



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LENEXA, KS 66214  
PHONE: 913.492.7400

**MEP ENGINEERS**  
PKMR  
13300 W. 98TH ST.  
LENEXA, KS 66215  
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**CIVIL ENGINEERS**  
Russell Planning & Engineering Inc.  
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**INTERIOR DESIGNER**  
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DUAL HOTEL DURANGO - AC & HIS

AC HOTELS

JOB NO: 18-007  
DATE: 2018.03.07  
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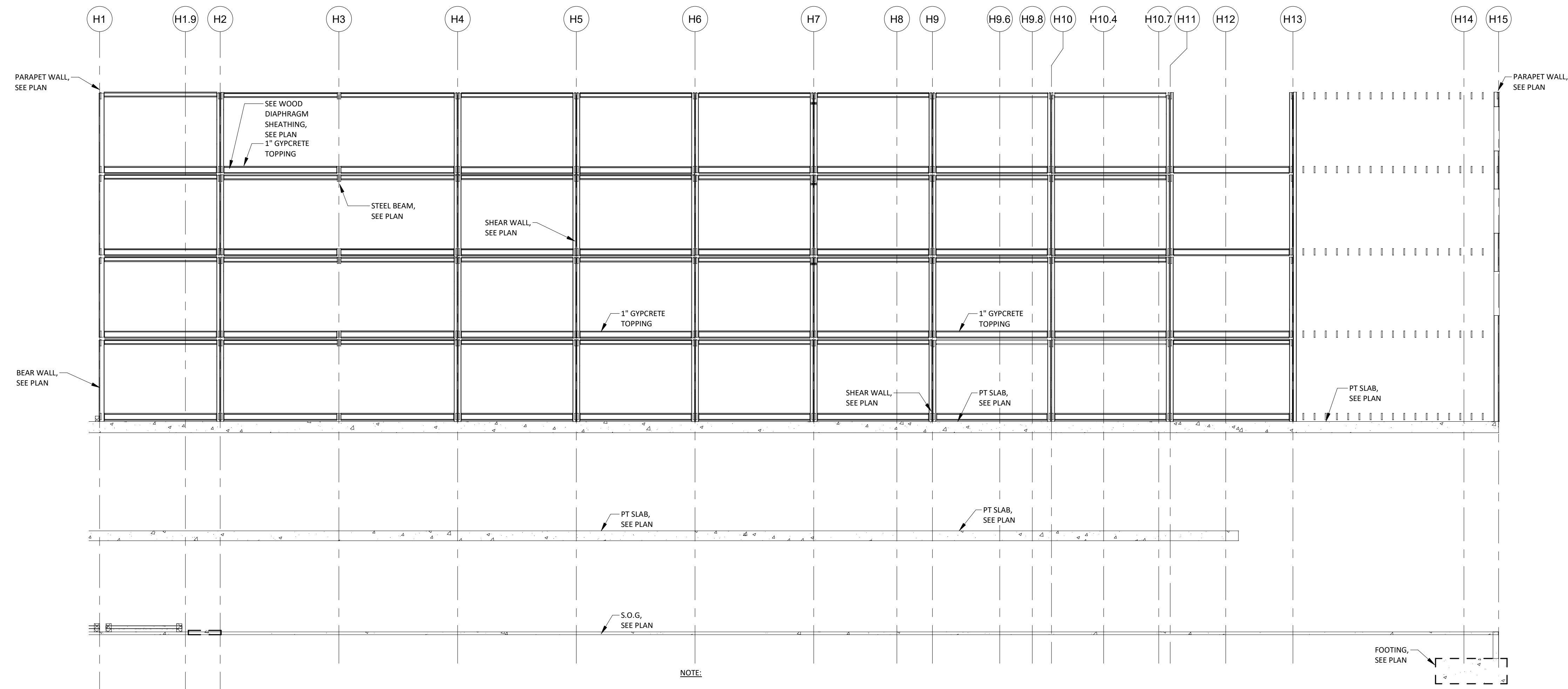
REVISIONS:

NEW 2019.06.14

STAIR FRAMING PLAN AND DETAILS

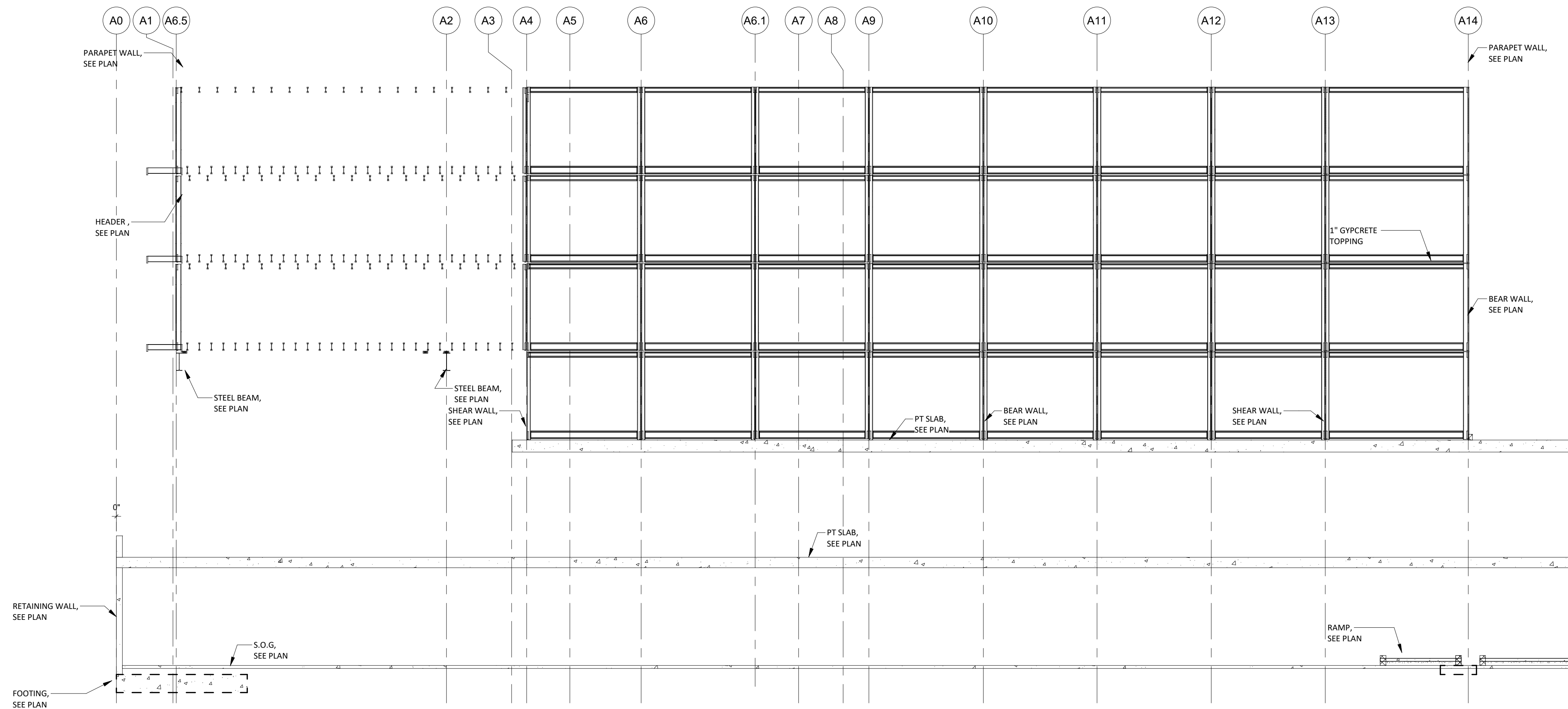
S-210

**BSE STRUCTURAL ENGINEERS**  
1132 West 79th Street  
Lenexa, Kansas 66214  
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Project Number 18-550



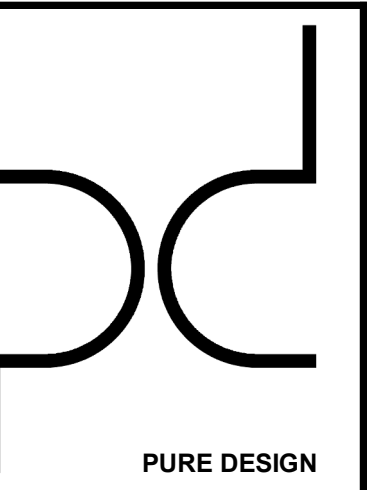
**NOTE:**  
 1.) BUILDING SECTIONS ARE SHOWN FOR SCHEMATIC PURPOSES ONLY. ACTUAL CONSTRUCTION TO MATCH CONSTRUCTION DOCUMENTS. REFERENCE ARCHITECTURAL, MECHANICAL, CIVIL, & STRUCTURAL DOCUMENTS.

01 SECTION  
 1/8" = 1'-0"



**NOTE:**  
 1.) BUILDING SECTIONS ARE SHOWN FOR SCHEMATIC PURPOSES ONLY. ACTUAL CONSTRUCTION TO MATCH CONSTRUCTION DOCUMENTS. REFERENCE ARCHITECTURAL, MECHANICAL, CIVIL, & STRUCTURAL DOCUMENTS.

02 SECTION  
 1/8" = 1'-0"



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**DUAL HOTEL DURANGO**  
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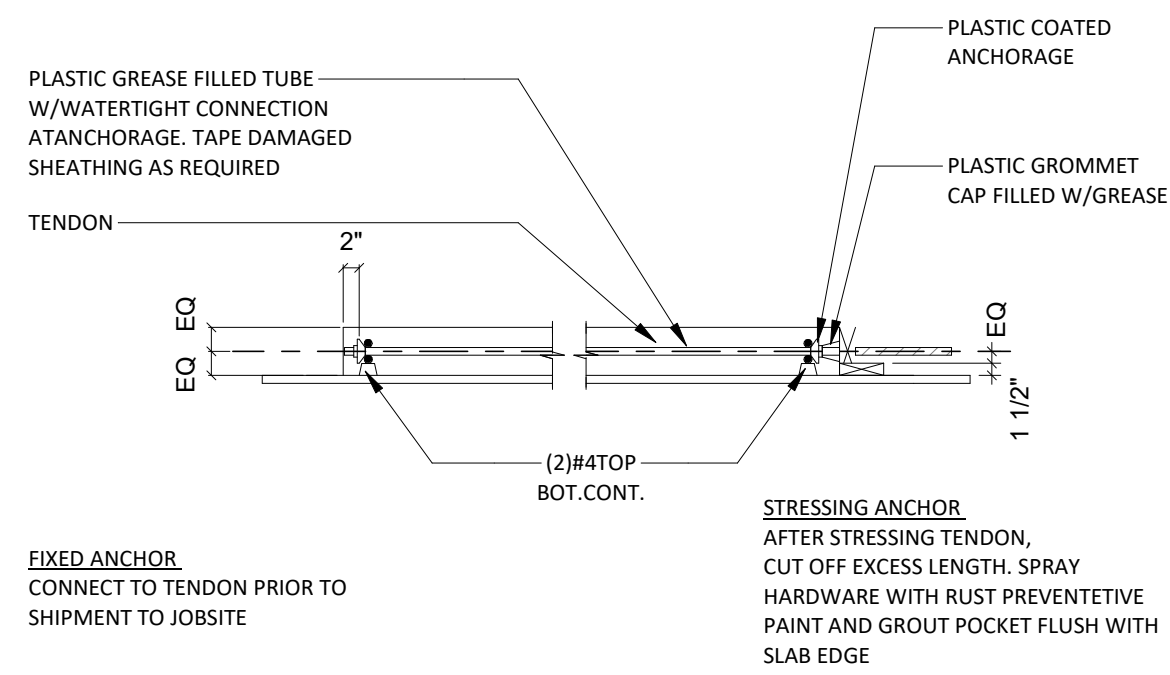


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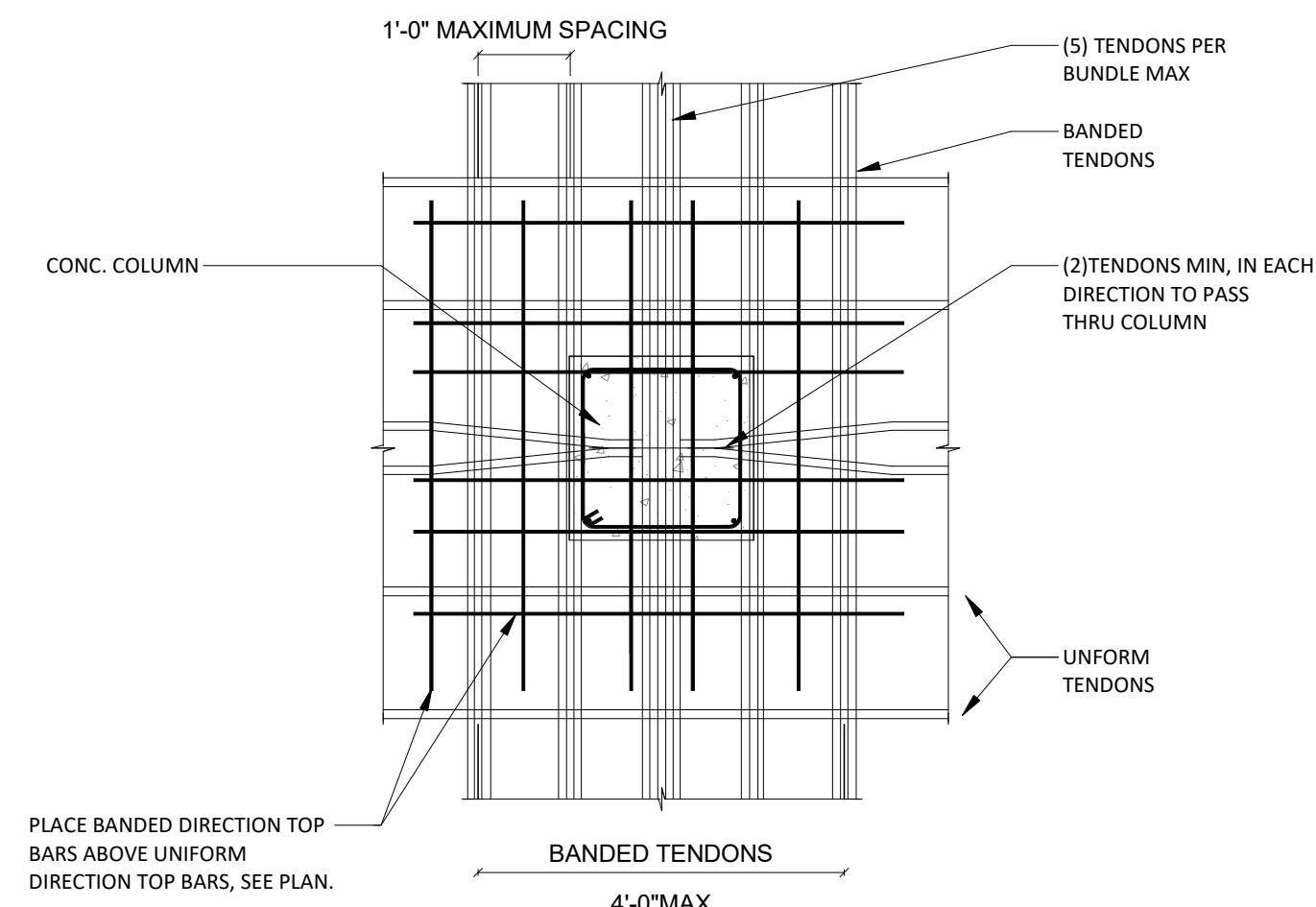
BUILDING SECTION

S-401

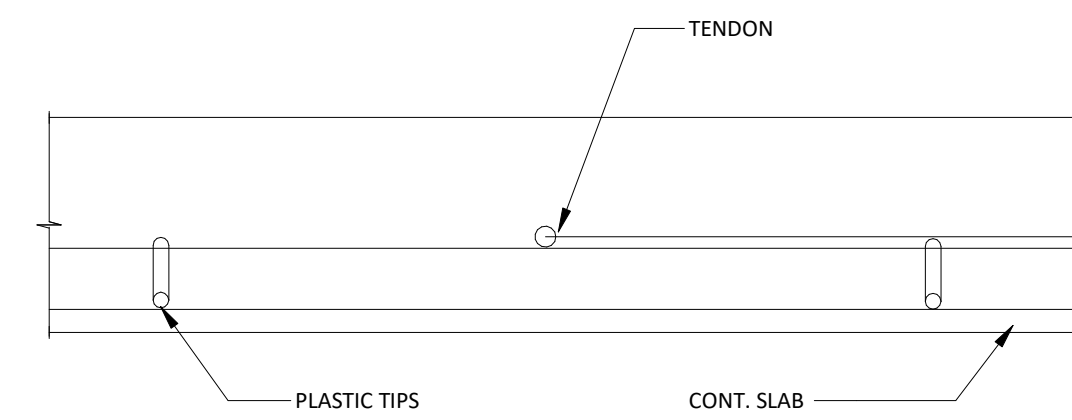




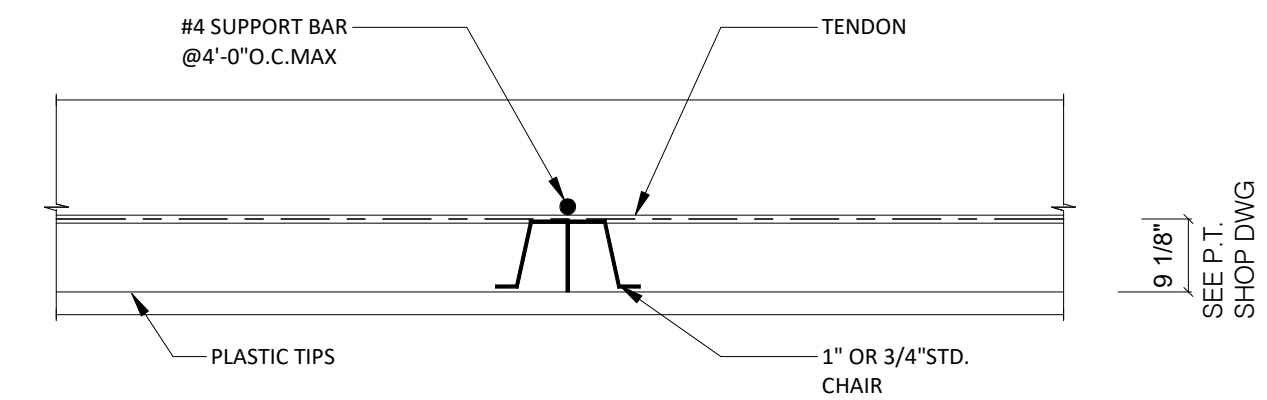
**TYP. POST TENSION ANCHOR DETAIL (ENCAPSULATED SYSTEM)**



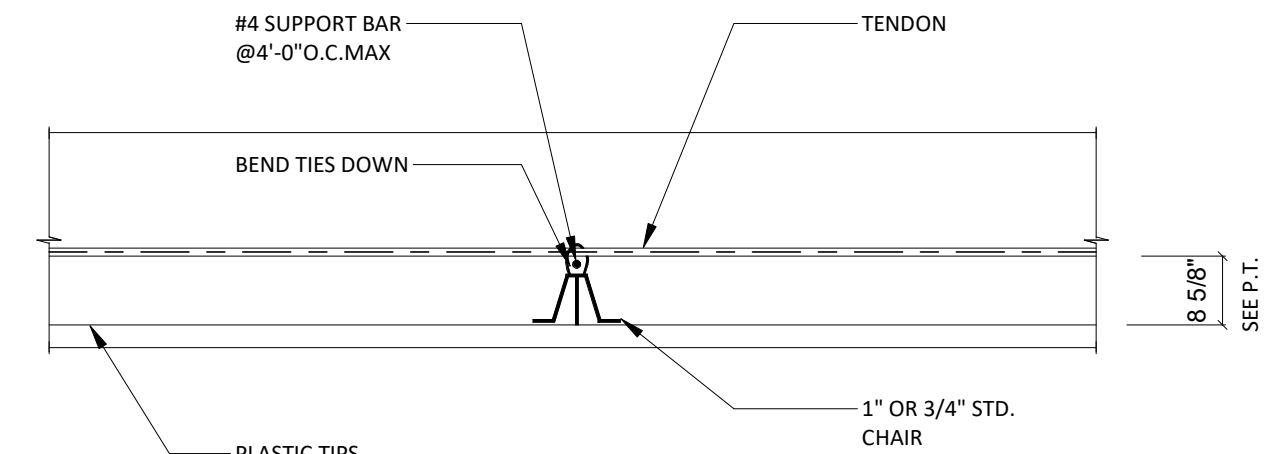
**SLAB REINFORCEMENT OVER COLUMNS**



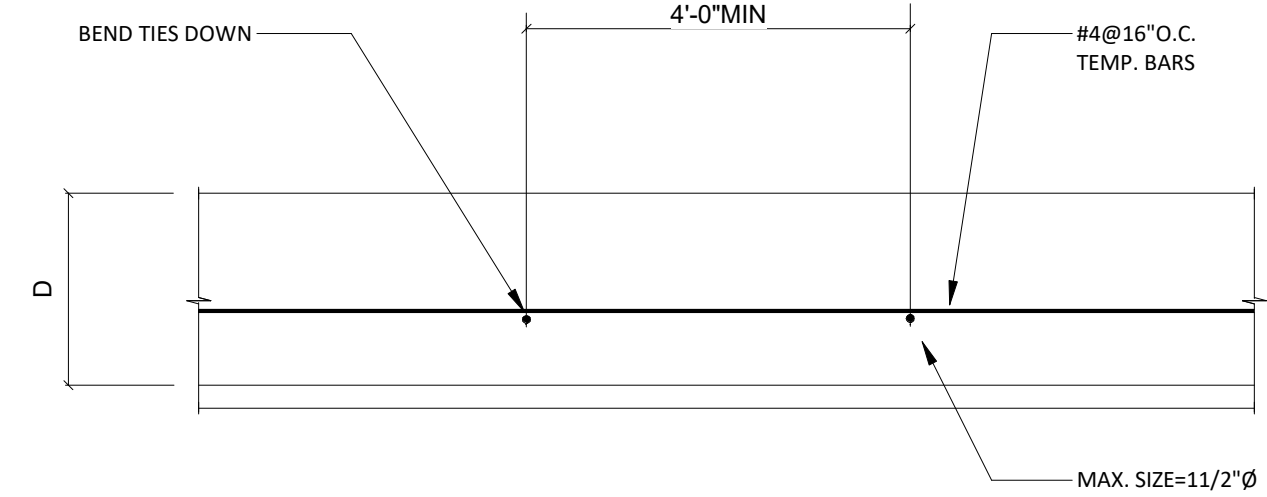
**TYP. SLAB BOLSTER SUPPORT**



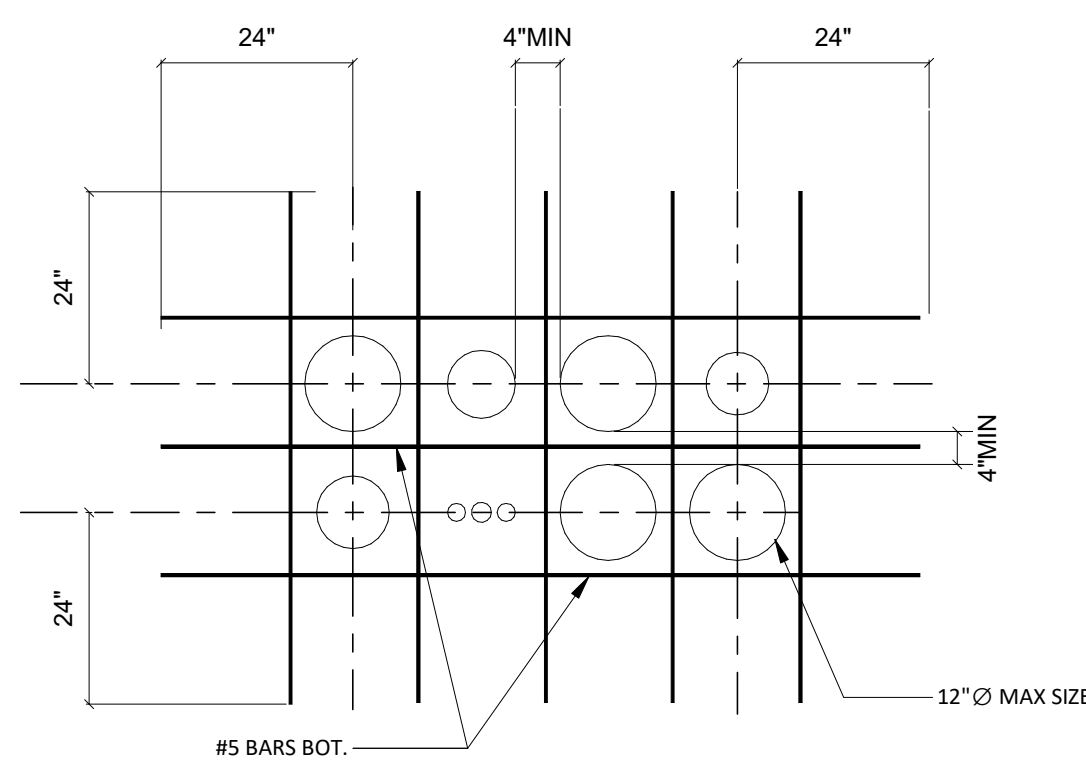
**TYP. CHAIR SUPPORT**



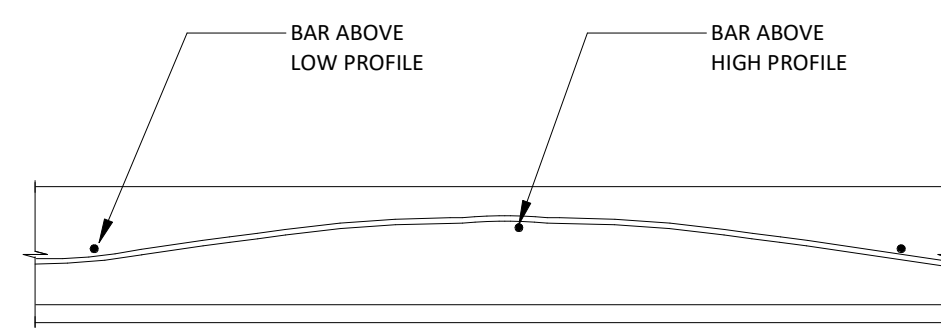
**TYP. HIGH CHAIR SUPPORT**



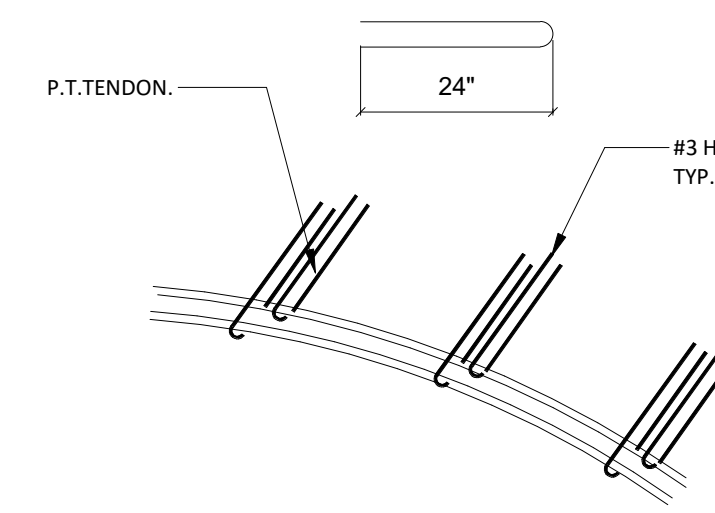
**TYP. IN-SLAB CONDUIT/PIPING**



**MULTIPLE SLAB PENETRATION DETAIL**



**TYP. TEMPERATURE STEEL PLACEMENT**



**TYP. HAIRPIN DETAIL AT CURVED B.T. TENDONS**

**POST-TENSIONING NOTES**

- TENDONS:**
- TENDON PROFILE**  
THE TENDON SHAPE IS A REVERSE PARABOLA. THE TENDON HEIGHTS ARE SHOWN ON EACH TENDON PLAN.  
a. REFERENCE HEIGHT IS FROM THE BOTTOM OF PT SLAB TO THE CENTROID OF TENDON.  
b. TENDONS ALONG AND OVER INTERIOR WALLS SHOULD BE LAID OUT FLAT (WITHOUT PROFILE) AT THEIR HIGH POINT; SEE DETAIL 27 a.  
c. BANDED TENDONS ALONG AN INTERIOR WALL MAY ALL BE PLACED FLAT AND AT THEIR HIGH POINTS EITHER OVER OR ADJACENT TO THE WALL.  
DISTRIBUTED TENDONS PARALLEL TO AN INTERIOR WALL SHOULD BE PLACED FLAT AND AT THEIR HIGH POINTS OVER A FRACTION OF THEIR TRIBUTARY AS INDICATED IN TYP. DETAIL 27 b.  
THE REMAINDER OF THE DISTRIBUTED TENDONS CAN BE TRANSITIONED BY GRADUAL MODIFICATION OF THEIR LOW POINTS TO FOLLOW THE PROFILE OF ADJACENT DESIGN STRIPS.
  - TENDONS ALONG EXTERIOR WALLS SHOULD BE PLACED FLAT AND ANCHORED AT THE CENTROID OF THE SLAB IN THE FIRST SPAN. TENDONS SHOULD BE ANCHORED AT THE CENTROID OF THE SLAB EVEN IF THERE IS A TRANSVERSE BEAM OR DROP CAP/PANEL AT THE SLAB EDGE. SEE DETAILS 28.**
  - TENDON SPACING**  
a. MAXIMUM SPACING OF DISTRIBUTED TENDONS IS THE LESSER OF EIGHT TIMES THE SLAB THICKNESS OR 5'.  
b. MINIMUM SPACING BETWEEN TENDONS IS A DISTANCE THAT IS LARGER THAN THE HORIZONTAL WIDTH (HW) OF THE BUNDLES (IF THE TENDON IS BUNDLED) (HW IS THE SUM OF THE DIAMETER OF EACH TENDON IN ONE BUNDLE), PLUS THE SUM OF THE WIDTHS OF THE DIAMETERS OF ALL REBARS PLACED BETWEEN THE TWO BUNDLES; SEE DETAILS OF TENDON SPACING IN SLAB 21.
  - PLACEMENT OF TENDONS**  
a. THE BANDED TENDONS ARE PLACED WITHIN THE WIDTH OF / ABOUT 4 FT. OVER THE SUPPORTS;  
b. THE UNIFORMLY DISTRIBUTED TENDONS ARE UNIFORMLY PLACED WITHIN THE ENTIRE TRIBUTARY WIDTH OF A DESIGN STRIP OF A COLUMN LINE. THE TRIBUTARY WIDTH OF THE COLUMN LINE IS DEFINED AS HALF OF THE ADJACENT SPAN OF EACH SIDE OF THE COLUMN LINE;  
c. MINIMUM TENDONS OVER SUPPORTS:  
A MINIMUM OF TWO TENDONS IN EACH DIRECTION MUST PASS DIRECTLY OVER EACH SUPPORT, REGARDLESS OF HOW MANY STRANDS EACH TENDON CONTAINS.
  - PLACEMENT OF TERMINATED TENDONS:**  
THE ADDED OR DISCONTINUOUS TENDONS ARE RECOMMENDED TO BE STAGGERED 12" WHEN THEY TERMINATE AS A GROUP AT ONE LOCATION. SEE TYPICAL DETAIL OF PLACEMENT OF TERMINATED TENDONS.

**NON-PRESTRESSED REINFORCEMENT:**

- TOP REBARS**  
THE TOP REBARS FOR BOTH DIRECTIONS MUST BE PLACED OVER THE COLUMN SUPPORTS WITHIN A DEFINED BAND IN EACH DIRECTION. THE BAND IS DEFINED AS ONE QUARTER OF THE ADJACENT SPAN OF EACH SIDE OF THE COLUMN. THE TOP REBARS FOR EACH DIRECTION ARE TYPICALLY PLACED PERPENDICULAR TO EACH OTHER.
- BOTTOM REBARS**  
a. THE BOTTOM REBARS ARE UNIFORMLY DISTRIBUTED WITHIN THE ENTIRE TRIBUTARY WIDTH OF A COLUMN LINE IN UNIFORMLY DISTRIBUTED TENDON DIRECTION. THE TRIBUTARY WIDTH HAS BEEN DEFINED PREVIOUSLY, AND ARE TYPICALLY PLACED ALONG THE LINE OF SUPPORT (COLUMN).  
b. IN END SPANS, EXTEND ONE-THIRD OF THE BOTTOM BARS TO THE SUPPORTS. IN INTERIOR SPANS, EXTEND ONE QUARTER OF THE BOTTOM BARS TO THE SUPPORT.
- TERMINATION OF REBARS**  
FOR ALL CONDITIONS, ADJACENT REBARS ARE RECOMMENDED TO BE STAGGERED 12".  
SEE TYP. DETAILS 23.

**SUPPORT BARS:**

- FOR DISTRIBUTED TENDONS**  
THE #4 BARS ARE USED, PLACED NOT MORE THAN 4' APART, AND LAPPED 15". APART FROM THE BARS USED TO SECURE TENDONS OVER THE COLUMN SUPPORT LINES, A MINIMUM OF 5 SUPPORT BARS ARE TYPICALLY PROVIDED FOR EACH SPAN.
- FOR BANDED TENDONS**  
THE BANDED TENDONS ARE SUPPORTED BY SHORT BARS 3" LONGER THAN THE WIDTH OF THE BAND (4'). AT THE SIDES OF EACH COLUMN, TYPICALLY A HEAVIER BAR IS USED (#7). AWAY FROM THE COLUMNS, #4 BARS ARE USED, THE SUPPORT BARS ARE PLACED AT ABOUT 3'-6" INTERVALS.  
SEE TYP. DETAILS 24.

**TEMPERATURE AND SHRINKAGE REINFORCEMENT:**

FOR THE WEDGE-SHAPED REGIONS BETWEEN THE TWO SUPPORT LINES IN THE BANDED TENDON DIRECTION, NONPRESTRESSED REBARS ARE PLACED IN THE DIRECTION OF THE BANDED TENDONS, AND ARE ALTERNATED SO THAT HALF ARE PLACED AT THE TOP AND HALF AT THE BOTTOM OF THE SLAB.  
SEE TYP. DETAILS 25.

**TRIM REBARS AT RE-ENTRANT CORNERS:**

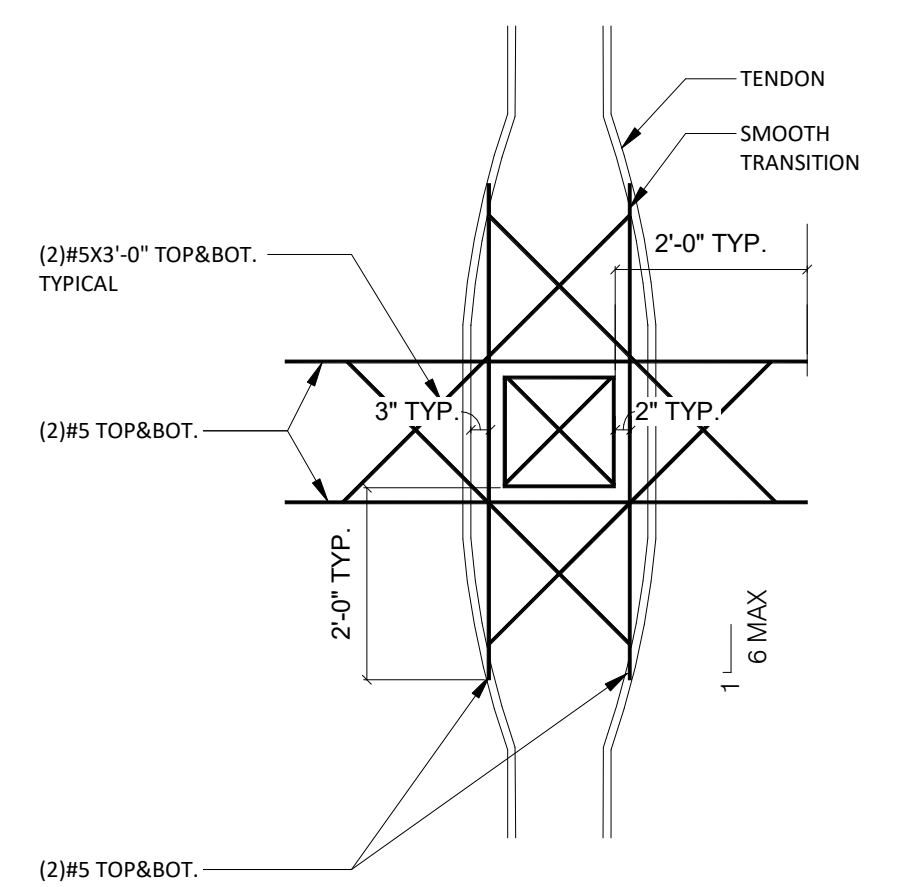
AT LOCATIONS OF STRESS CONCENTRATION SUCH AS RE-ENTRANT CORNERS. NON-PRESTRESSED REBARS MUST BE ADDED TO REDUCE CRACK WIDTH.  
SEE TYP. DETAIL 26 ON.

**OTHERS:**

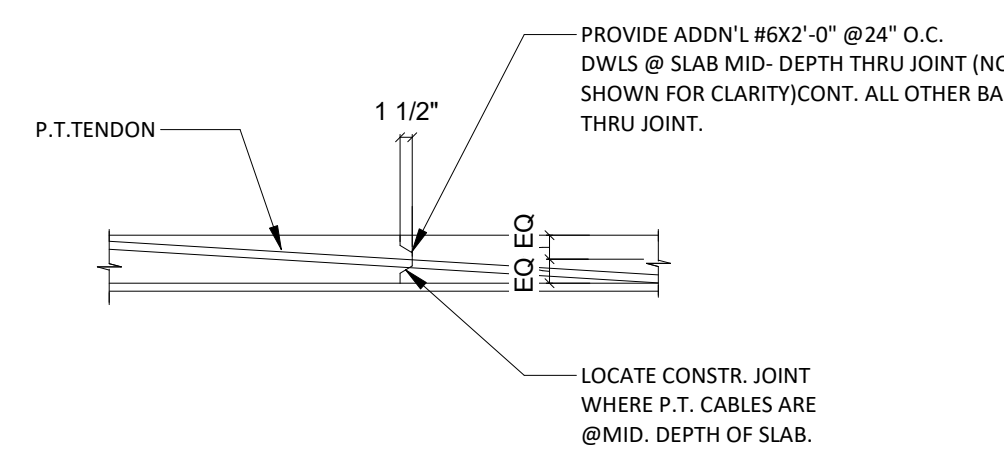
- REFER TO ARCH. & MEP DWGS FOR OPENING DIMENSIONS NOT SHOWN ON THE PLAN.
- REFER TO ARCH. DWGS FOR ELEVATION.
- U.N.O. DENOTES "UNLESS NOTED OTHERWISE".
- "C.G." DENOTES "CENTER OF GRAVITY OF STRAND (TENDON)".

**LEGEND:**

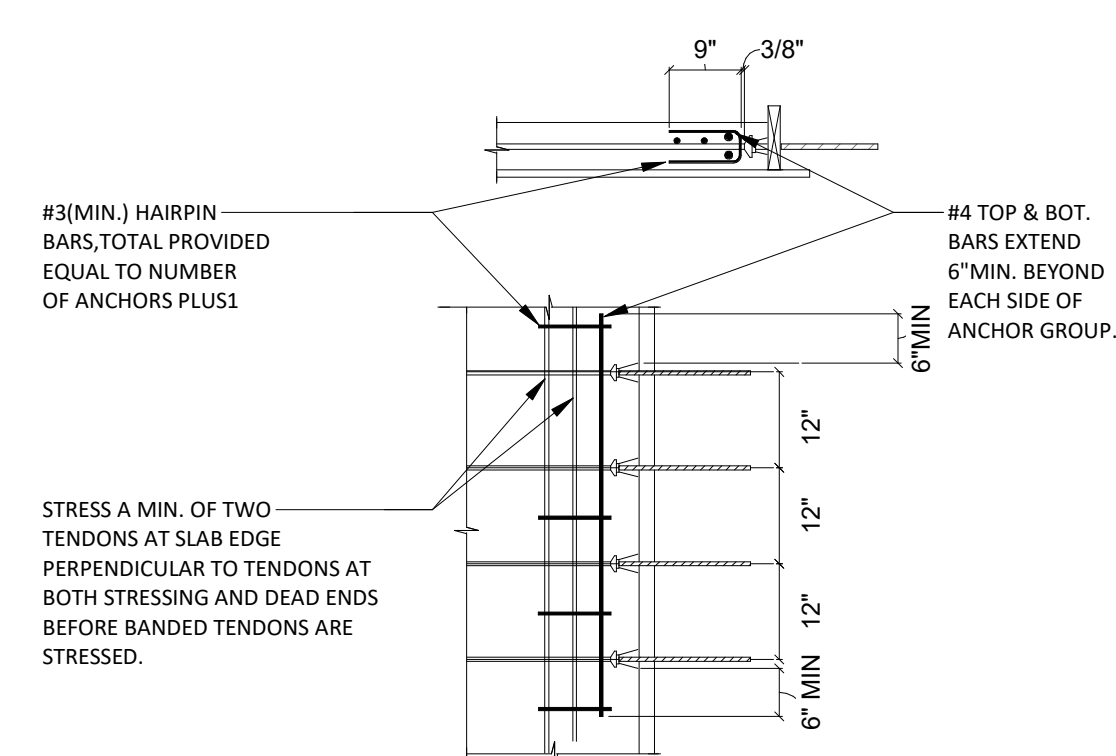
- "- - -" DENOTES "STRESSING END".
- "- - -" DENOTES "DEAD END".
- "- - -" DENOTES "UNIFORM".
- "- - -" DENOTES "BANDED".



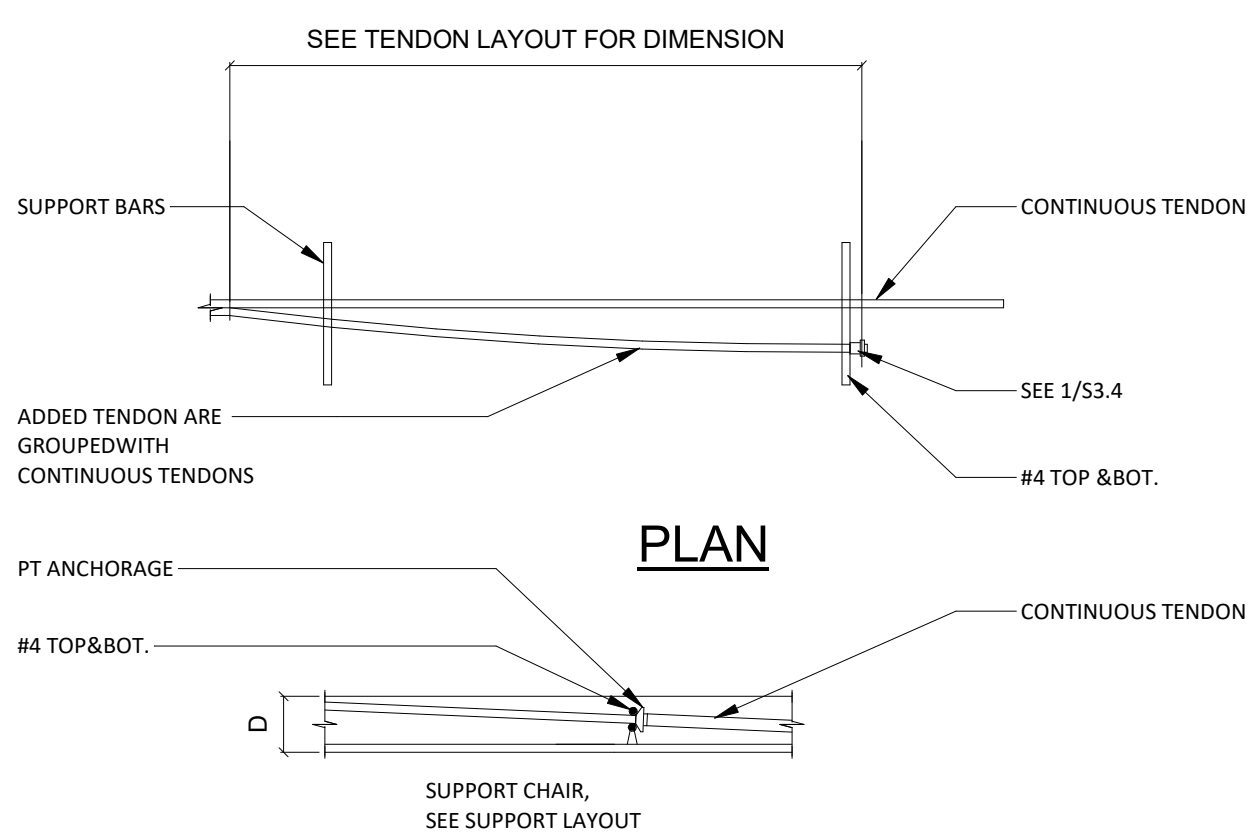
**TYP. SLAB OPENING**



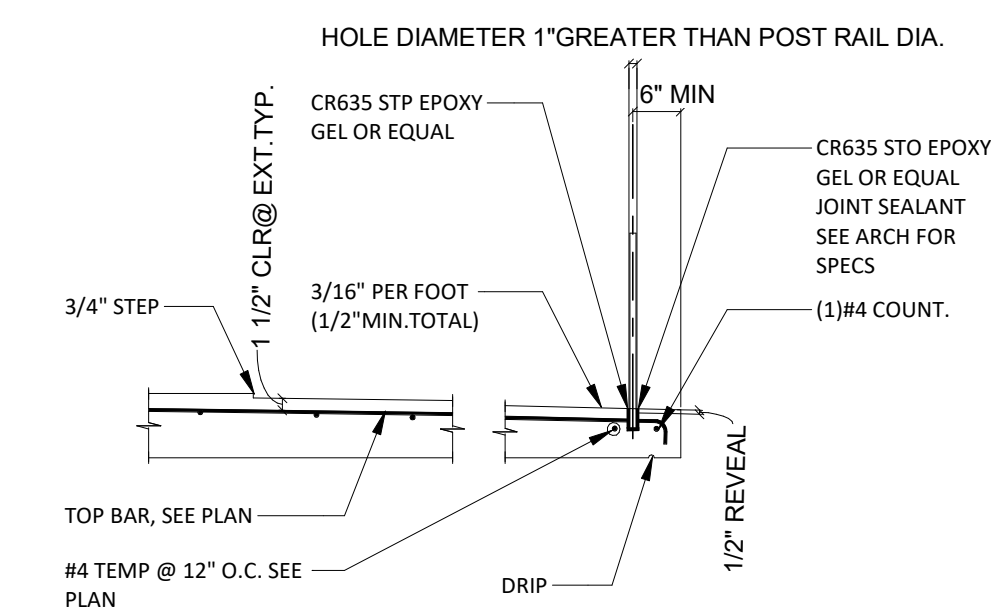
**SLAB CONSTRUCTION JOINT**



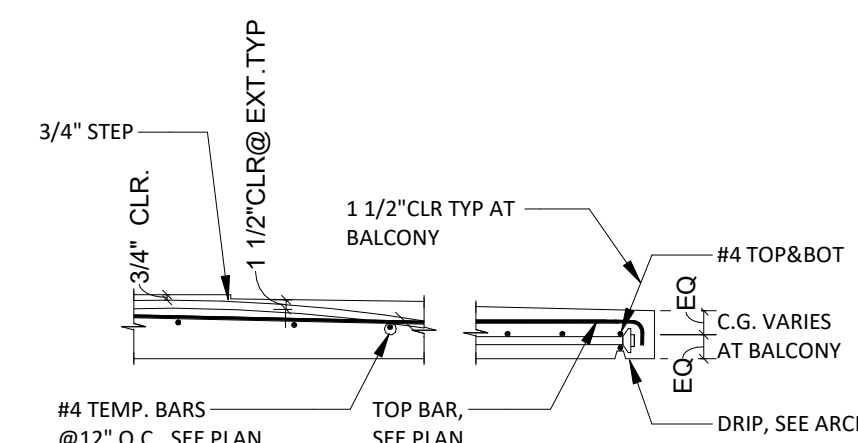
**ANCHORAGE ZONE REINF.**



**ADDED STRAND**



**CONCRETE BALCONY**



**TYP. BALCONY DETAIL**

**PURE DESIGN**

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**DUAL HOTEL DURANGO - AC & HIS**

AC HOTELS

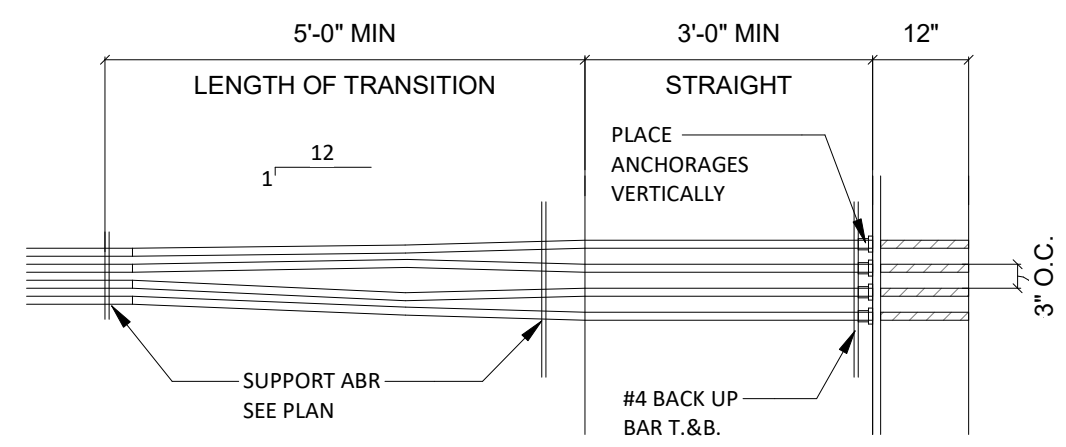
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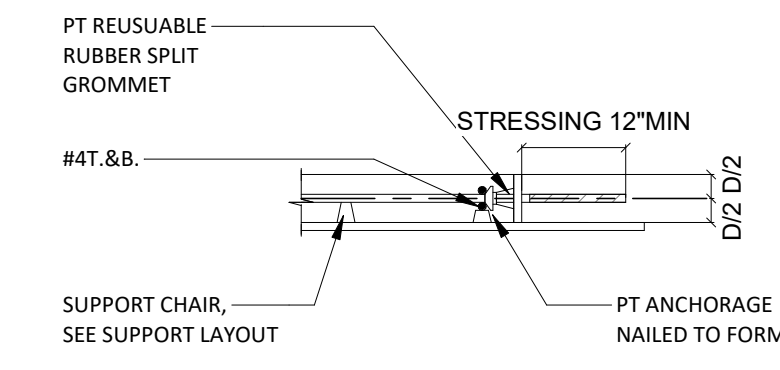
TYPICAL DETAIL

**S-601**

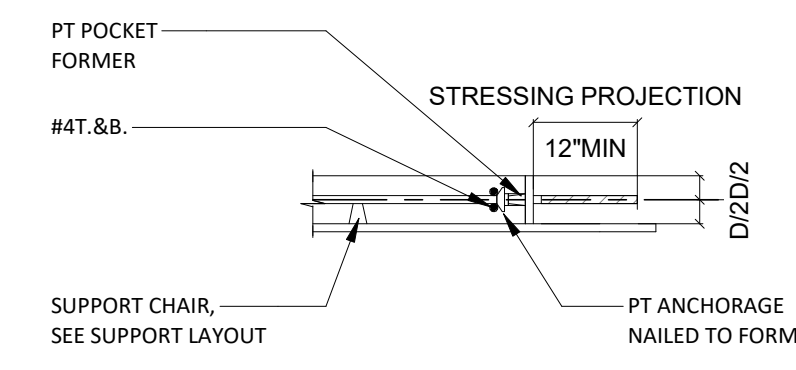
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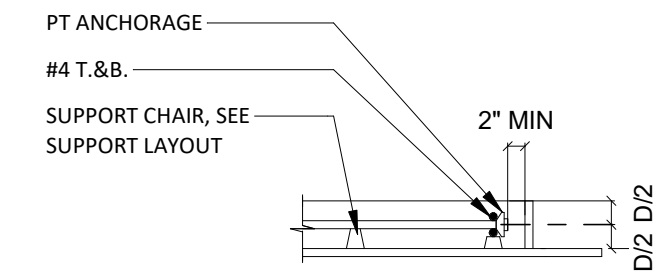
**FLARE AT ANCHORAGES** 16  
SLAB THICKNESS=7" OR MORE



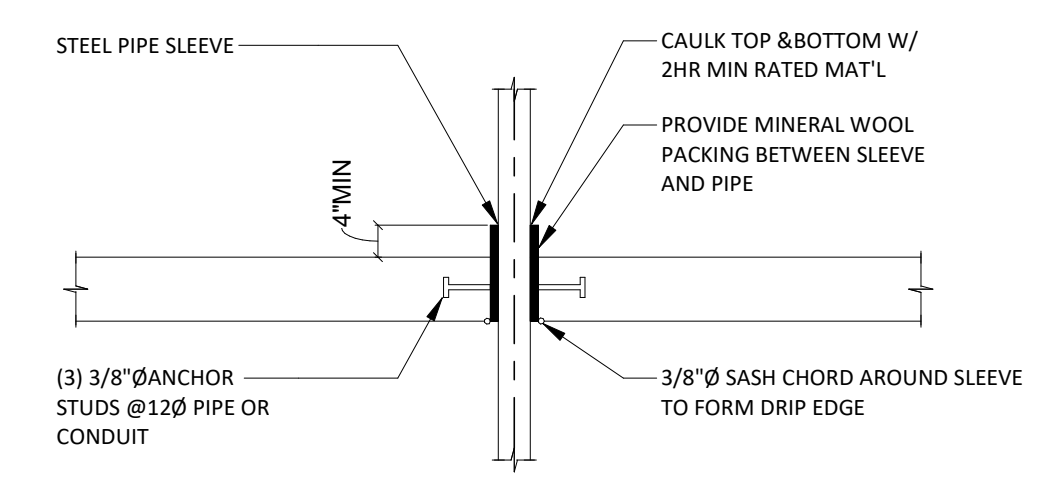
**INTERMEDIATE ANCHORAGE** 17  
SLAB THICKNESS=8" OR MORE  
NOTES:  
1. PIPE SLEEVE IS TO BE (2) PIPE SIZES LARGER FOR LINES UP TO 2" Ø  
2. PIPE SLEEVE IS TO BE (1) PIPE SIZE LARGER FOR LINES GREATER THAN 2" Ø



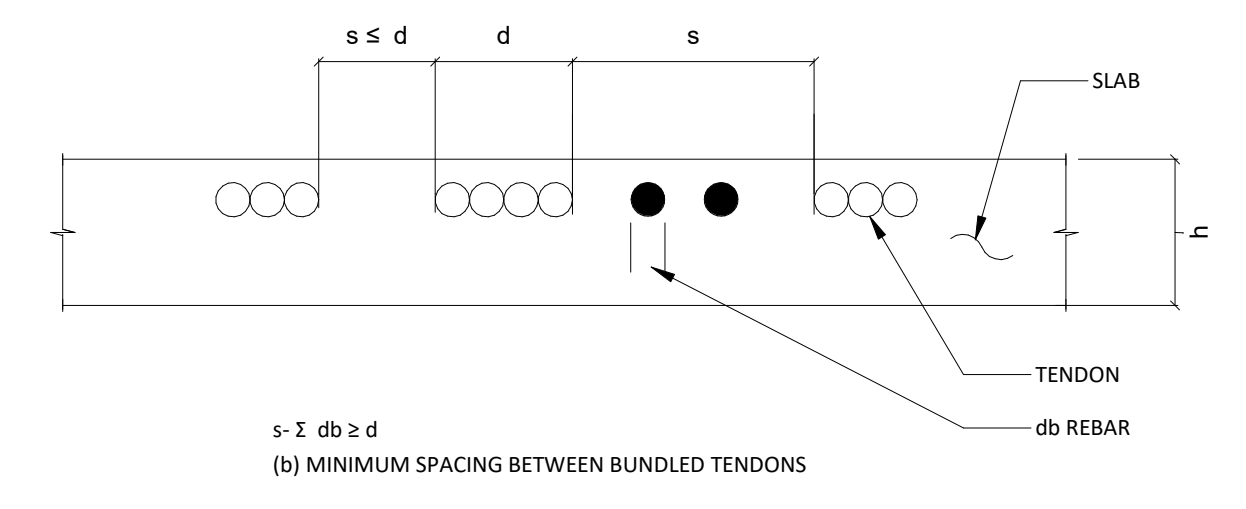
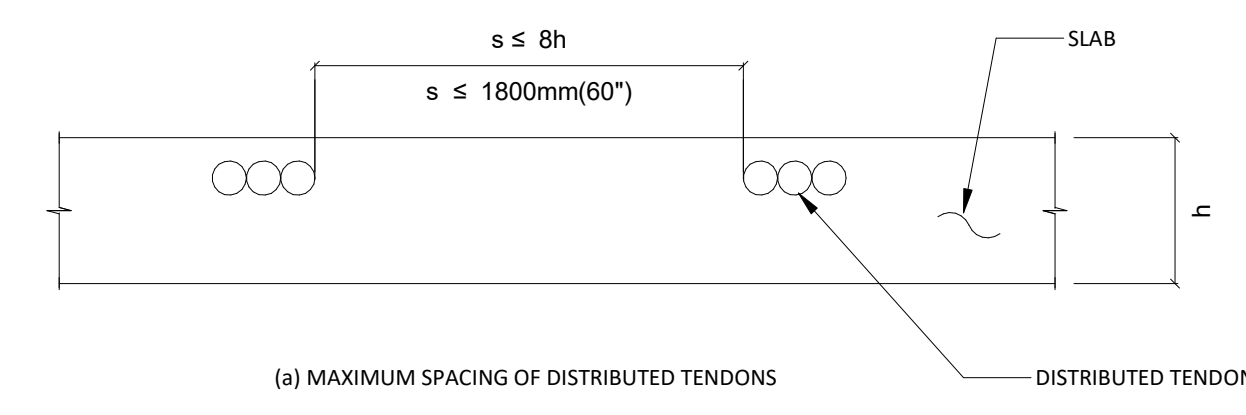
**STRESSING ANCHORAGE** 18  
SLAB THICKNESS=7" OR MORE



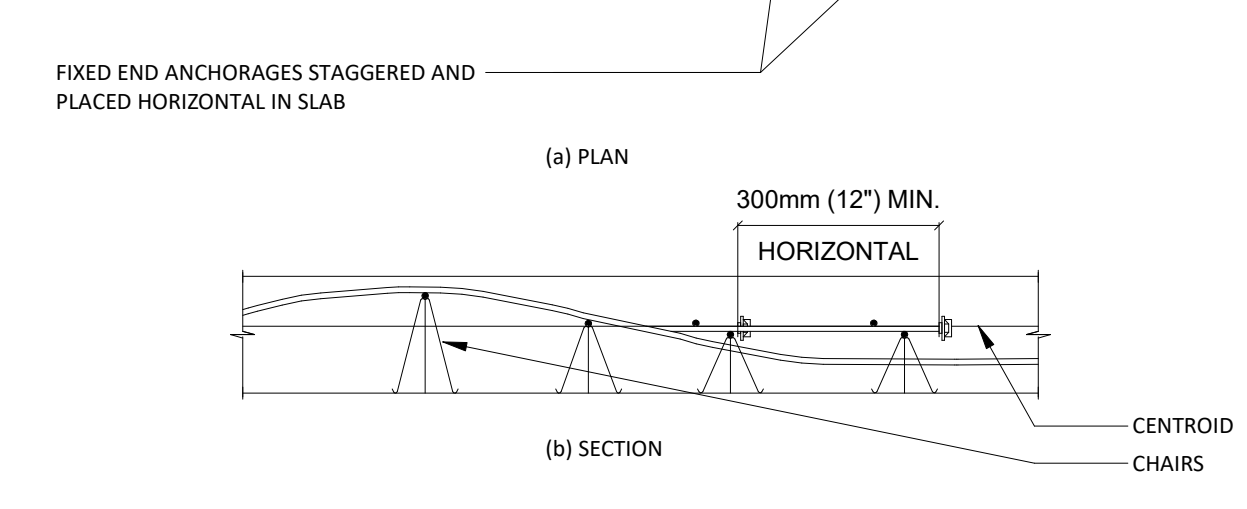
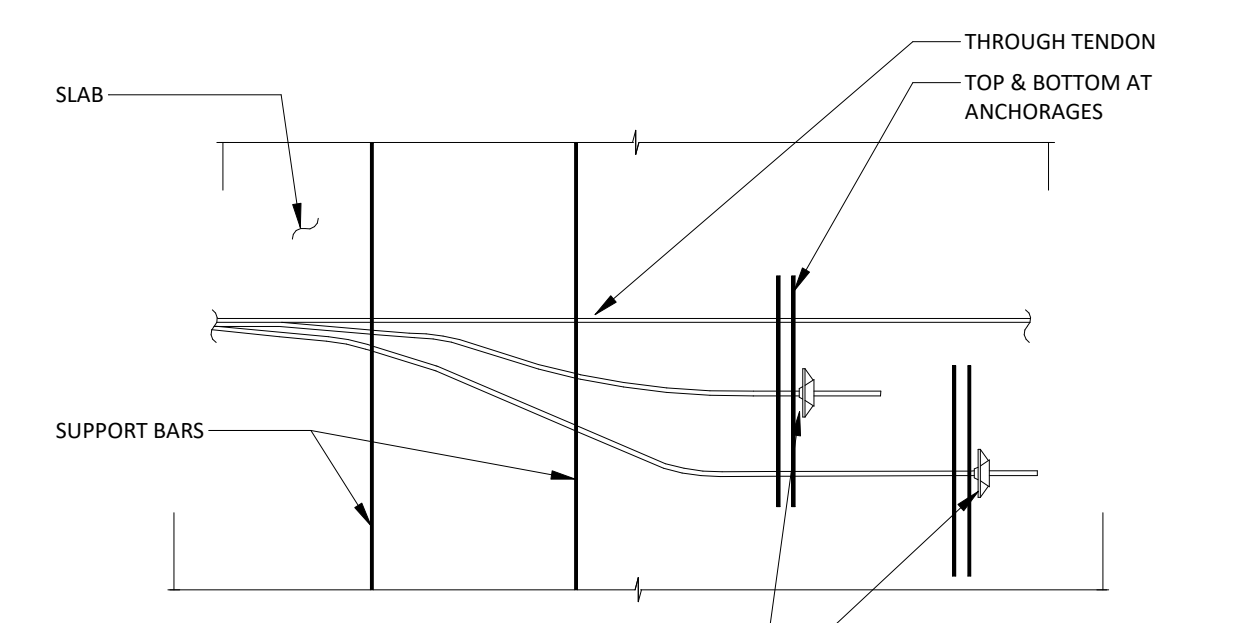
**DEAD END ANCHORAGE** 19  
SLAB THICKNESS=7" OR MORE



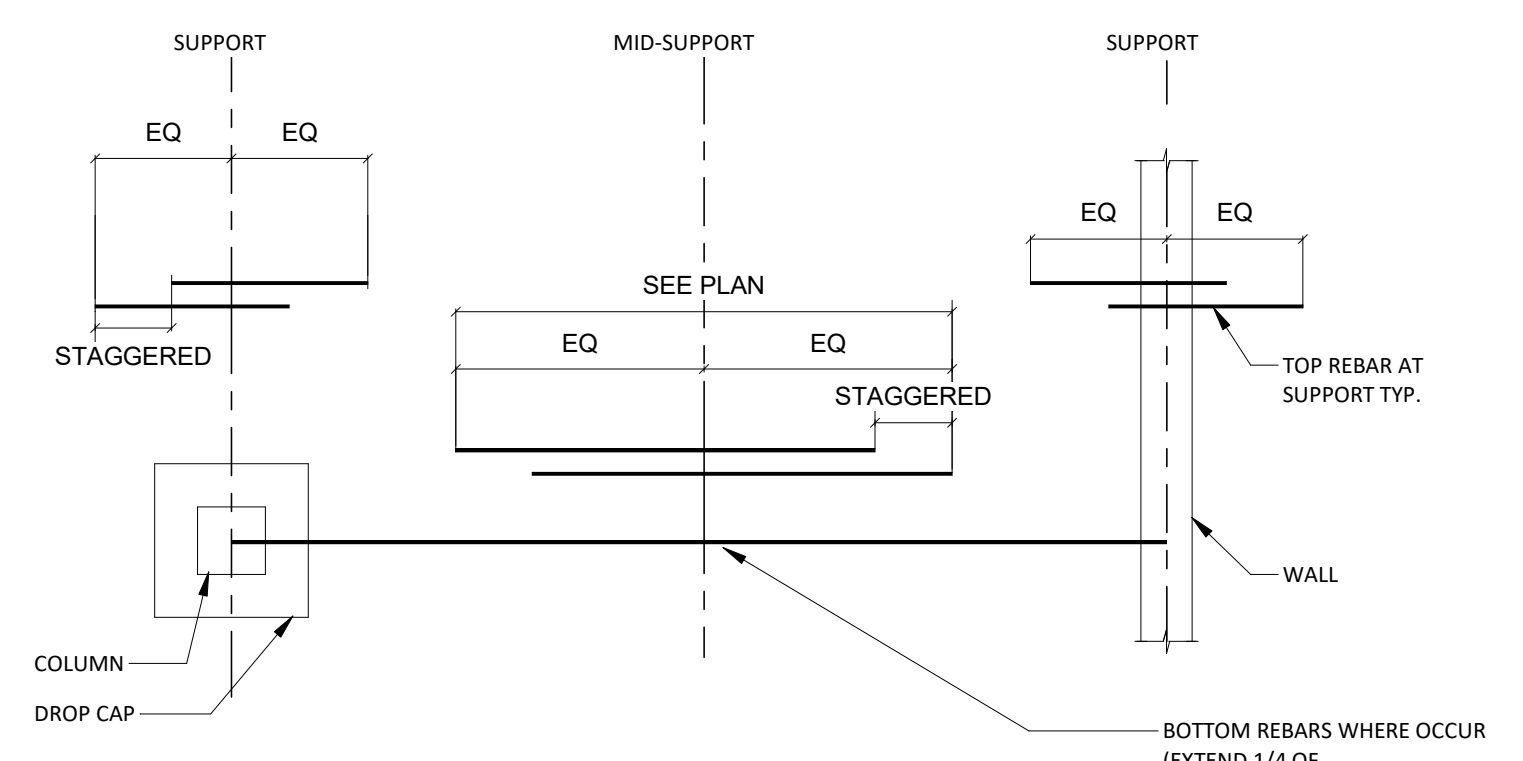
**TYP. SLEEVE DETAIL** 20



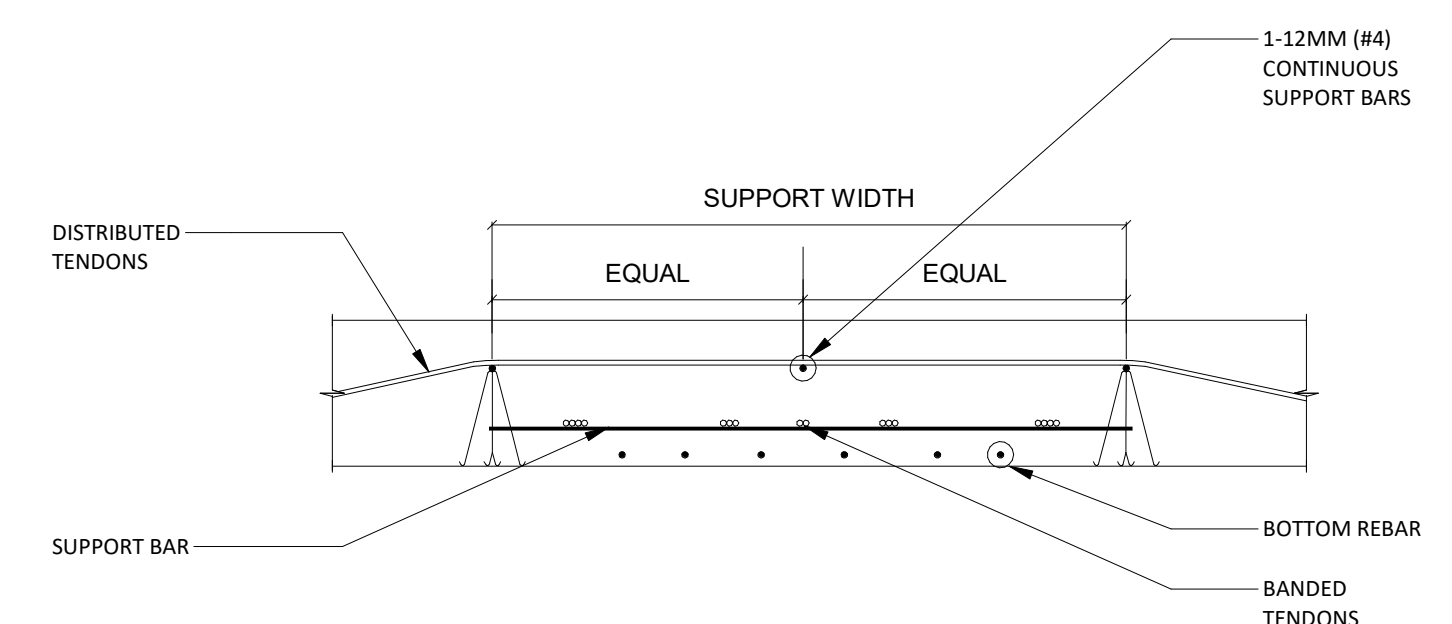
**TYP. TENDON SPACING IN SLABS** 21



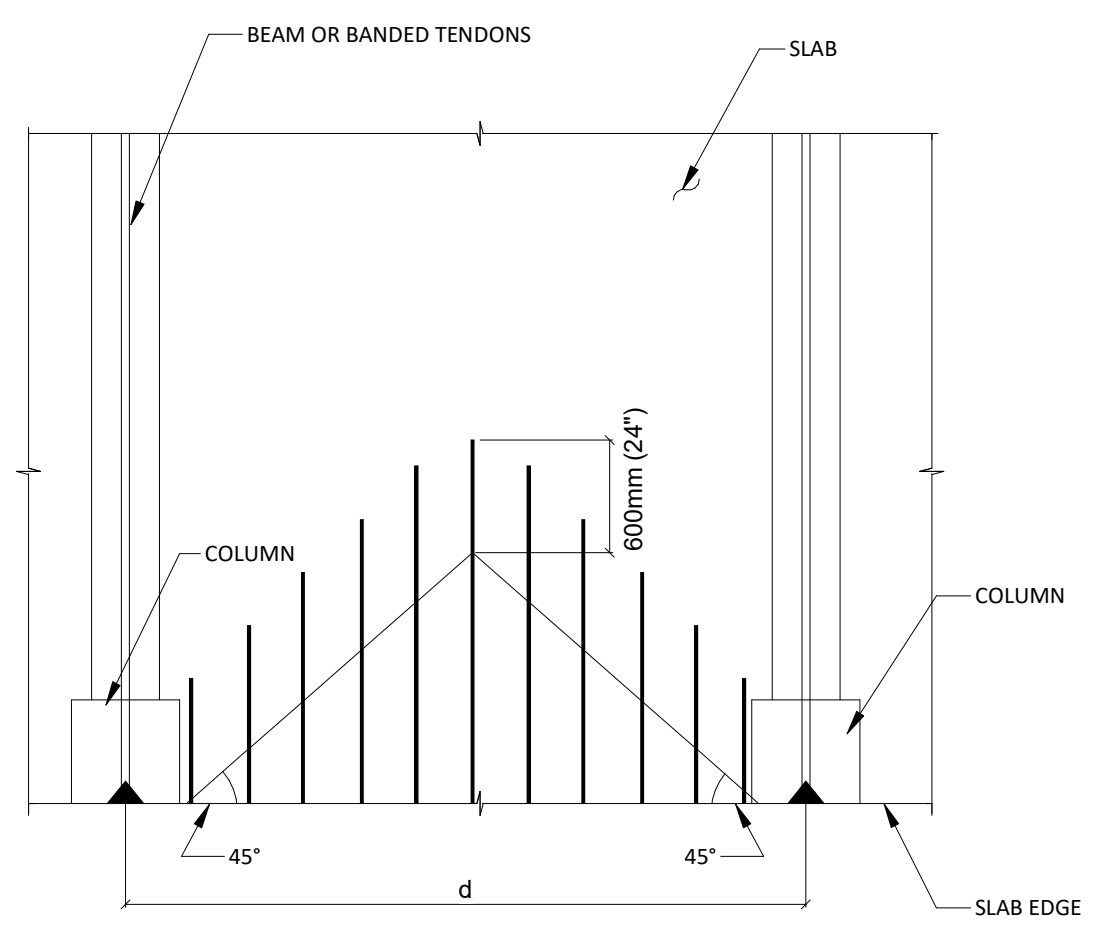
**TYP. PLACEMENT OF TERMINATED TENDONS** 22



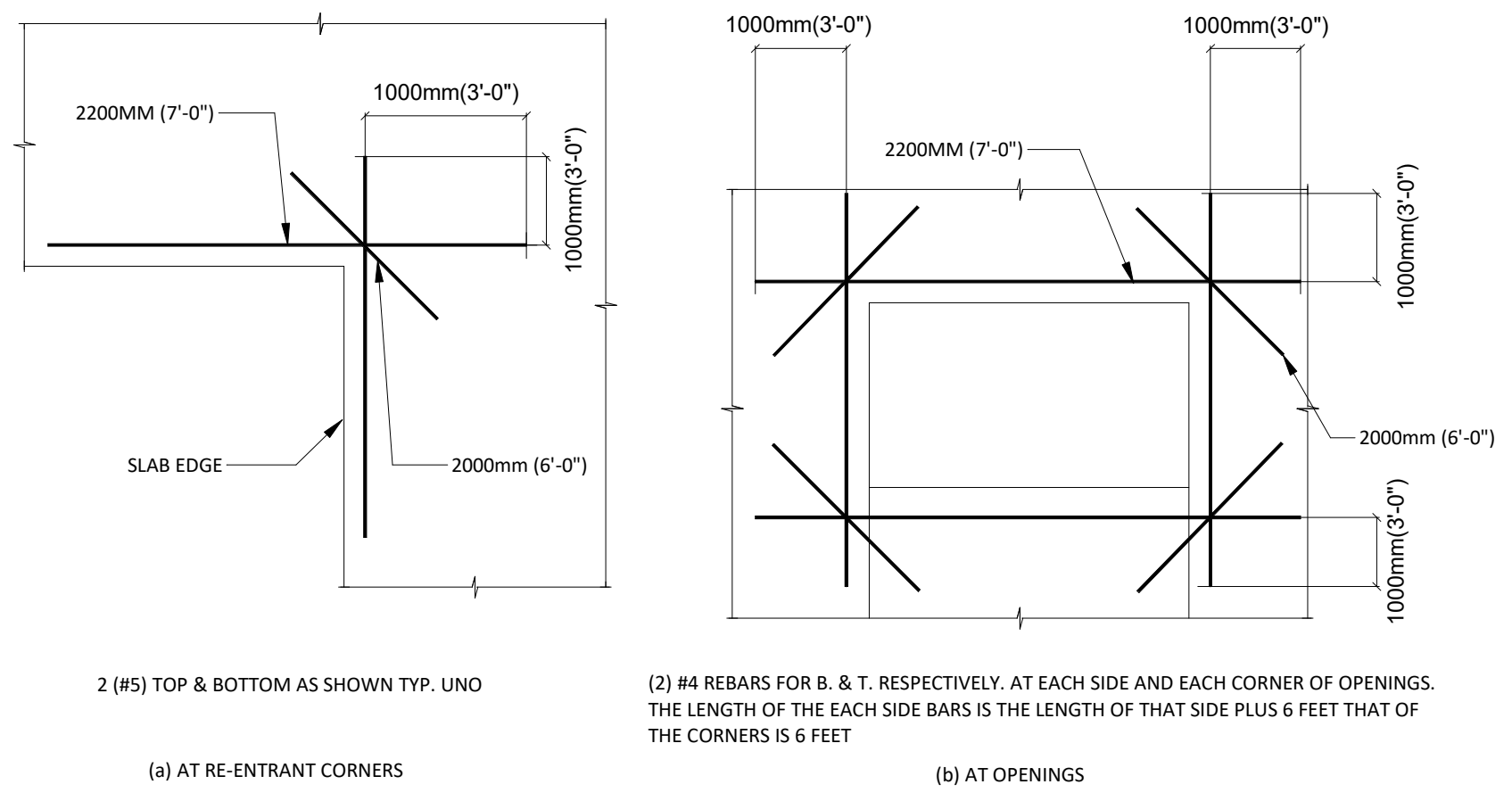
**TYP. ARRANGEMENT OF BARS IN SLAB** 23



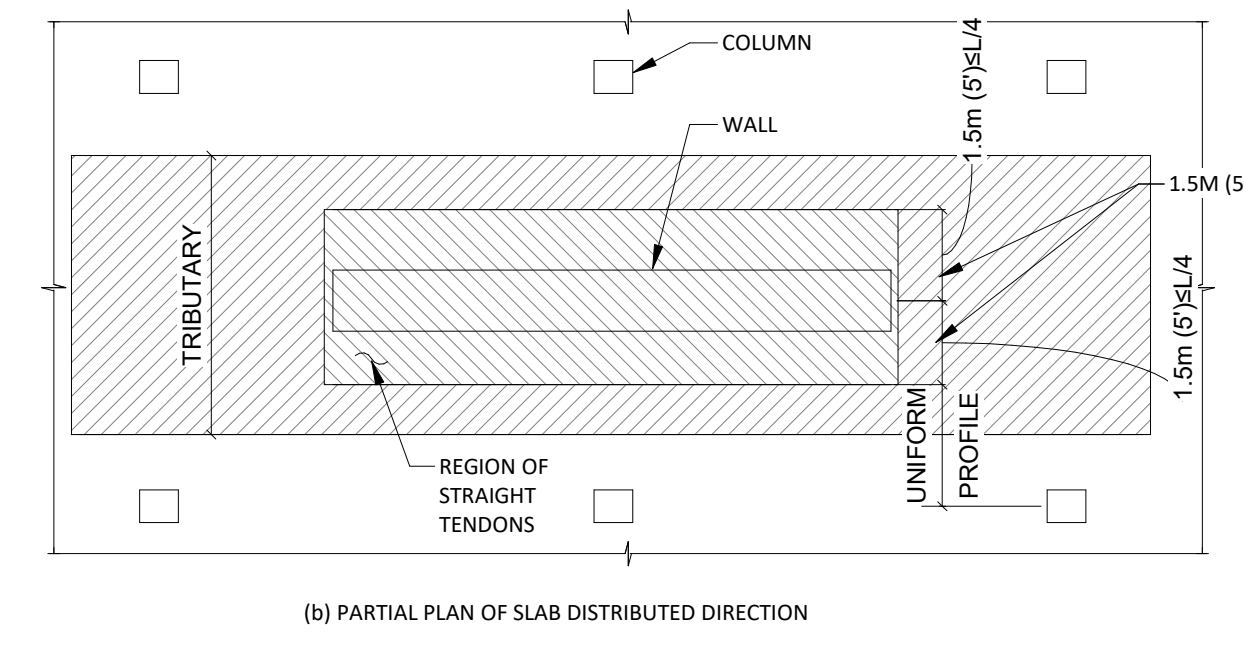
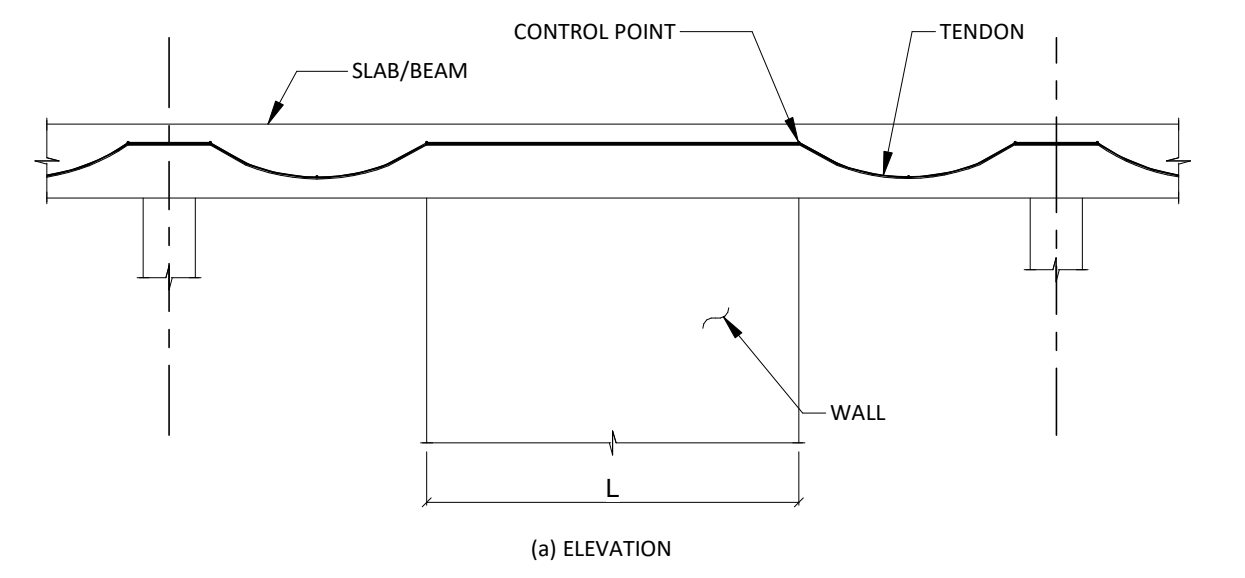
**TYP. SECTION THROUGH BANDED TENDONS** 24



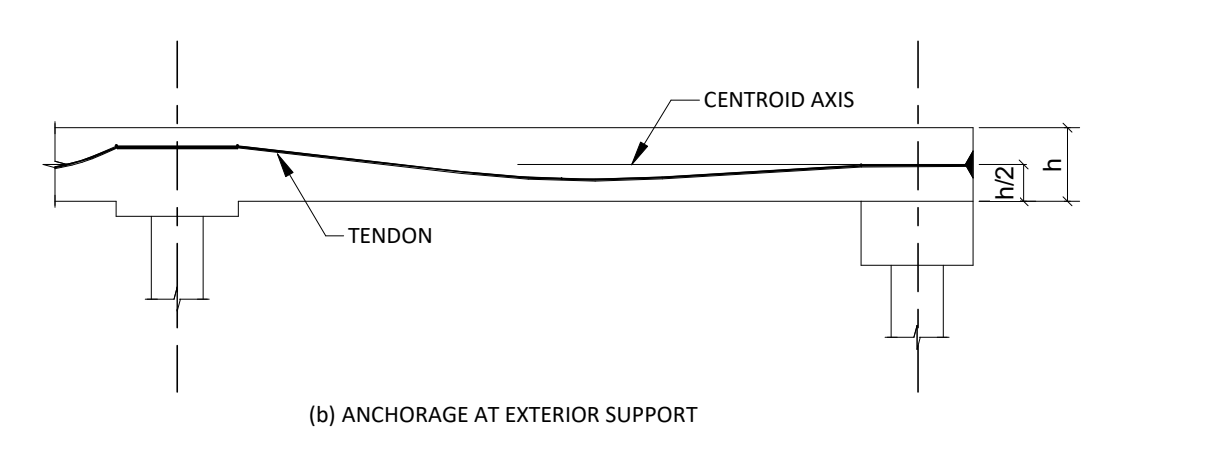
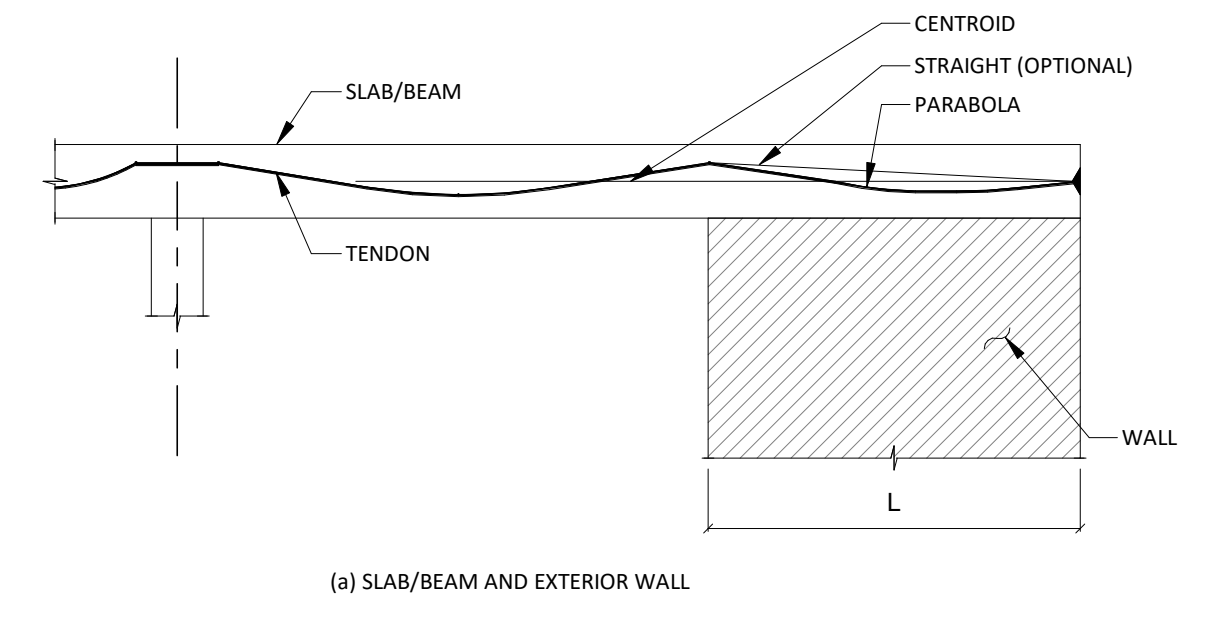
**TYP. ARRANGEMENT OF TEMPERATURE AND SHRINKAGE REINFORCEMENT** 25



**TYP. TRIM BARS AT RE-ENTRANT CORNERS AND OPENINGS** 26



**TYP. TENDONS ALONG OR OVER INTERIOR WALLS SHOULD BE LAID FLAT (WITHOUT PROFILE) AT THEIR HIGH POINT** 27



**TYP. TENDONS ALONG EXTERIOR WALLS SHOULD BE PLACED FLAT AND ANCHORED AT THE CENTROID OF THE SLAB IN THE FIRST SPAN** 28

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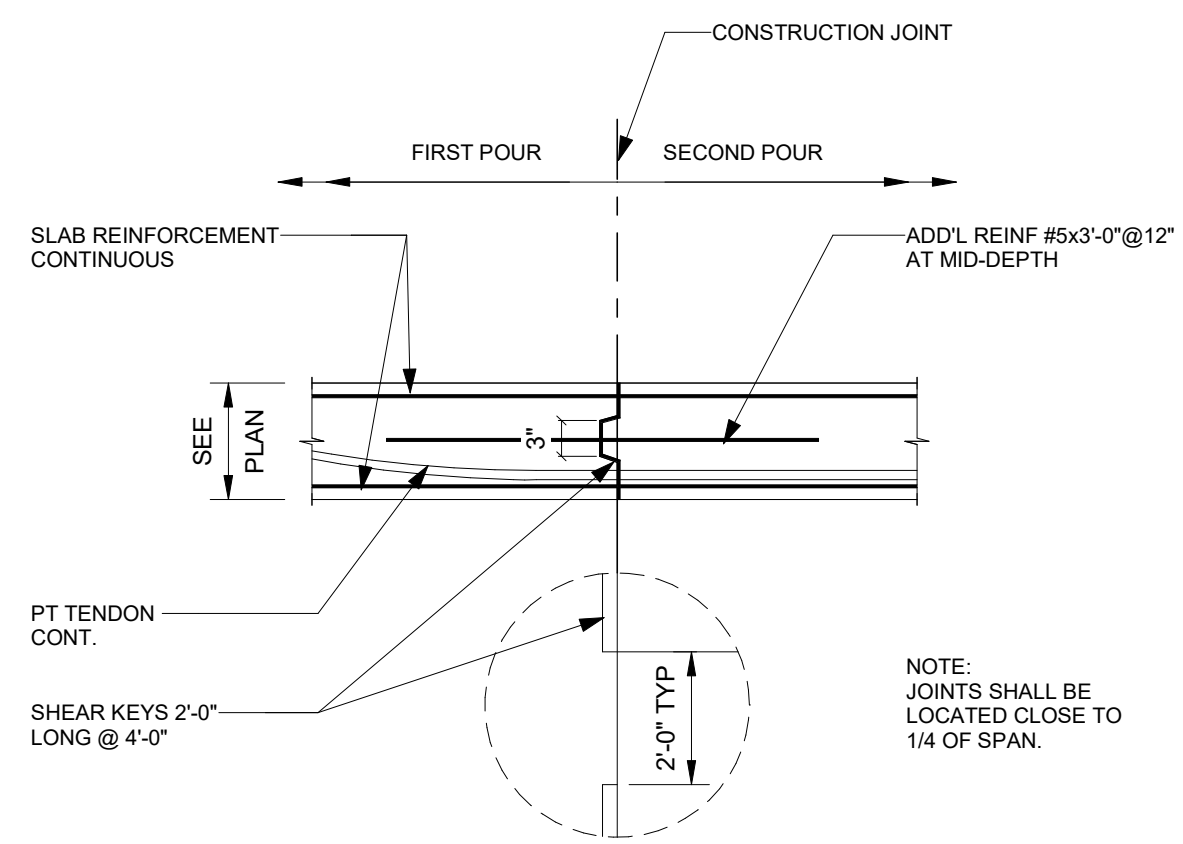
**DUAL HOTEL DURANGO**  
**- AC & HIS**

JOB NO: 18-007  
DATE: 2019.03.07  
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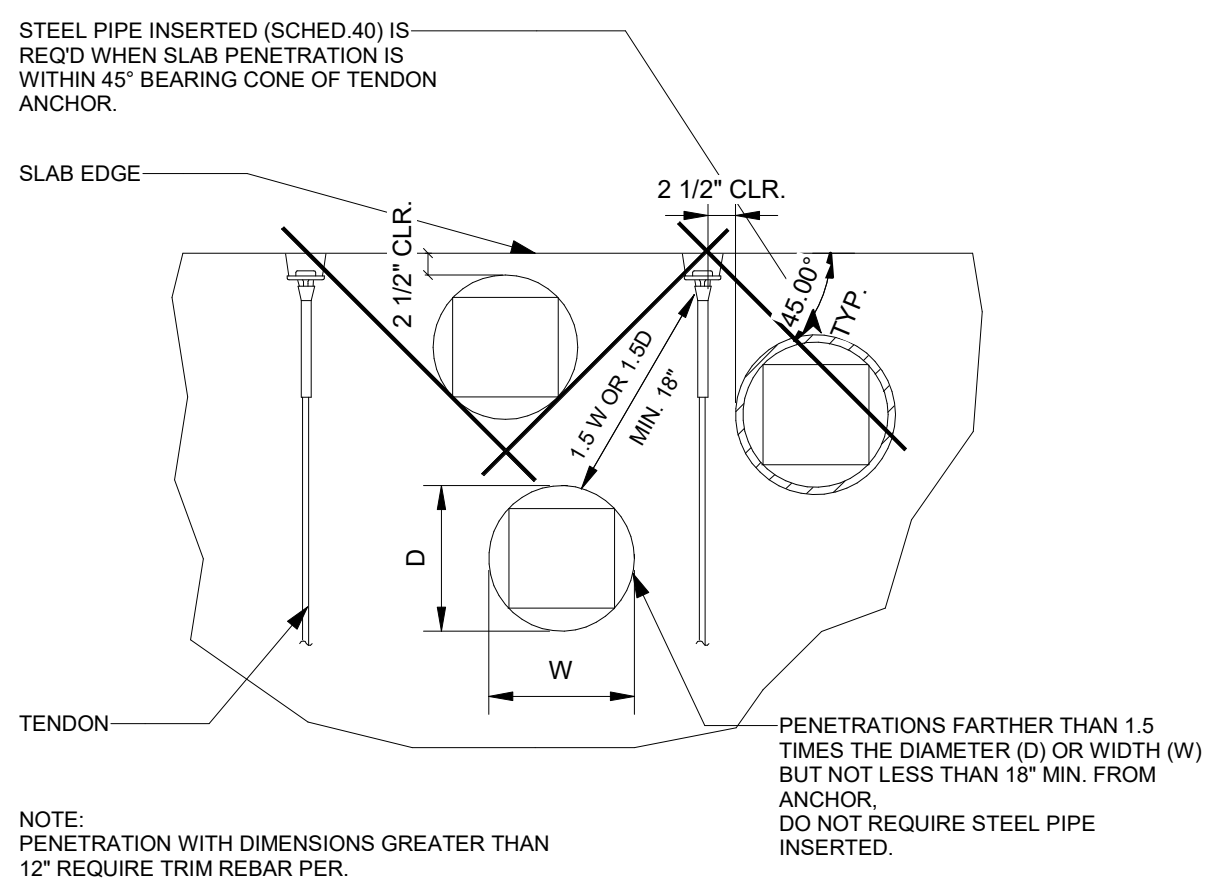
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TYPICAL DETAIL

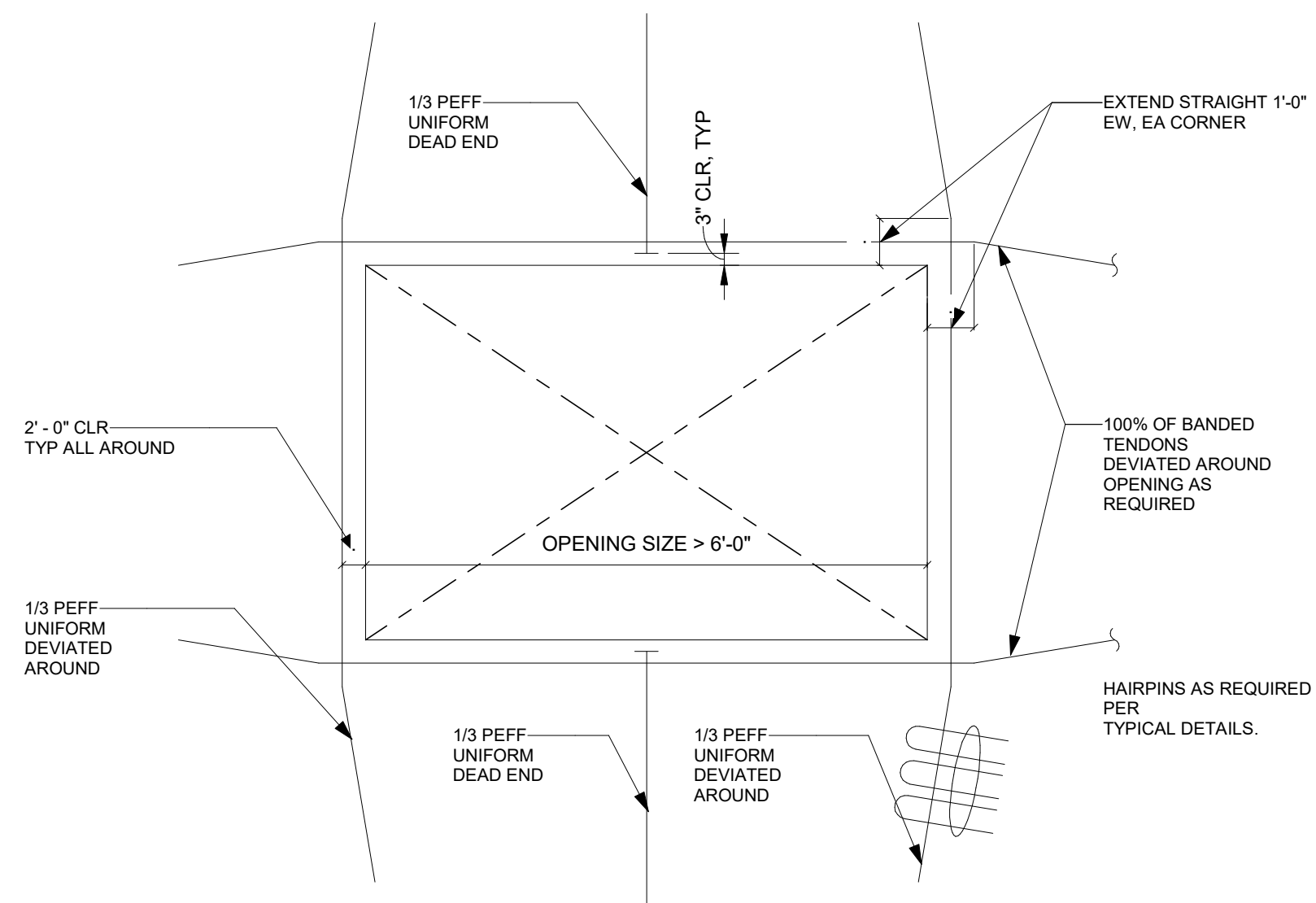
**S-602**



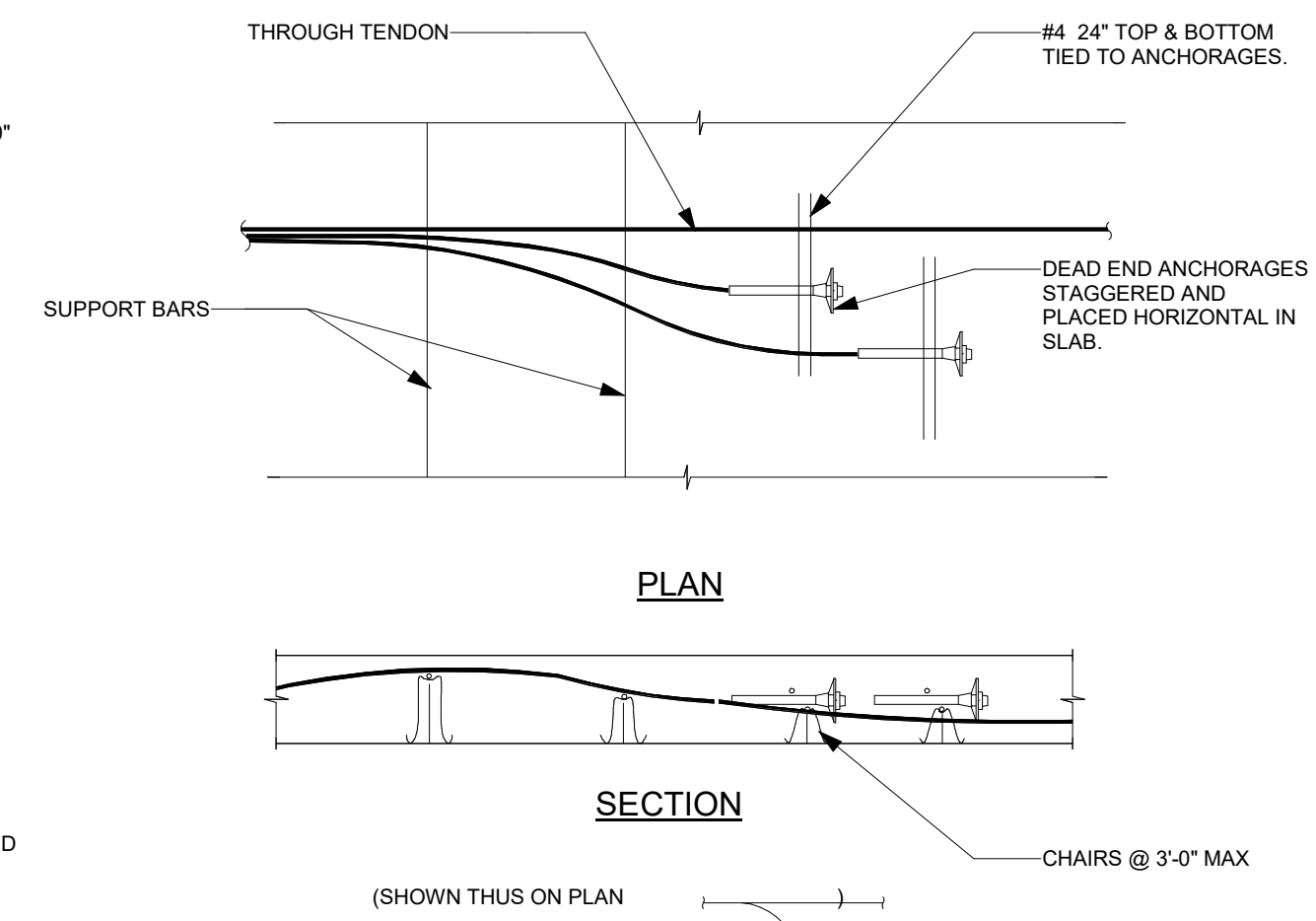
TYPICAL CONSTRUCTION JOINT WITH NO INTERMEDIATE STRESSING 29



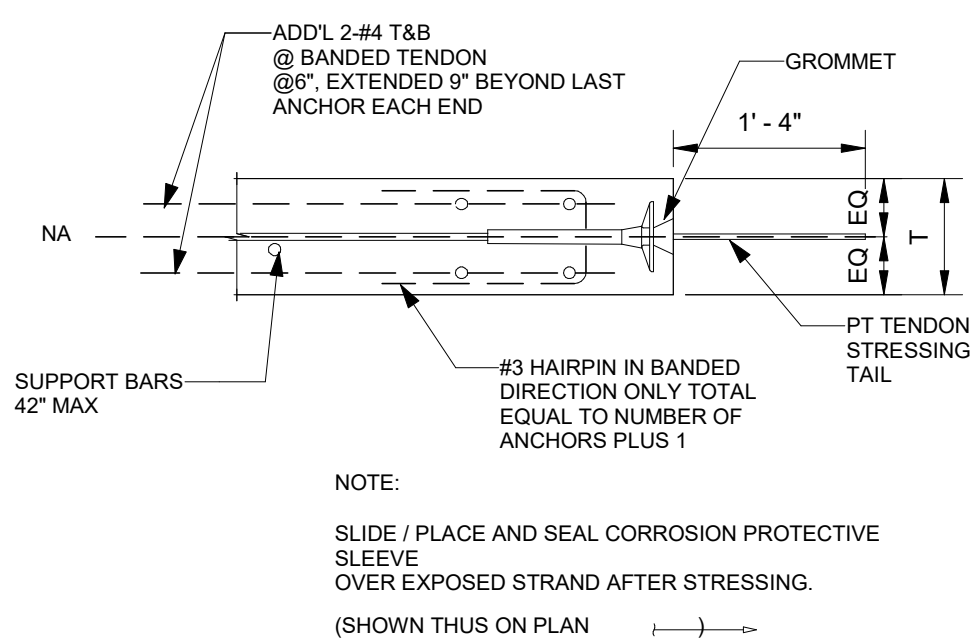
TYPICAL OPENINGS AT PT ANCHORAGE 30  
SCALE: NOT TO SCALE



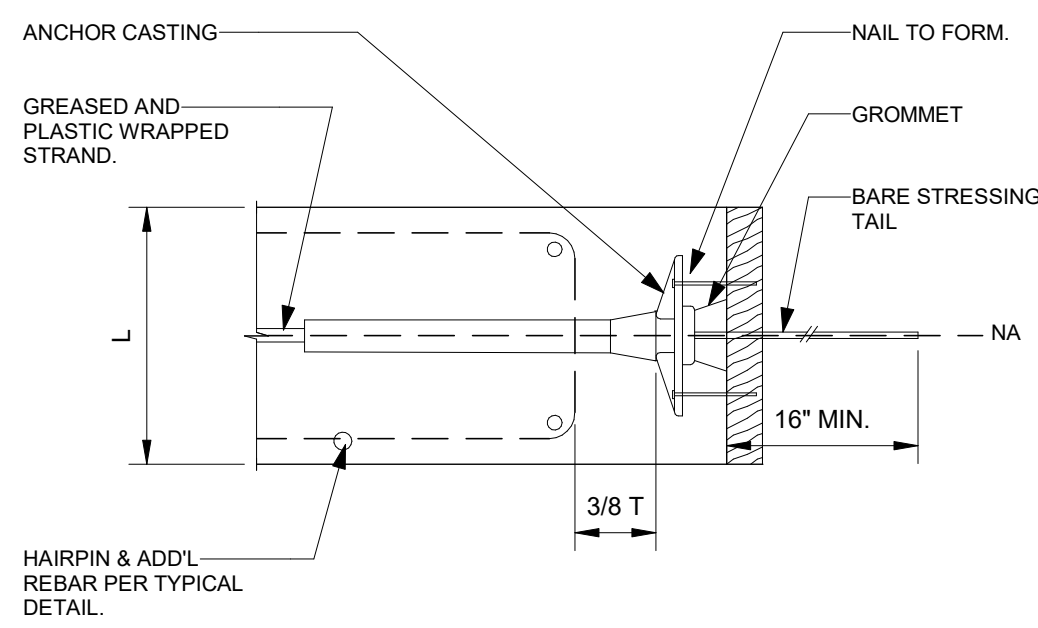
TYPICAL PT TENDON LAYOUT AT LARGE OPENINGS 31



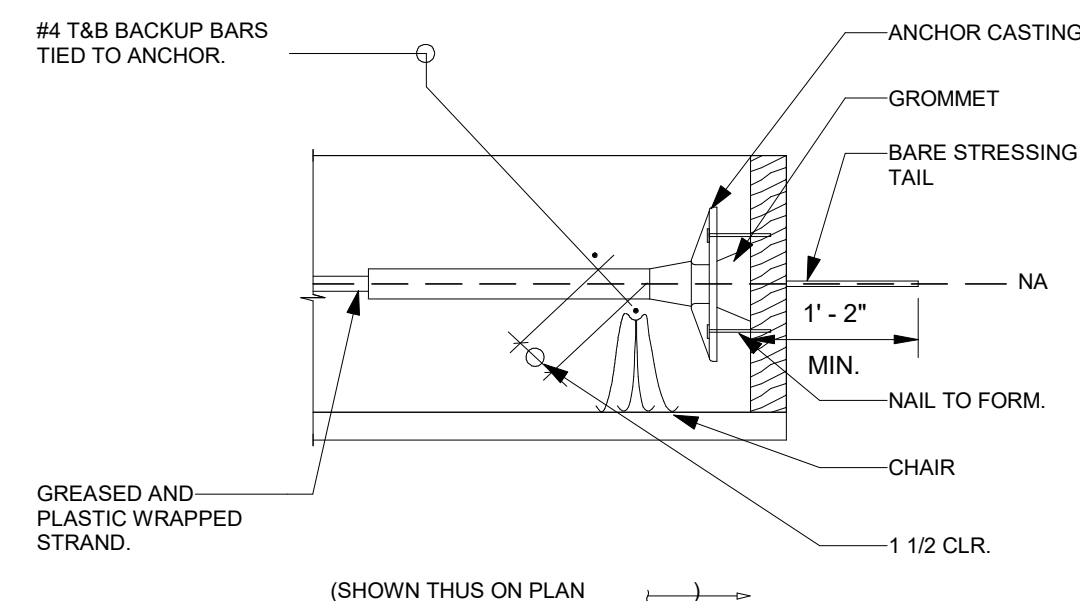
TYPICAL PLACEMENT OF ADDED TENDON 32  
SCALE: NOT TO SCALE



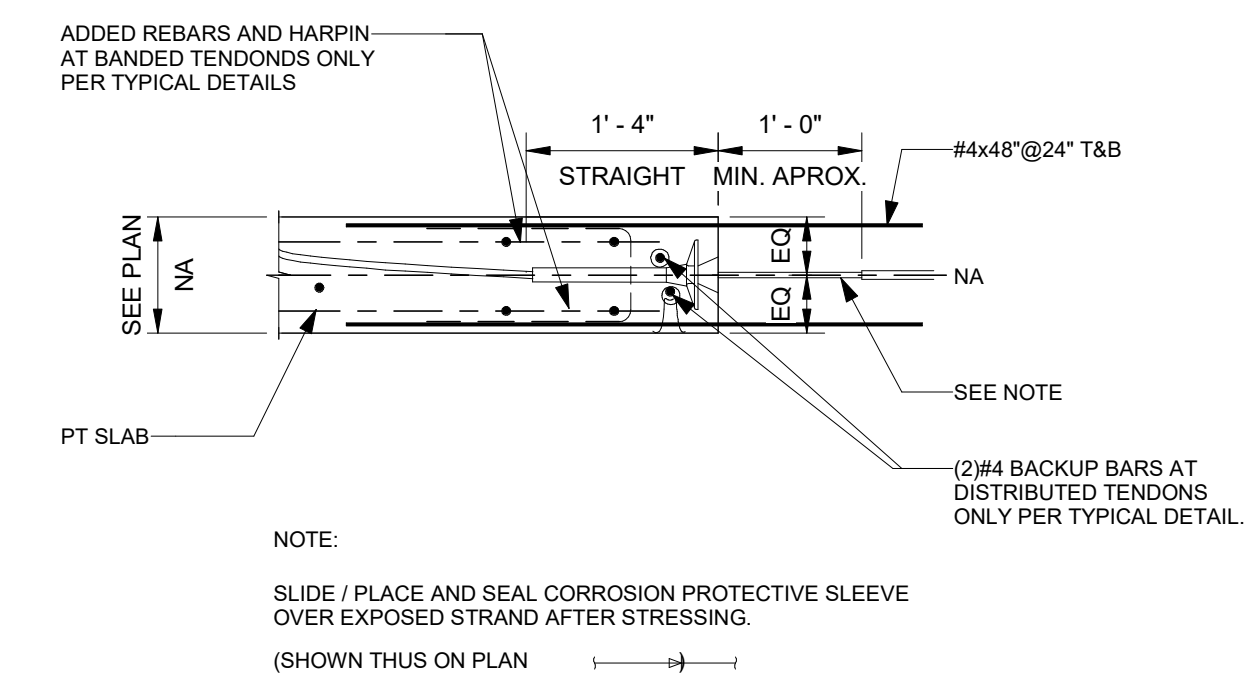
TYPICAL STRESSING END ANCHOR SHOWING HAIRPIN AND ADDED #4 BARS 33



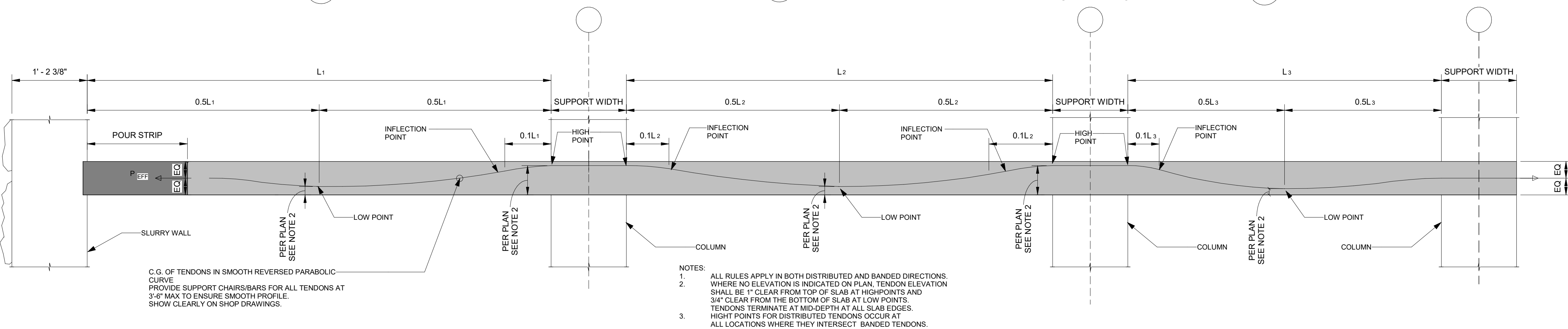
TYPICAL STRESSING END ANCHOR FOR BANDED TENDON 34



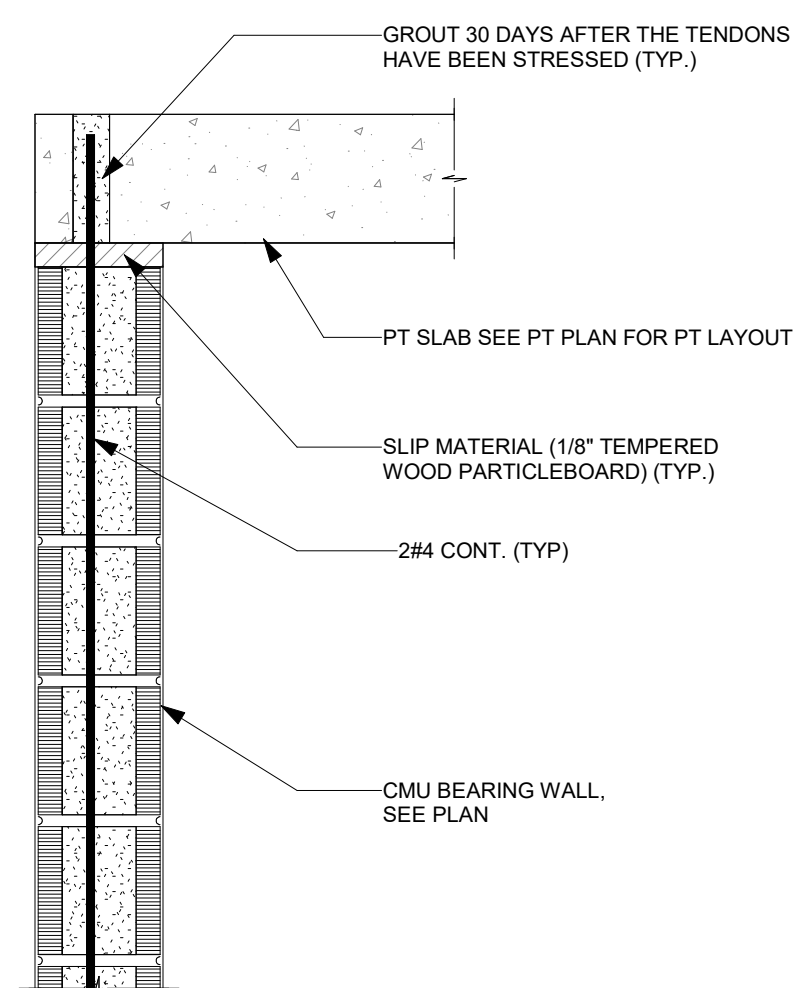
TYPICAL STRESSING END ANCHOR FOR DISTRIBUTED TENDONS 35



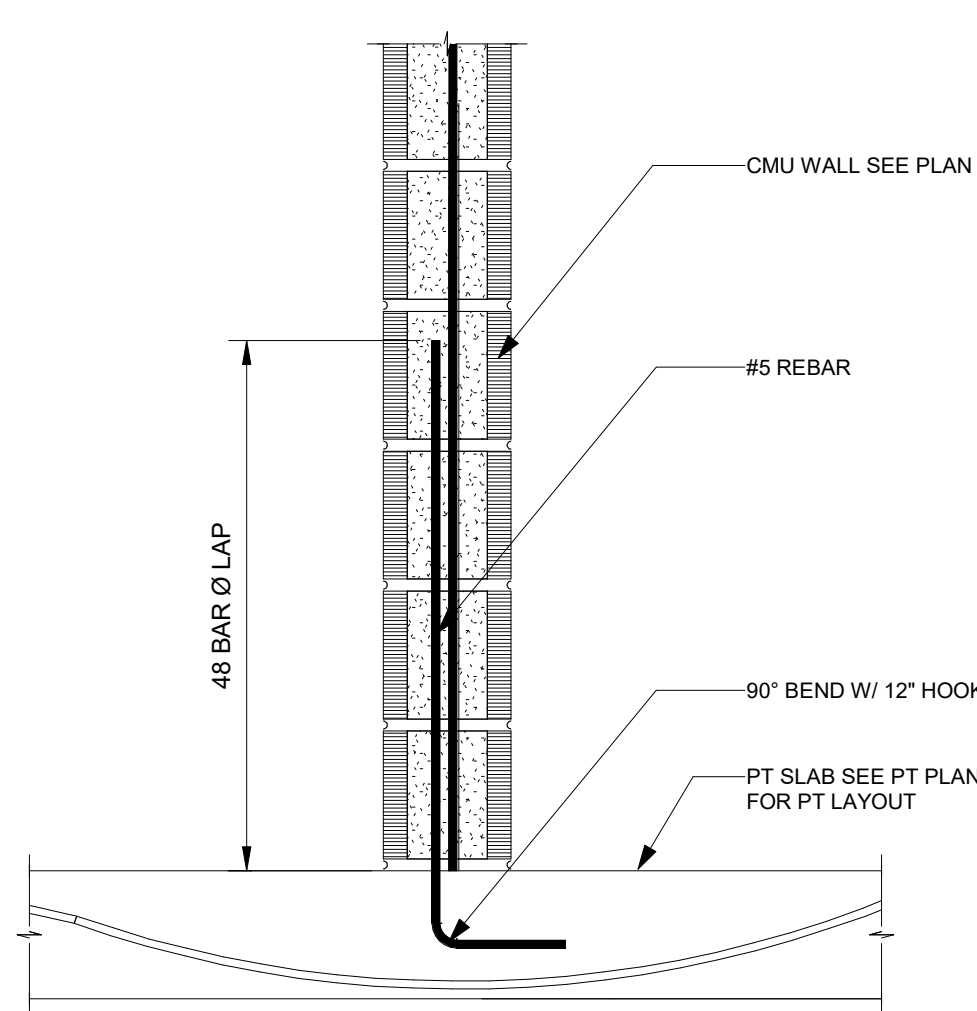
TYPICAL CONSTRUCTION JOINT WITH INTERMEDIATE STRESSING 36



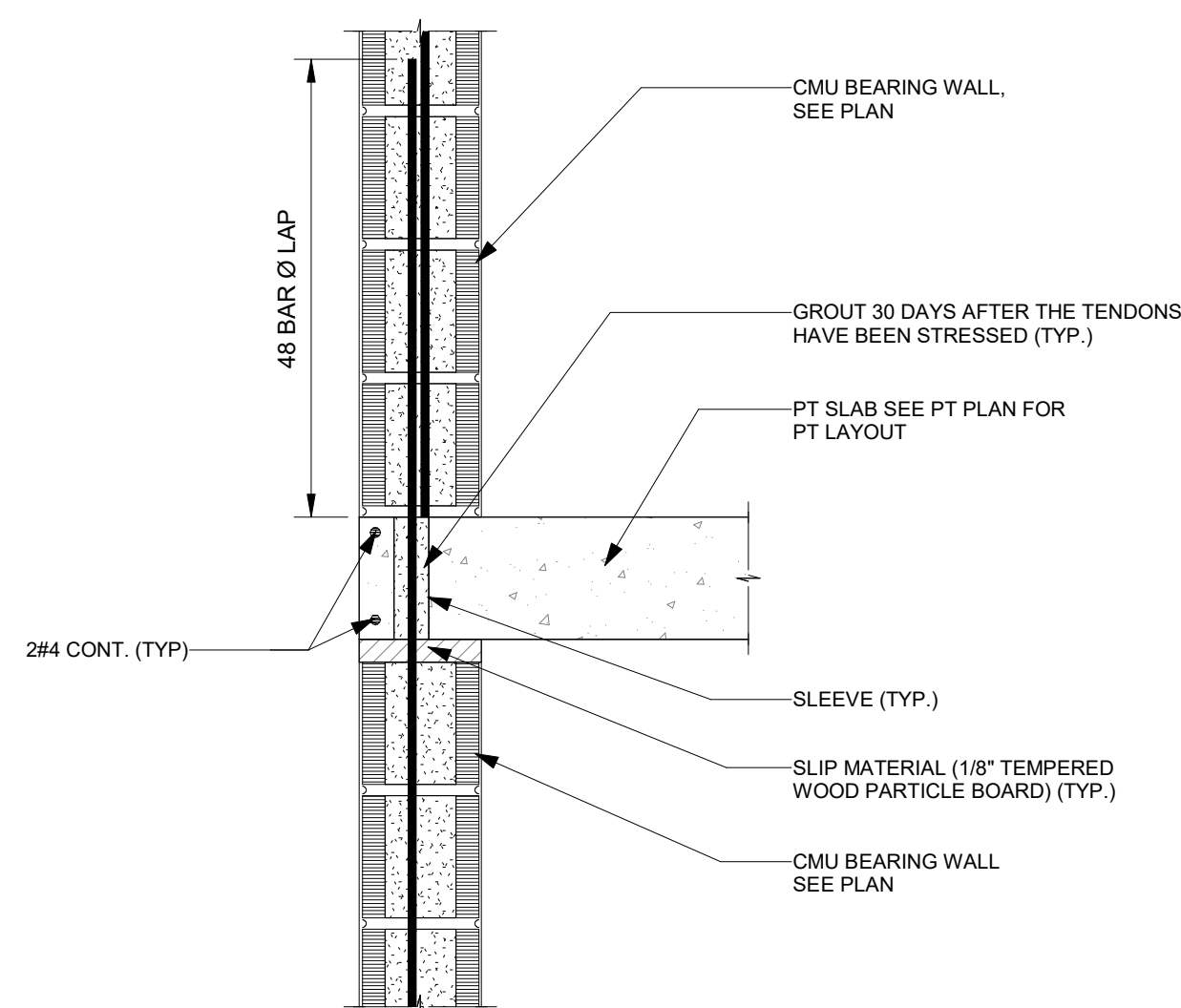
TYPICAL PT TENDON DRAPING DIAGRAM 37



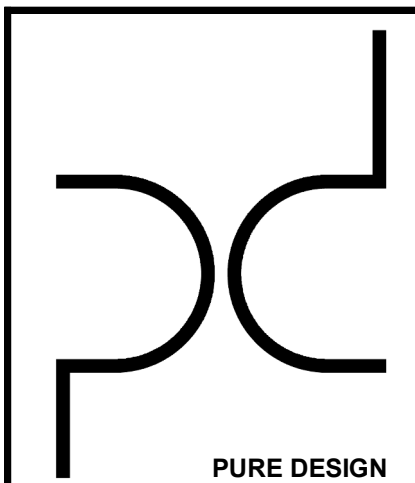
TYP. CONNECTION DETAIL EXTERIOR CMU BEARING WALL & PT SLAB 38



TYP. CONNECTIONS BETWEEN PT SLABS AND CMU WALL 39



TYP. CONNECTION DETAIL EXTERIOR CMU WALL & PT SLAB 40



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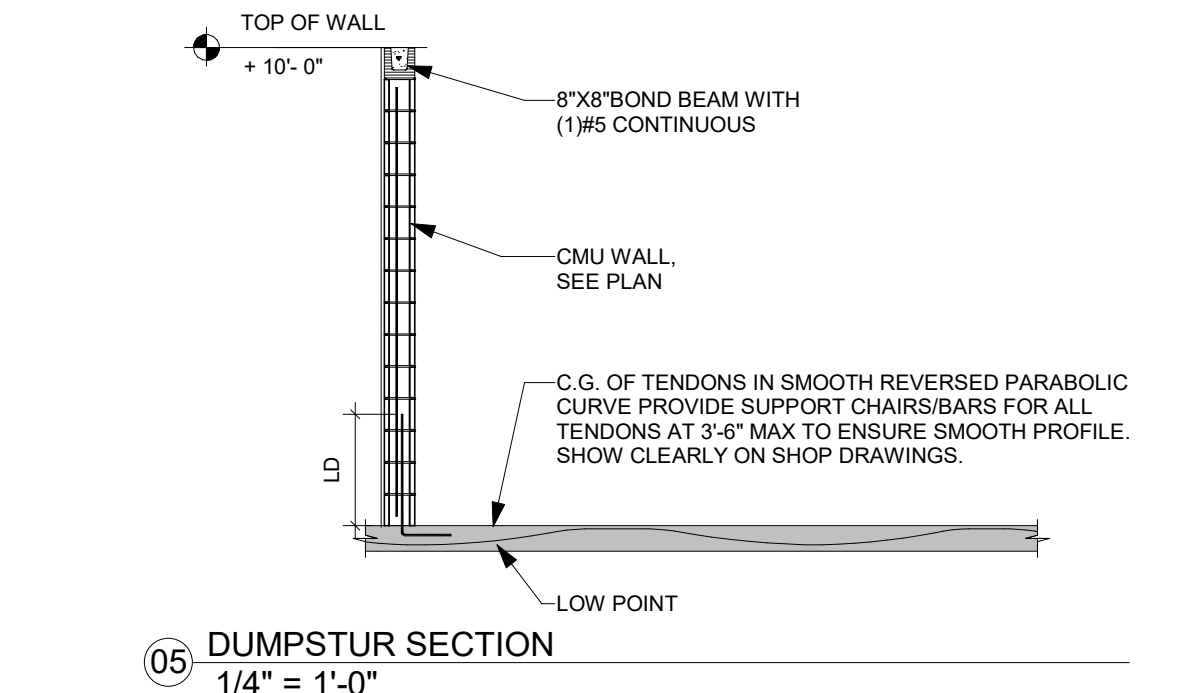
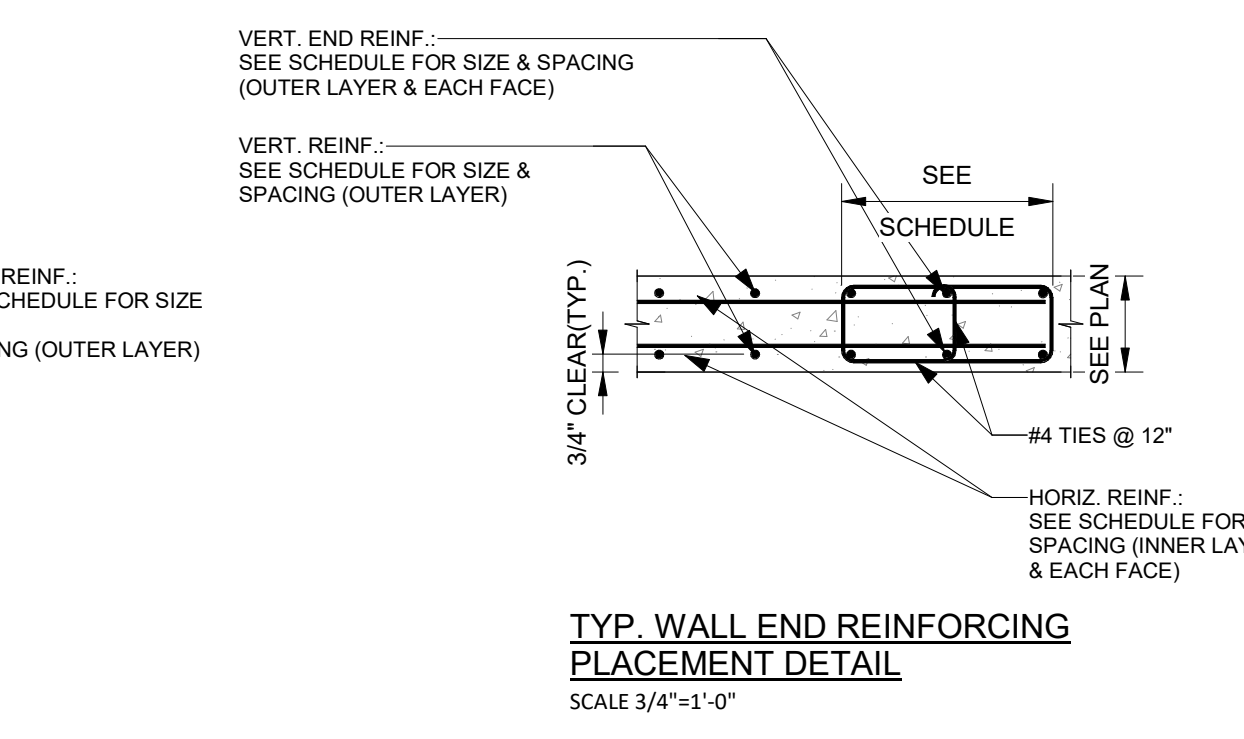
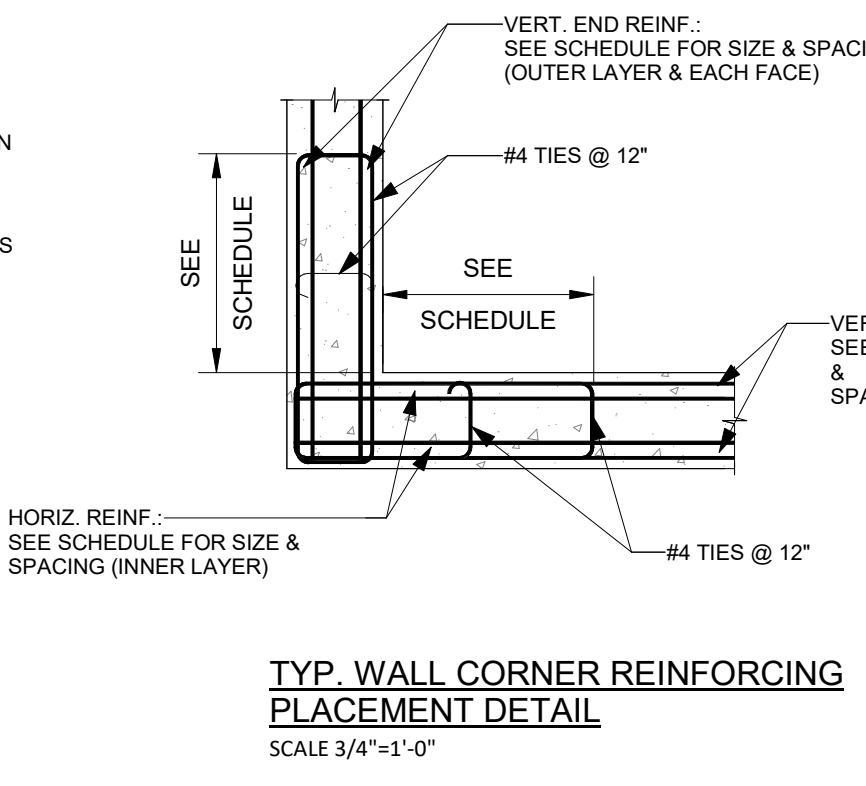
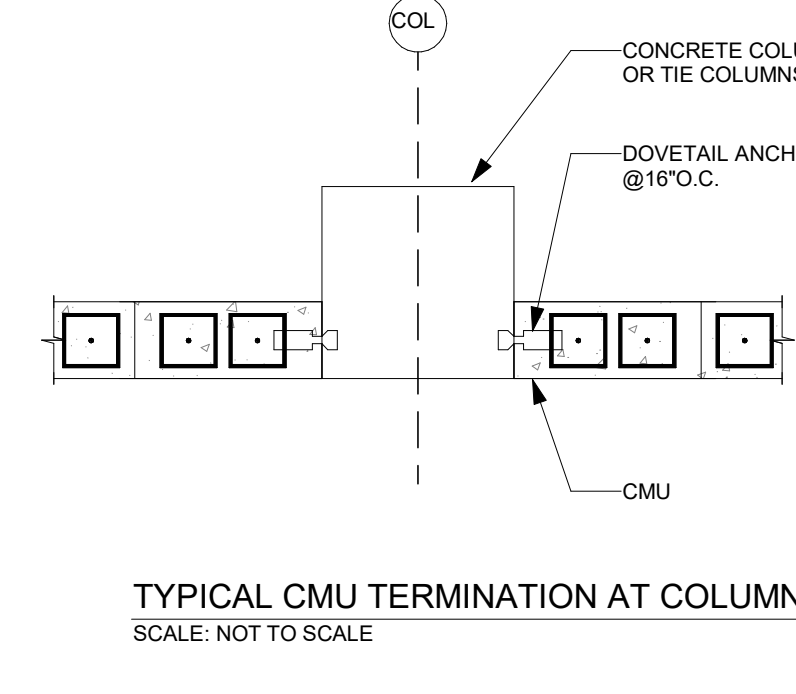
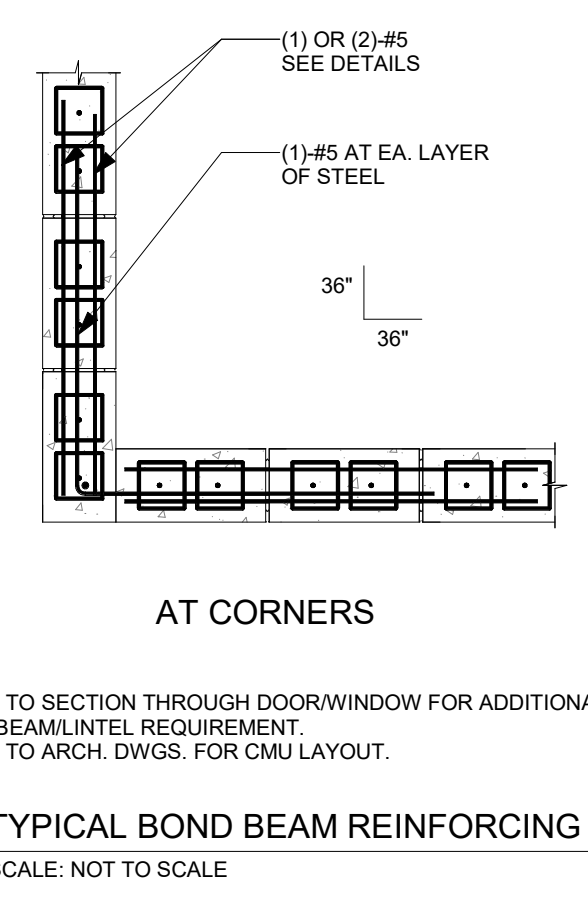
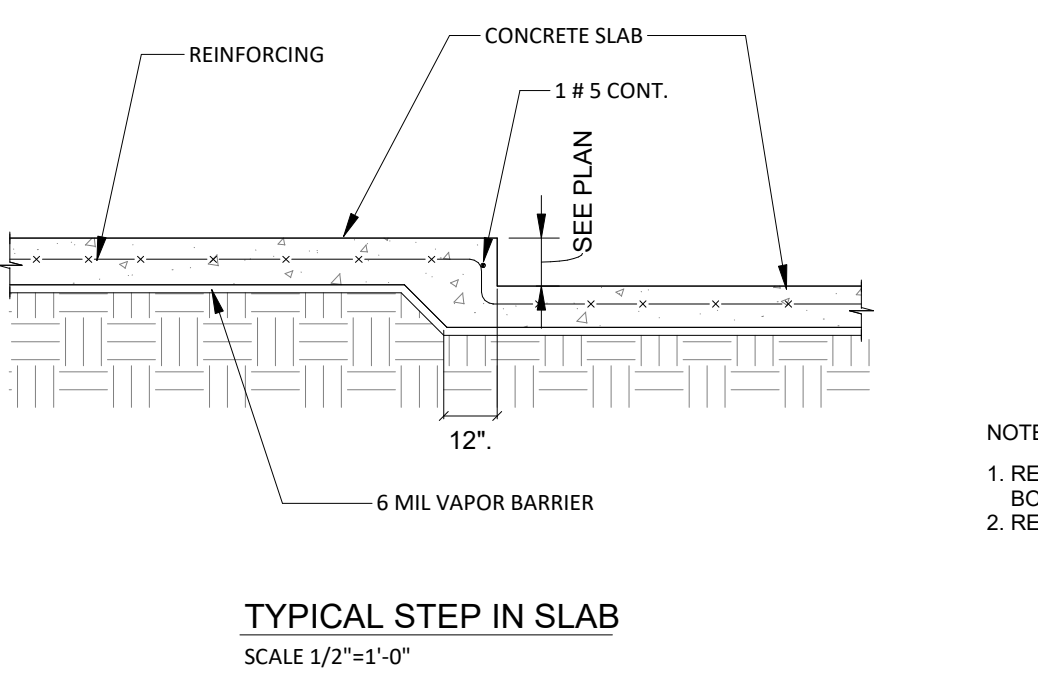
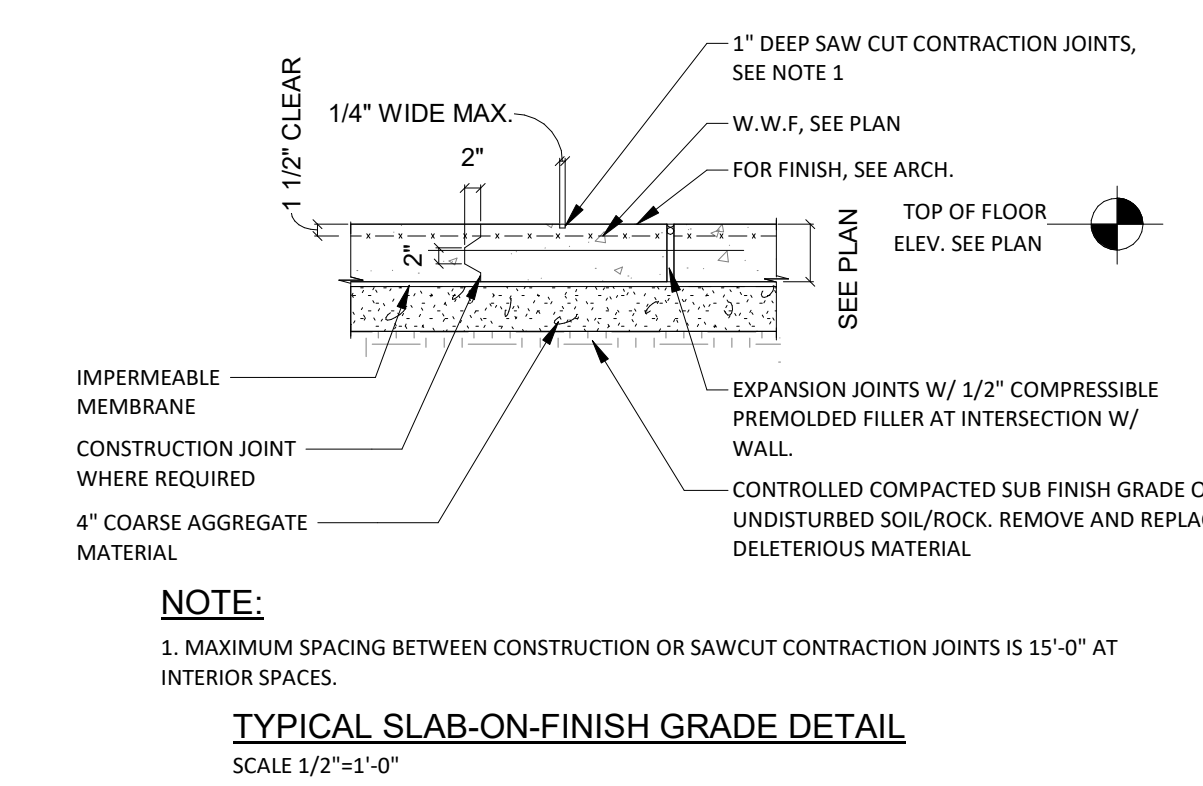
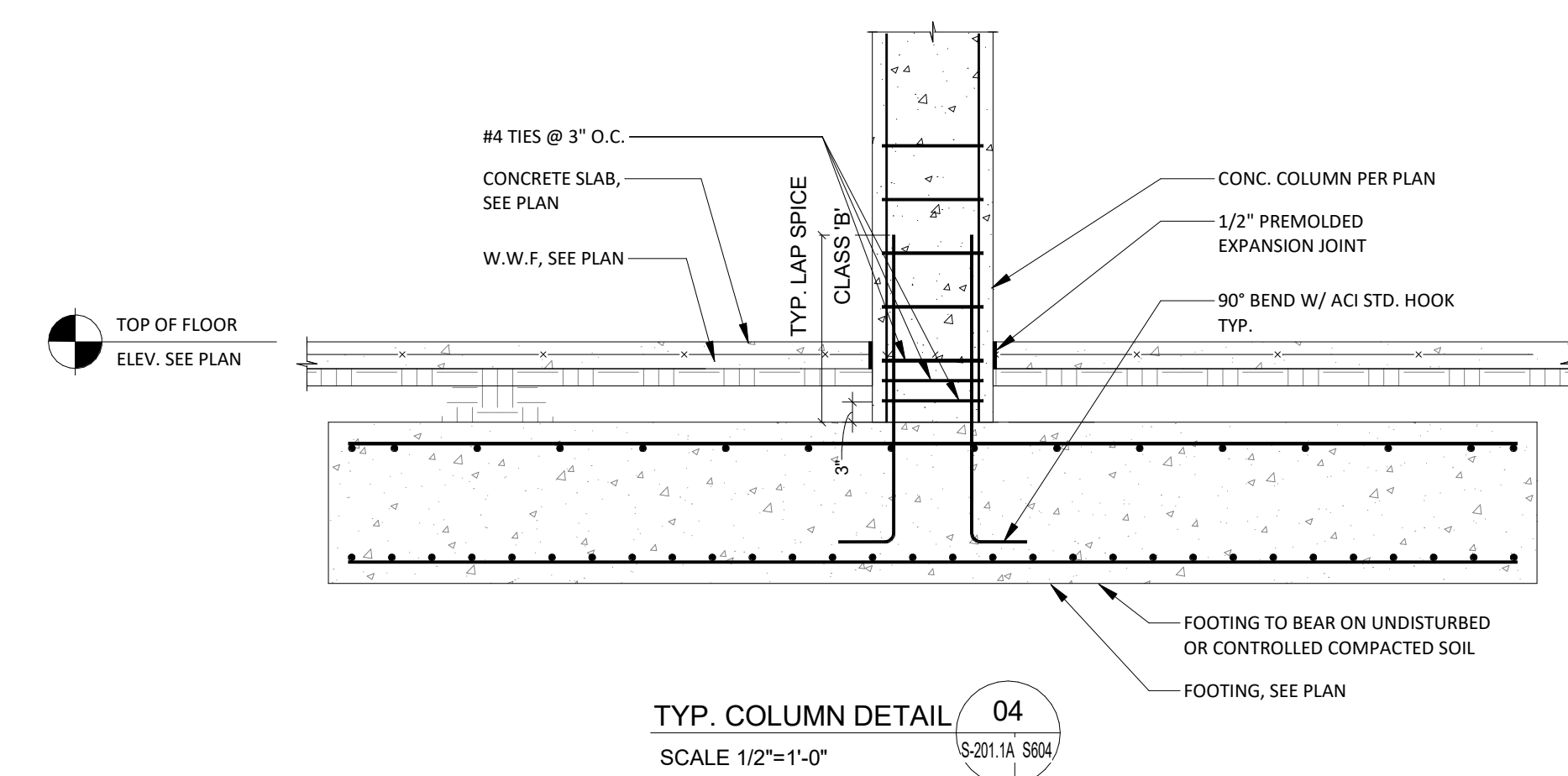
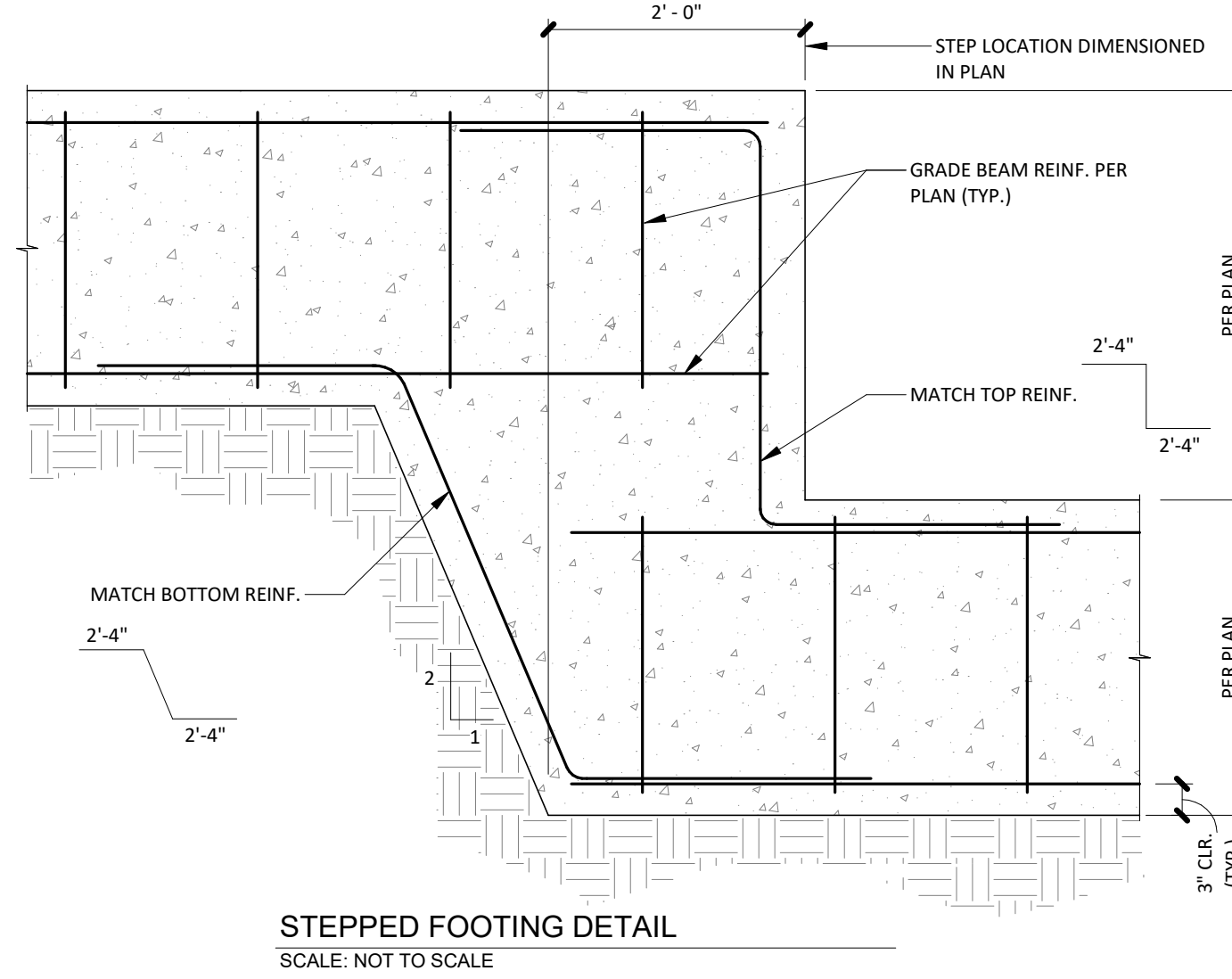
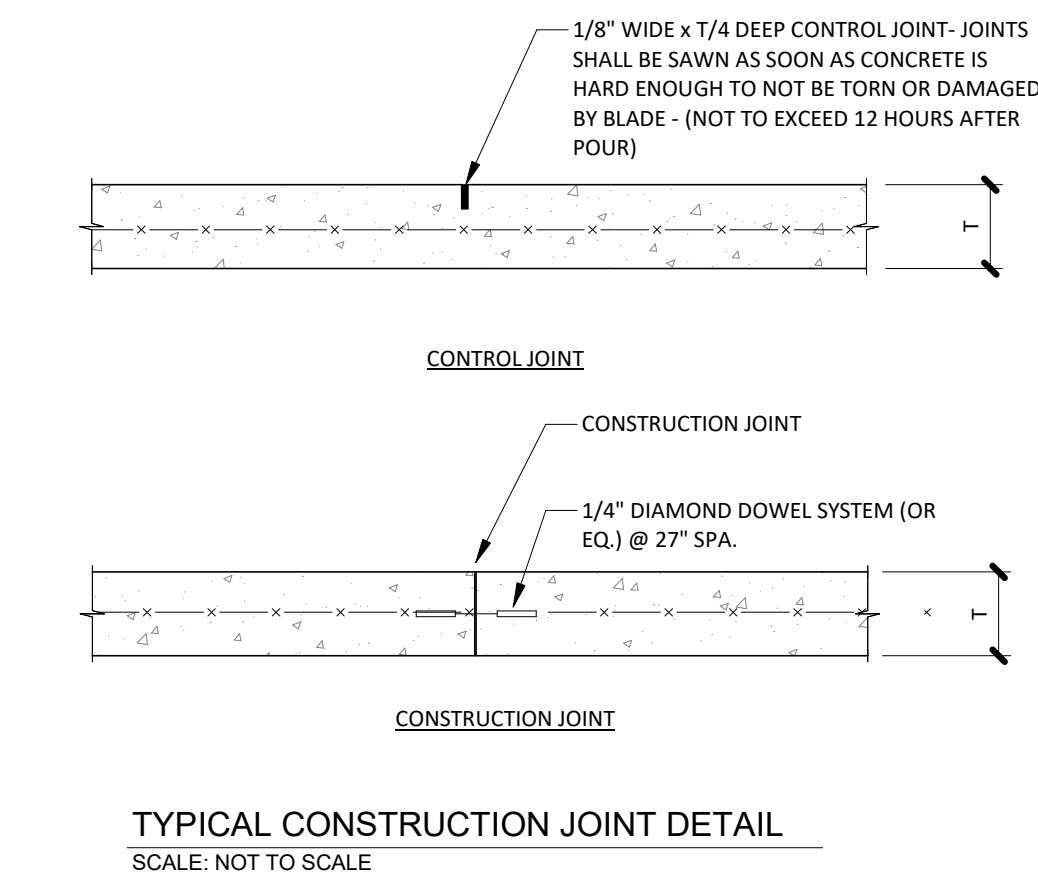
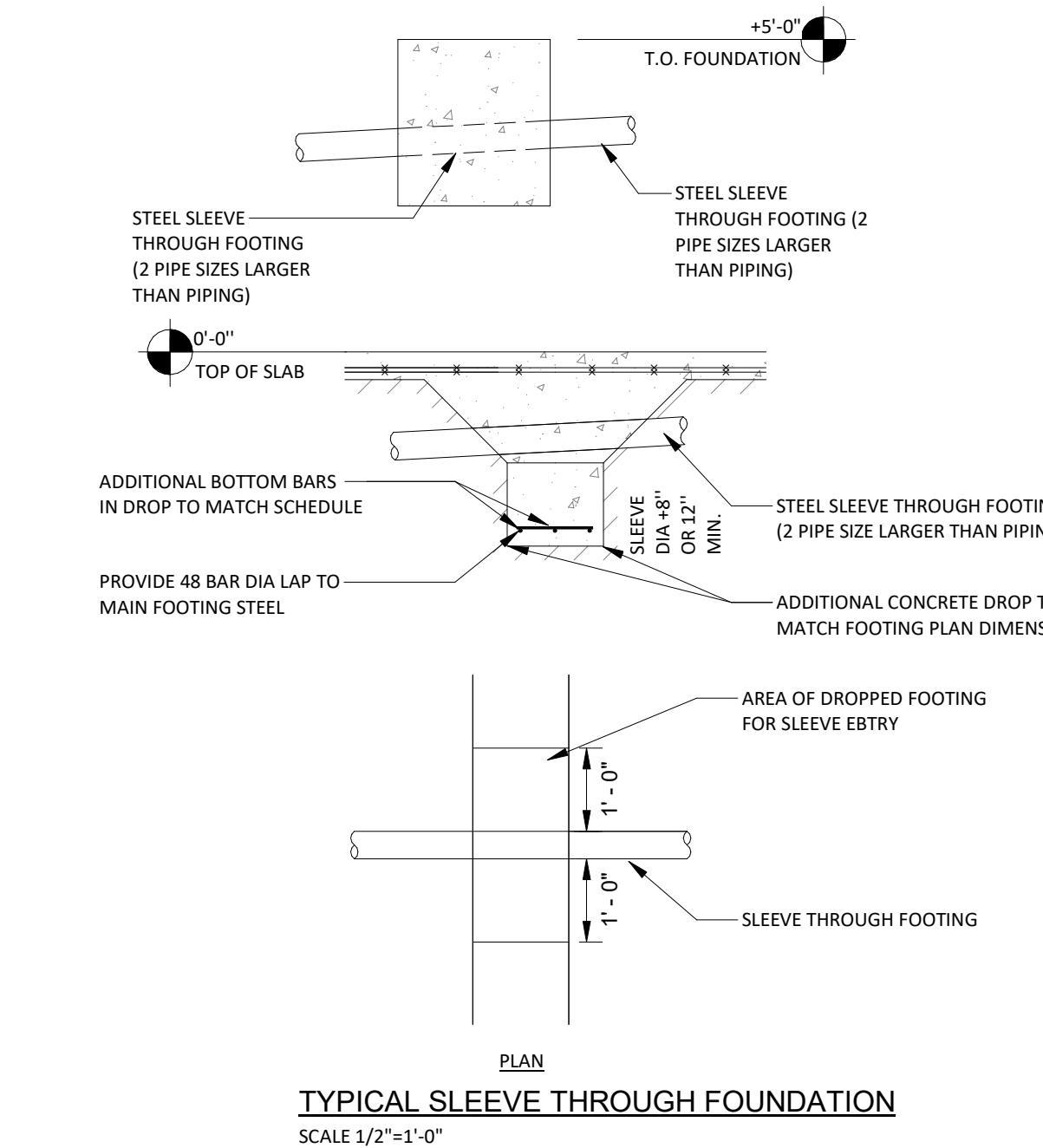
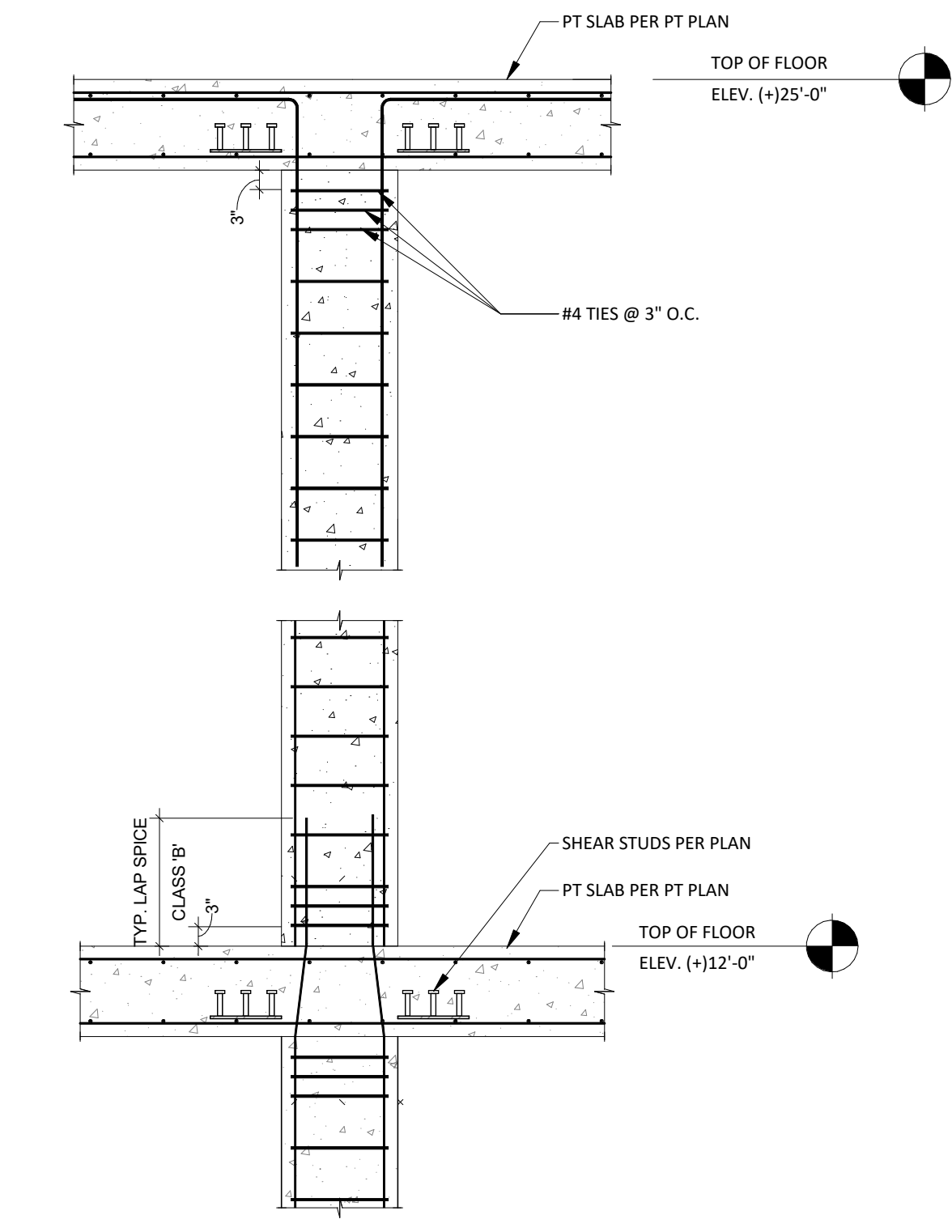
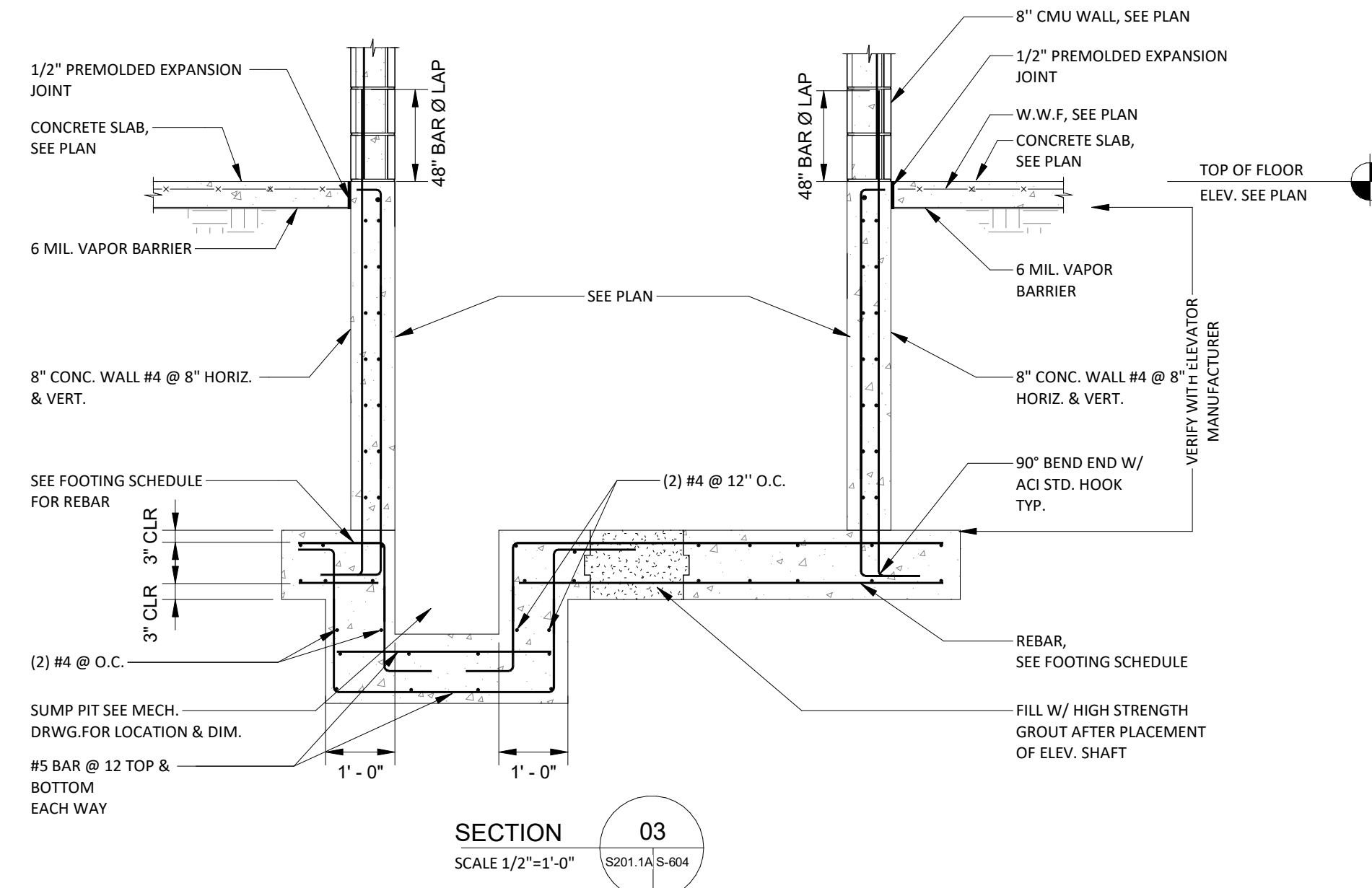
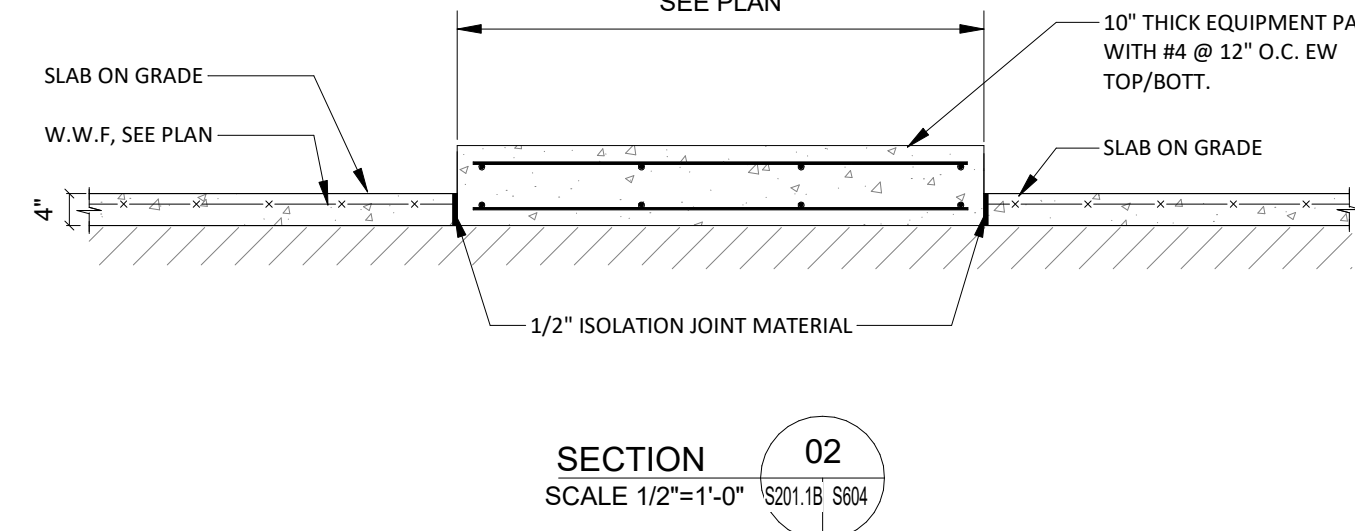
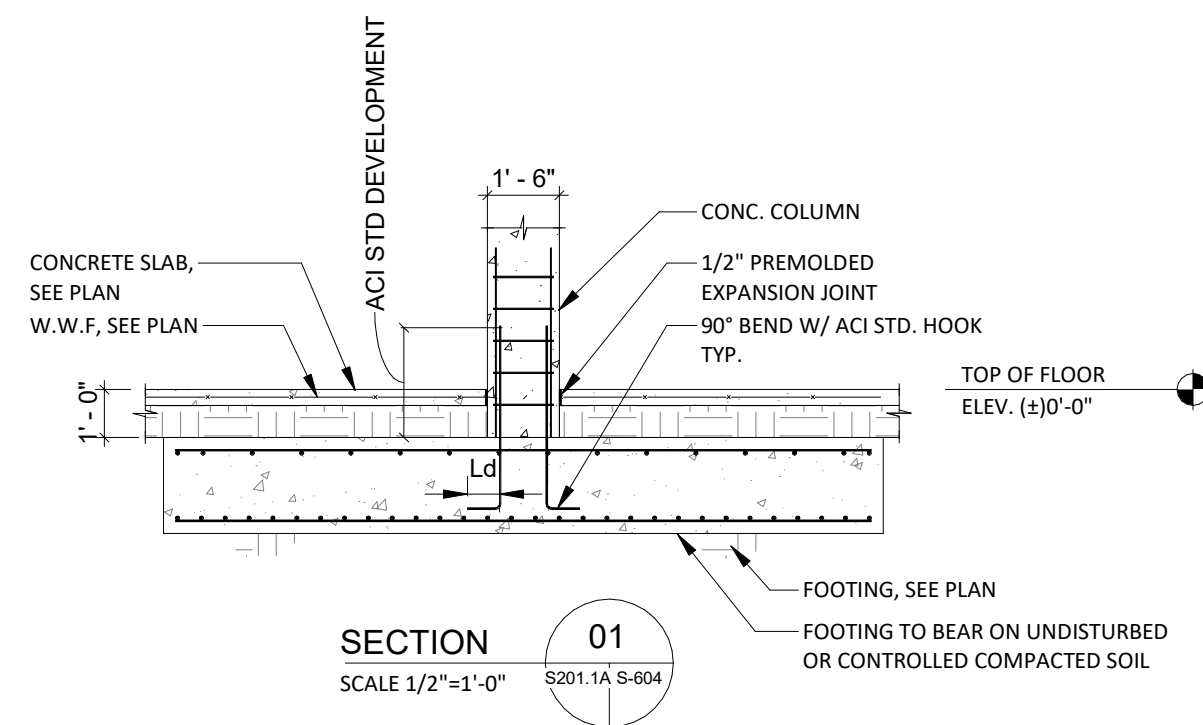
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JOB NO:	18-007
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TYPICAL DETAIL  
S-603

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05 DUMPSTUR SECTION  
1/4" = 1'-0"

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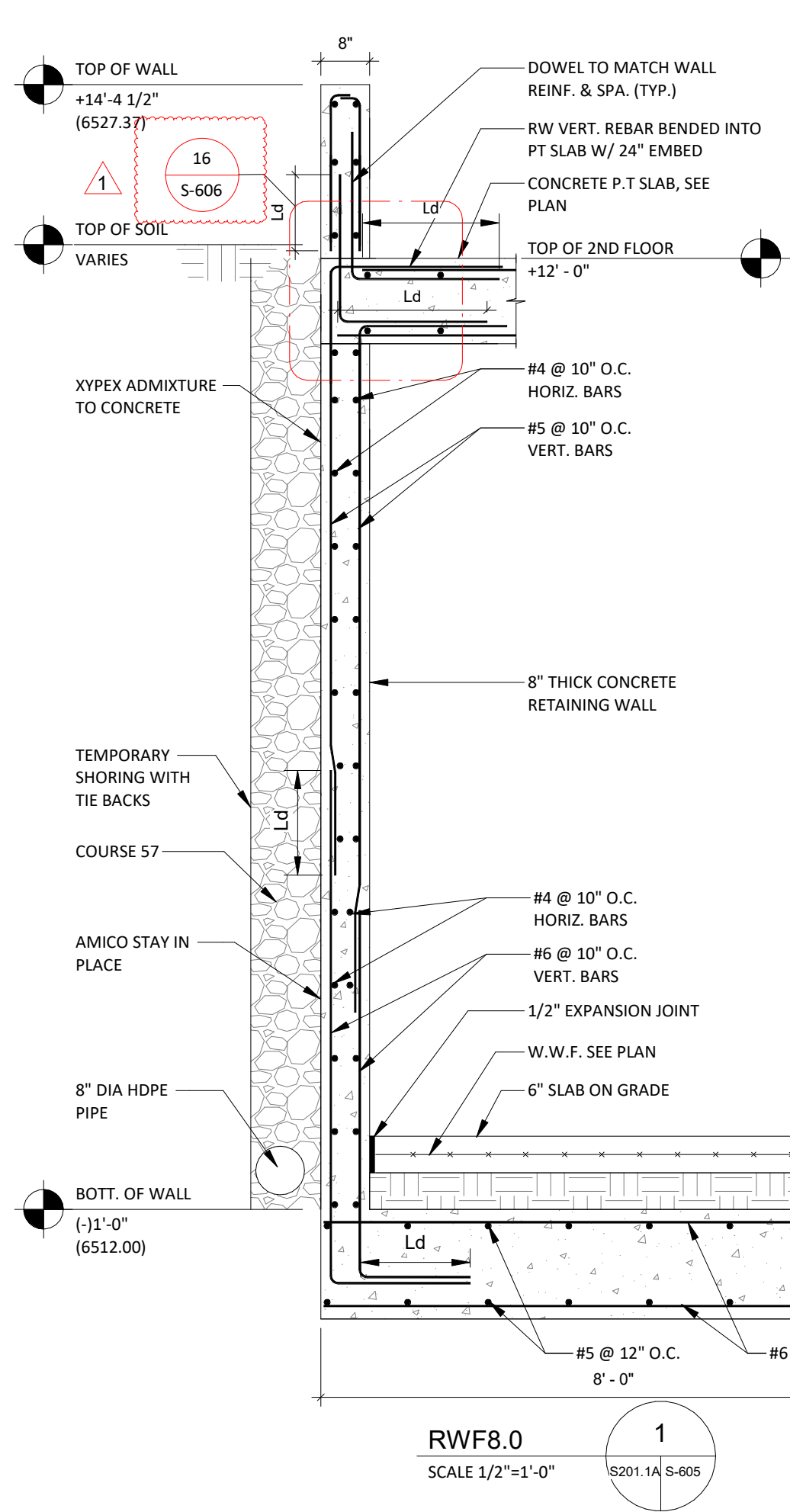
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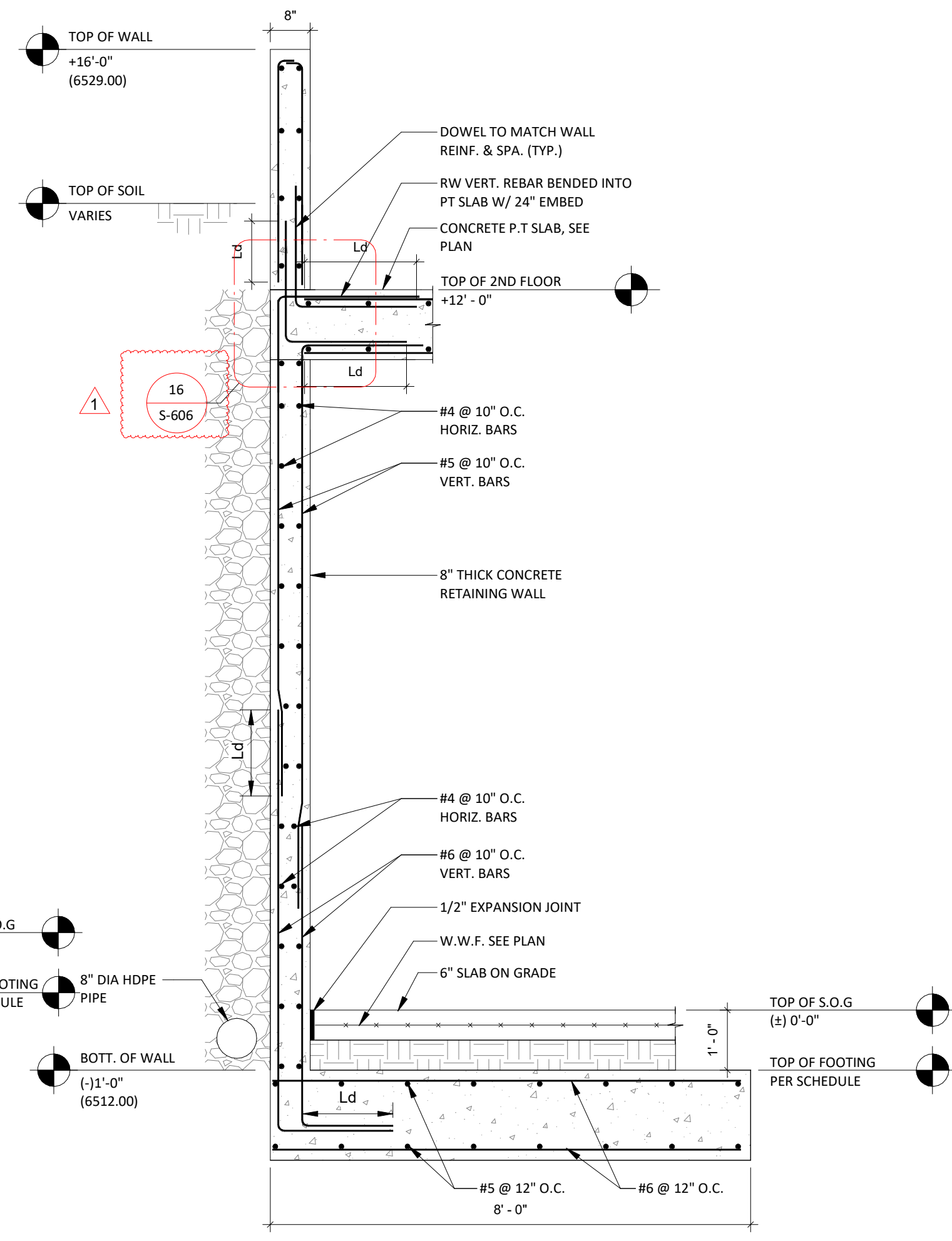
**TYPICAL DETAIL**

**S-604**

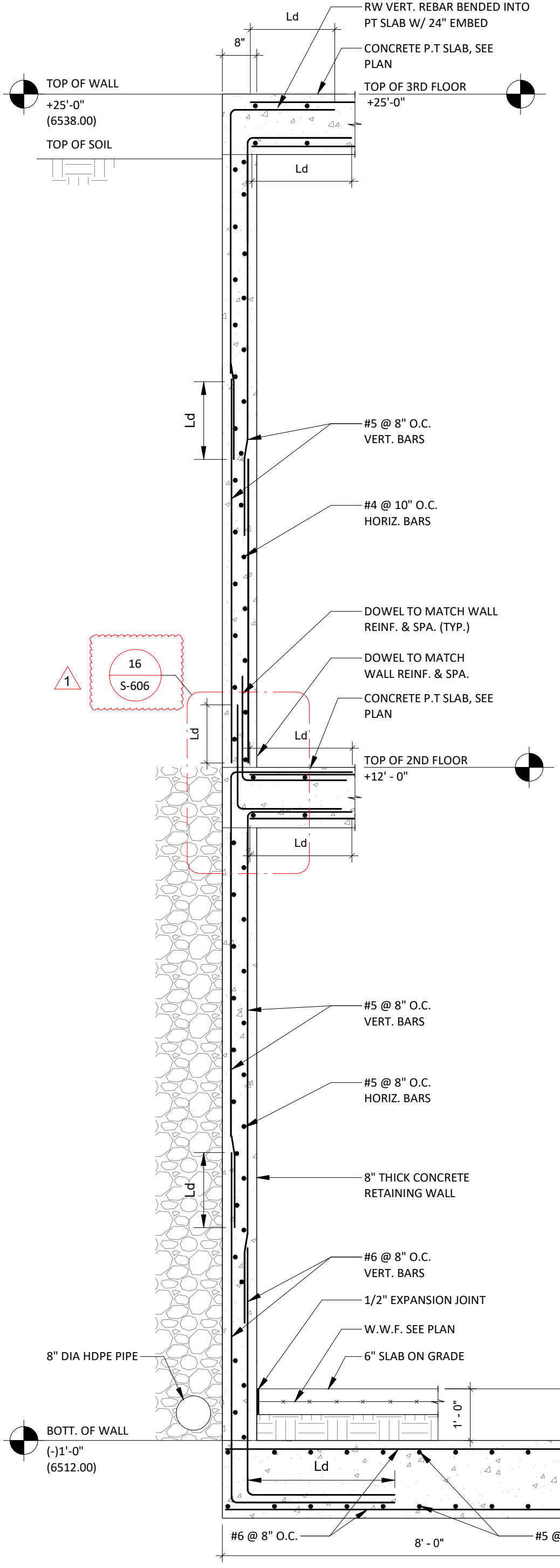
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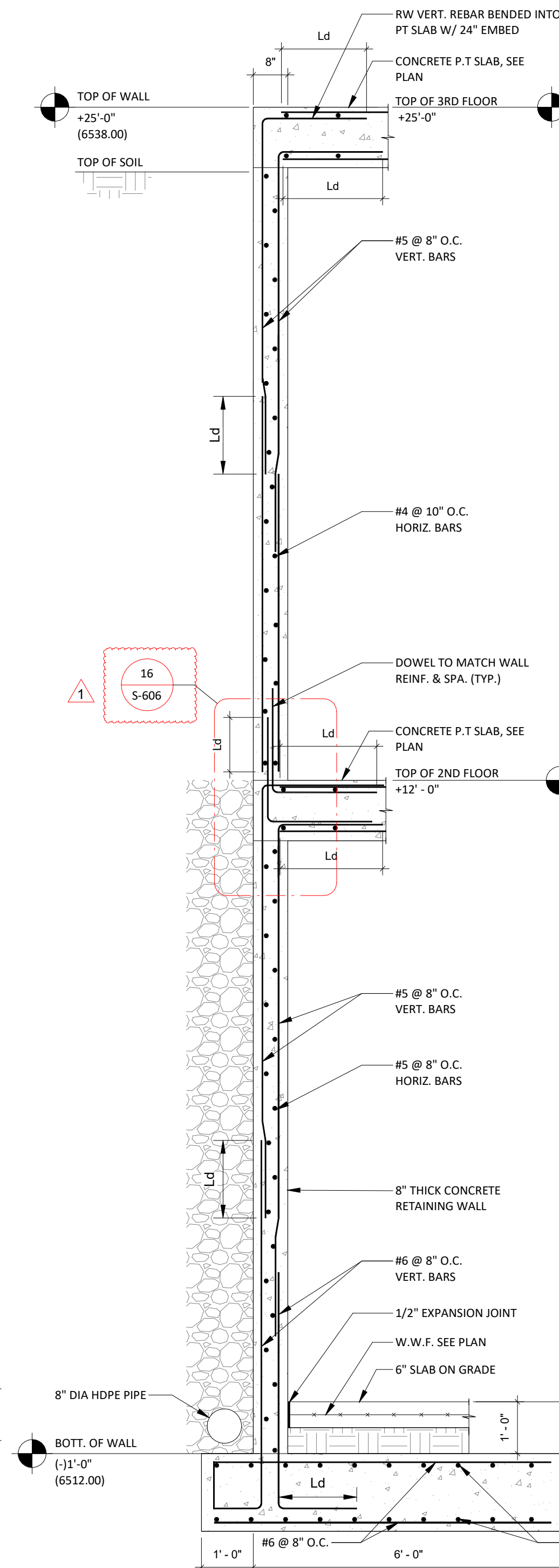
RWF8.0  
SCALE 1/2"=1'-0"  
S201.1A S-605



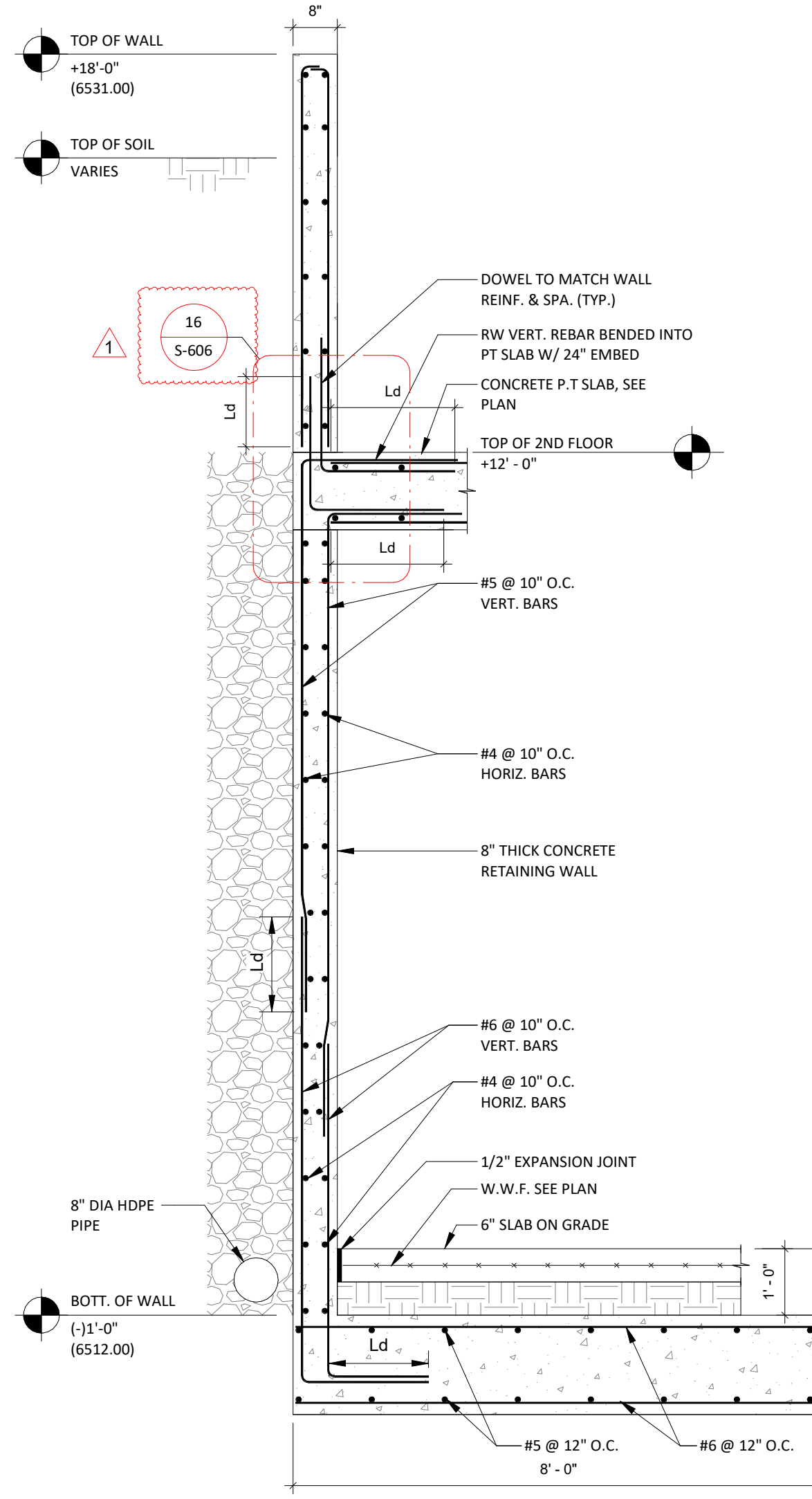
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S201.1A S-605



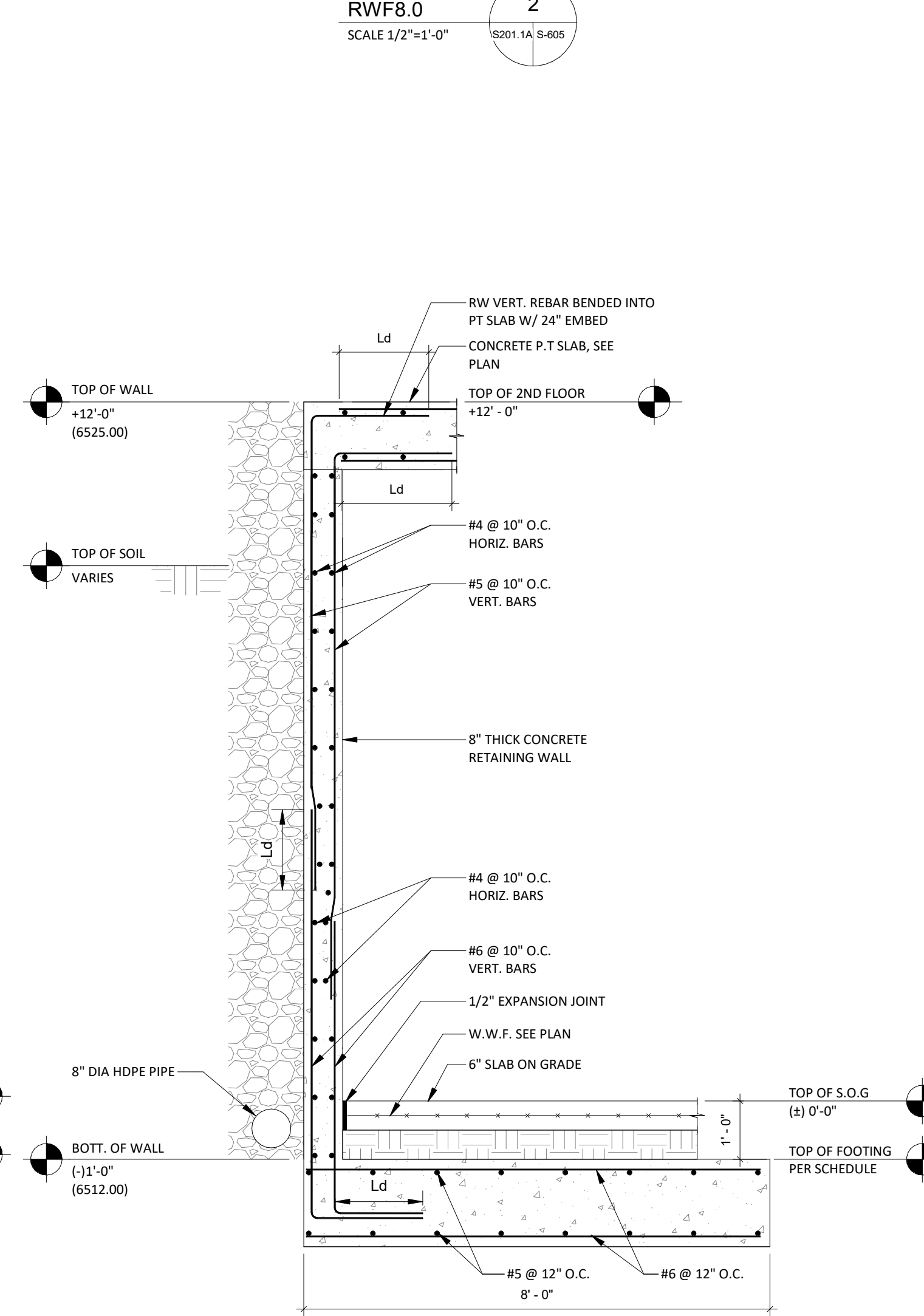
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S201.1B S-605



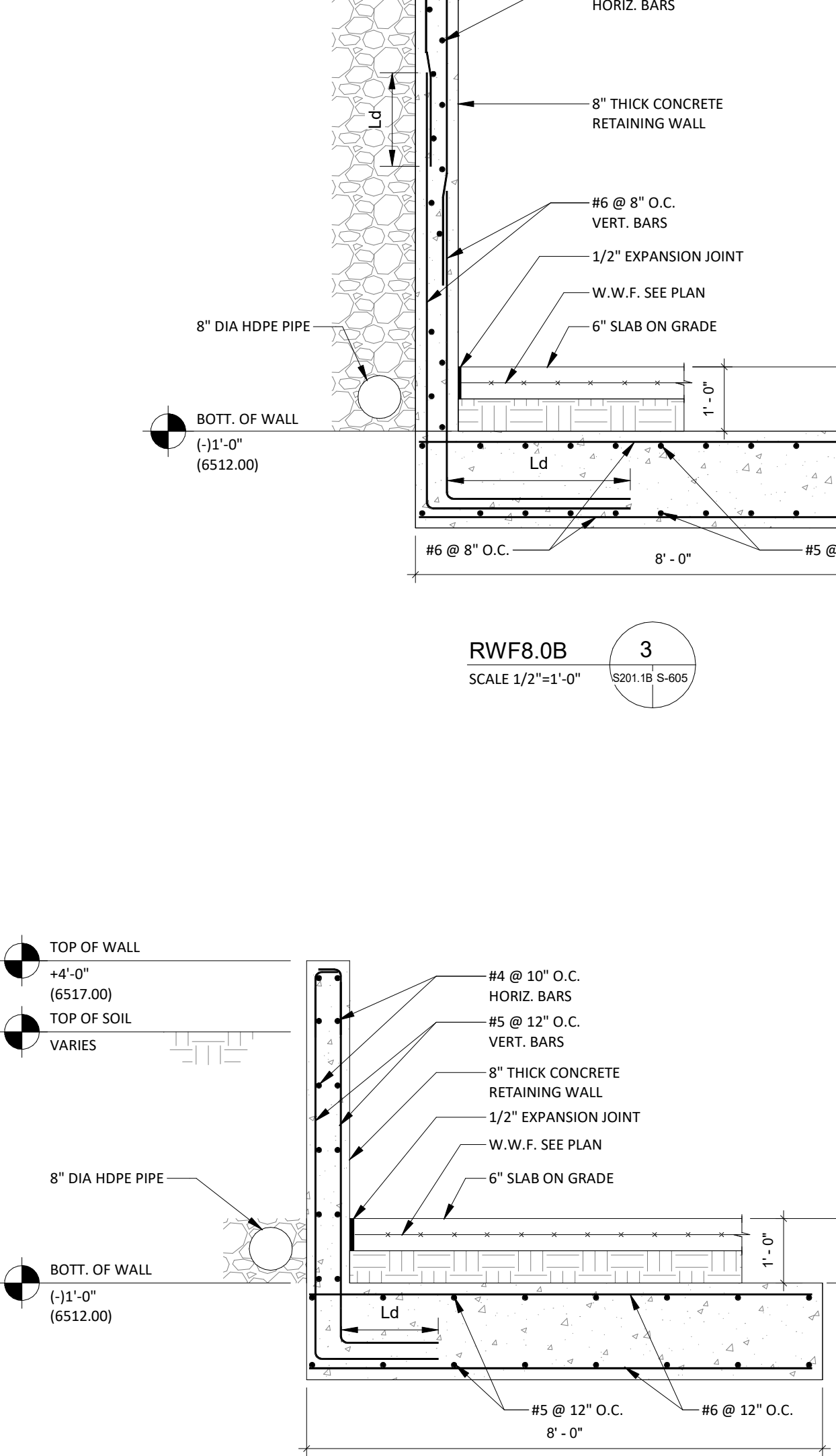
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S201.16 S-605



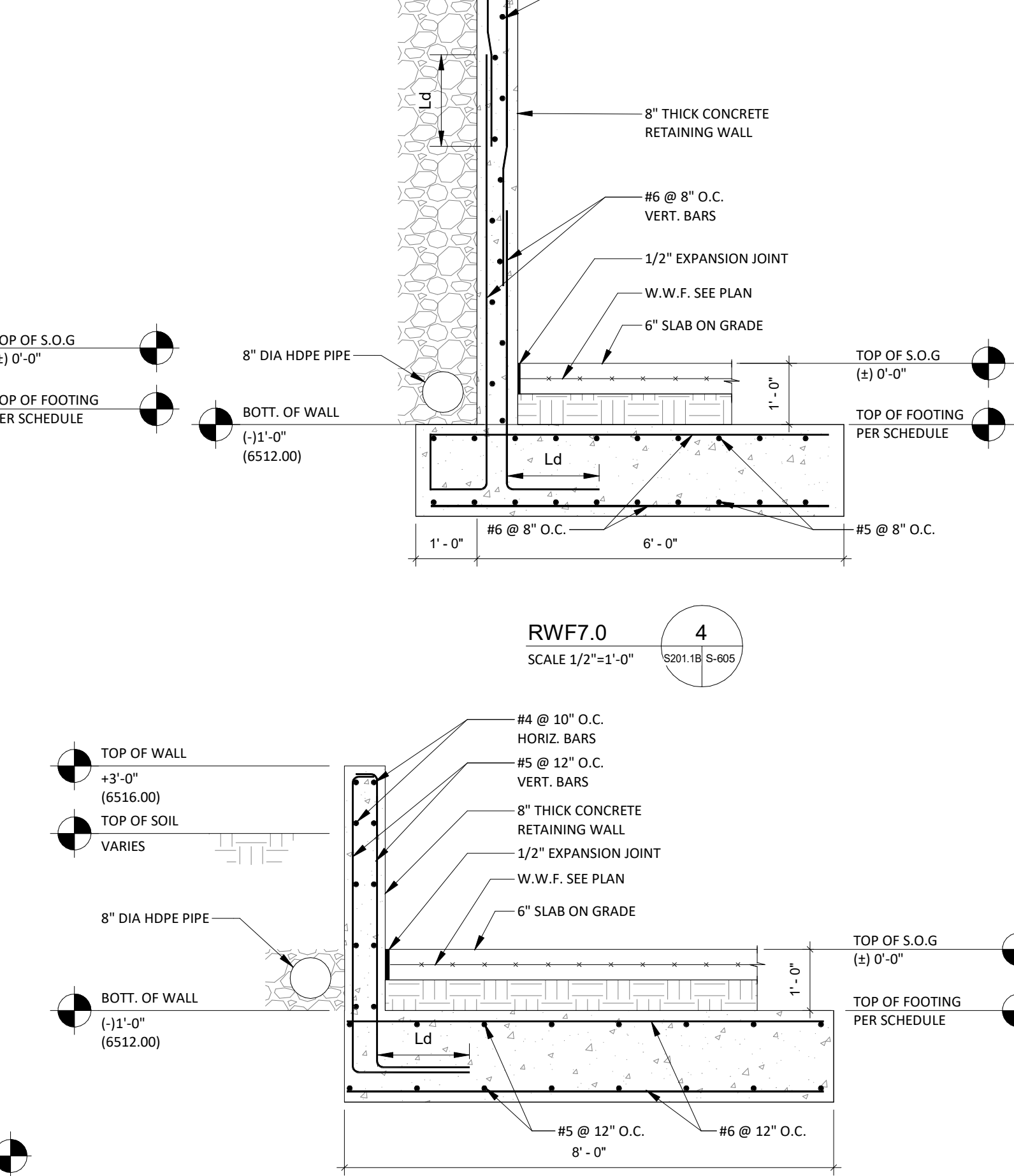
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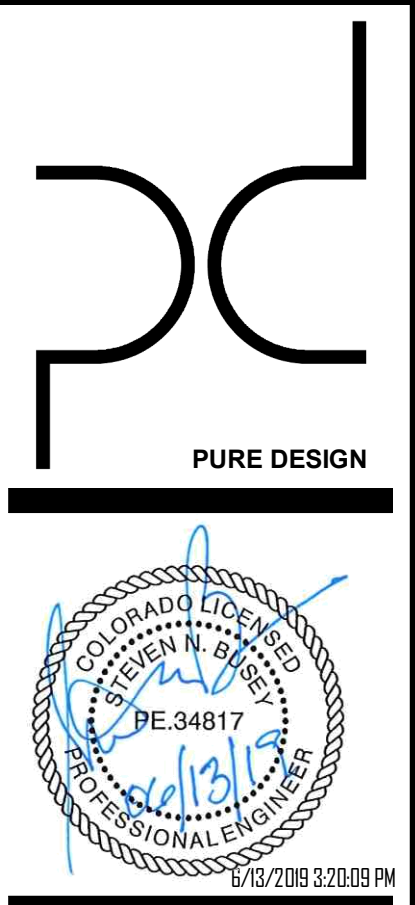


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S201.1A S-605



RWF8.0  
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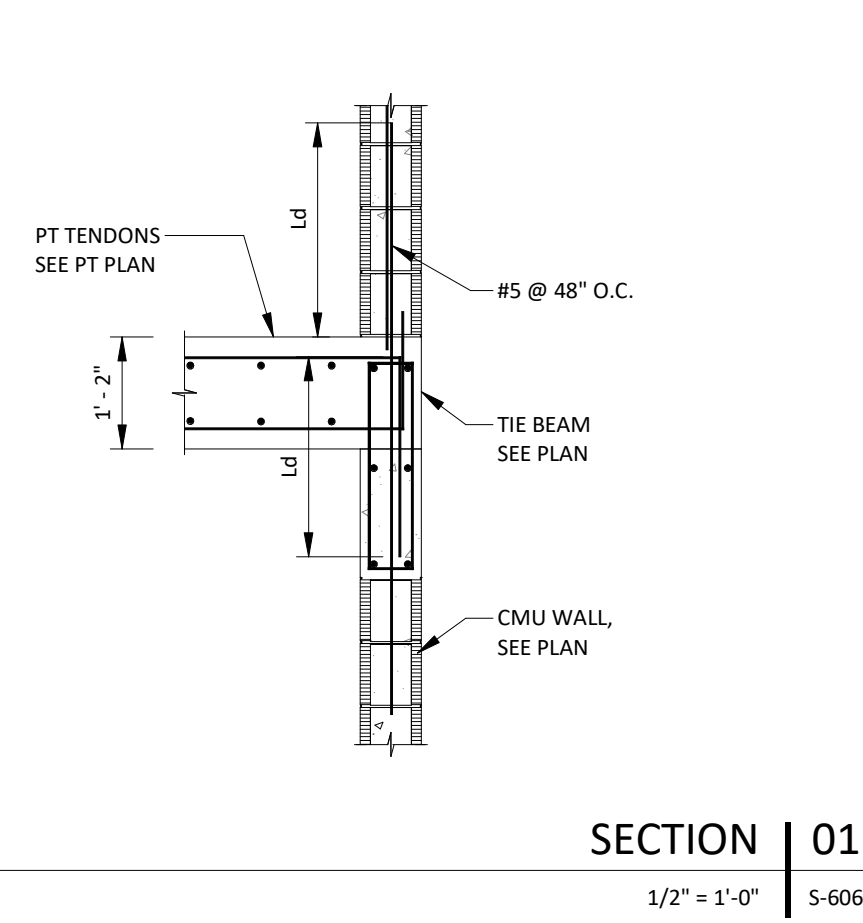
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1 05/17/2019

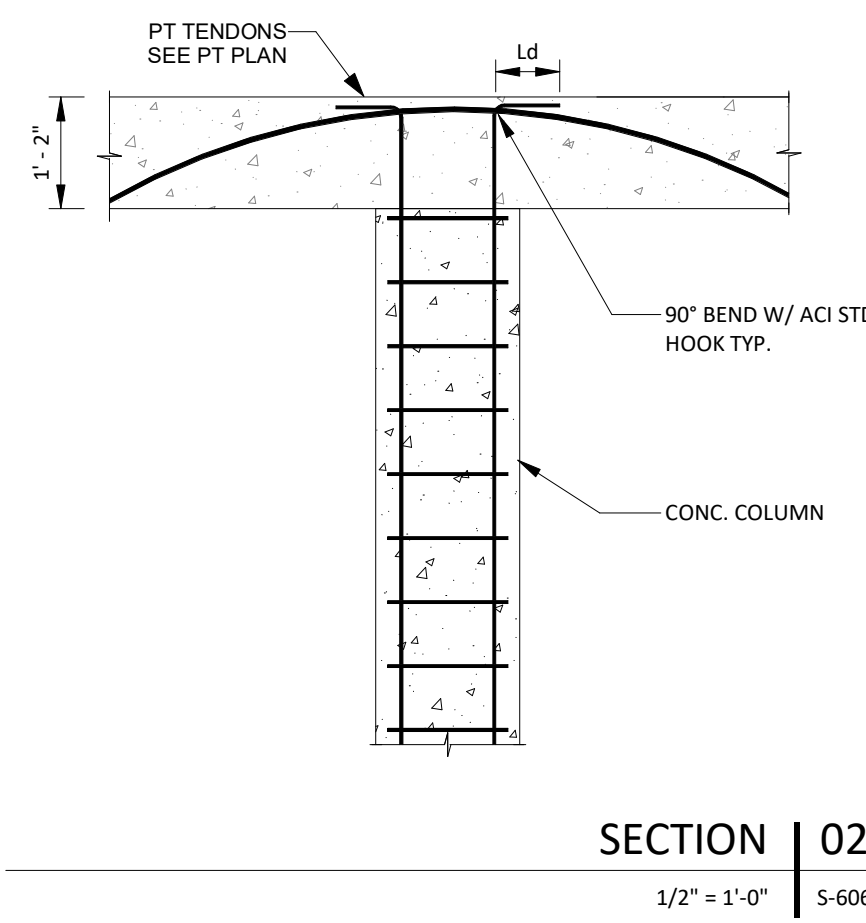
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TYPICAL DETAIL  
**S-605**

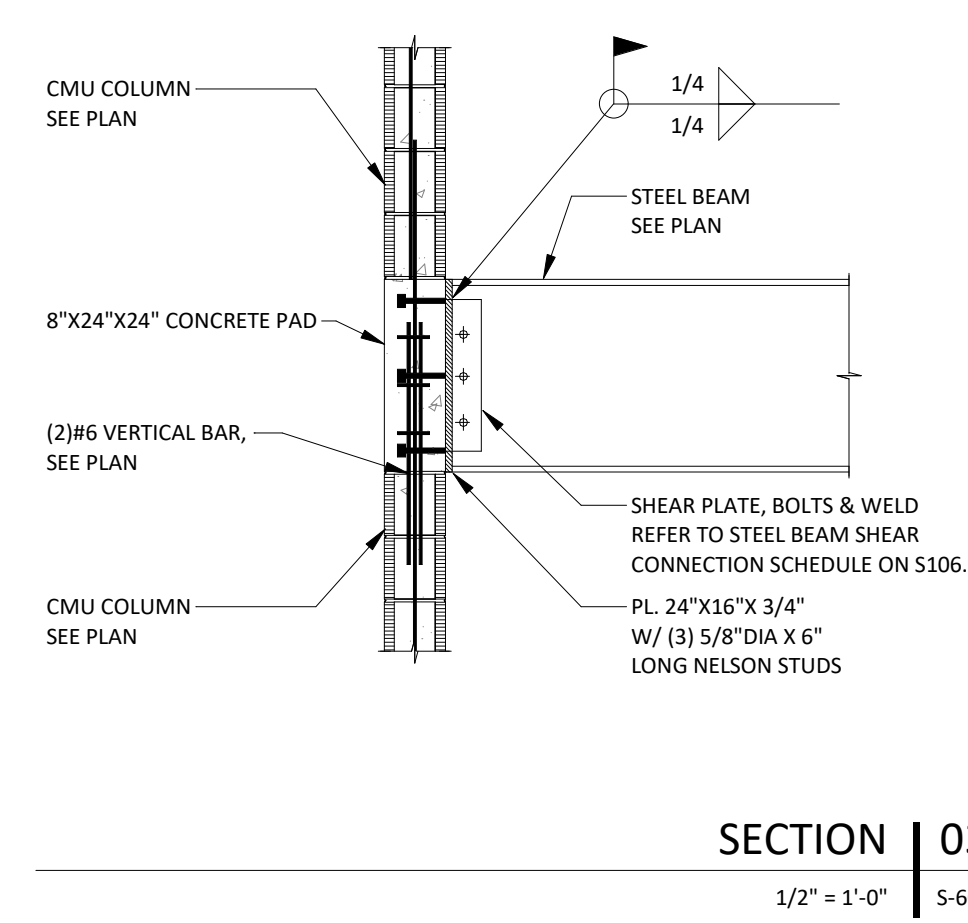




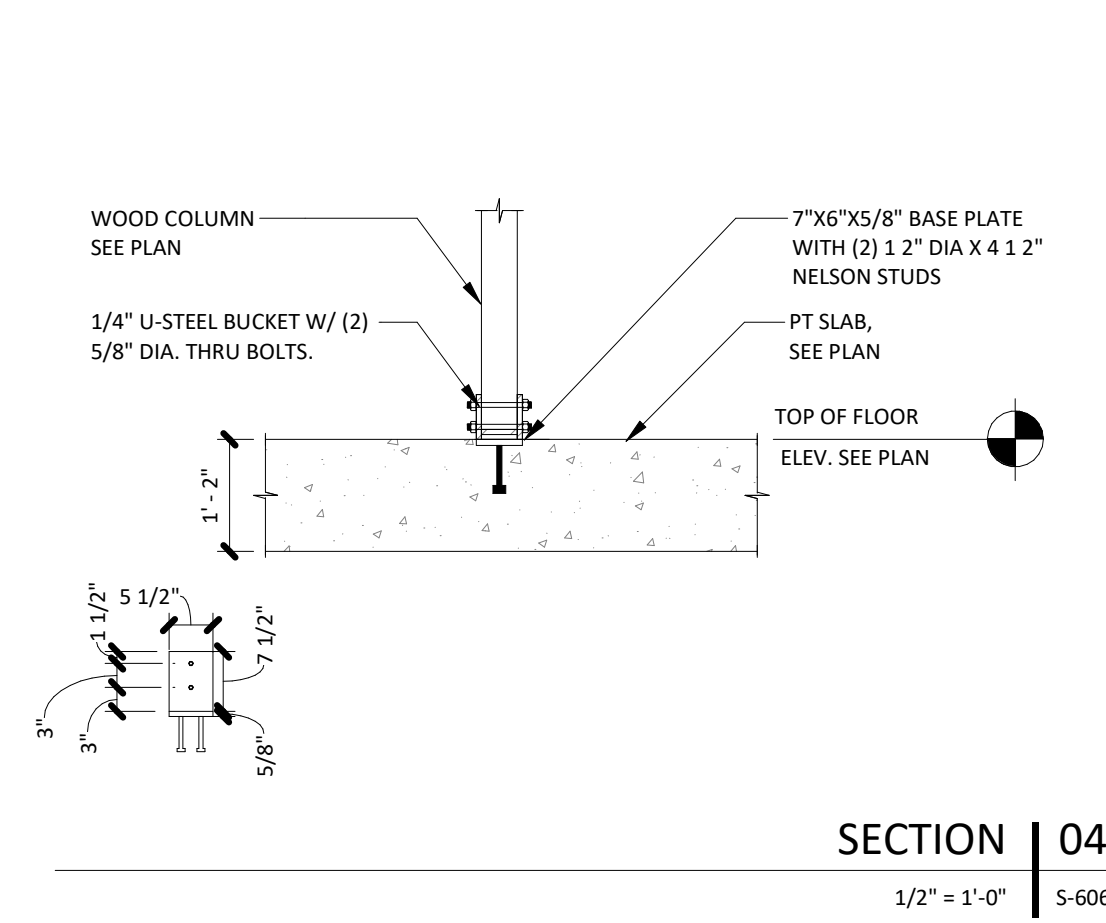
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1/2" = 1'-0" S-606



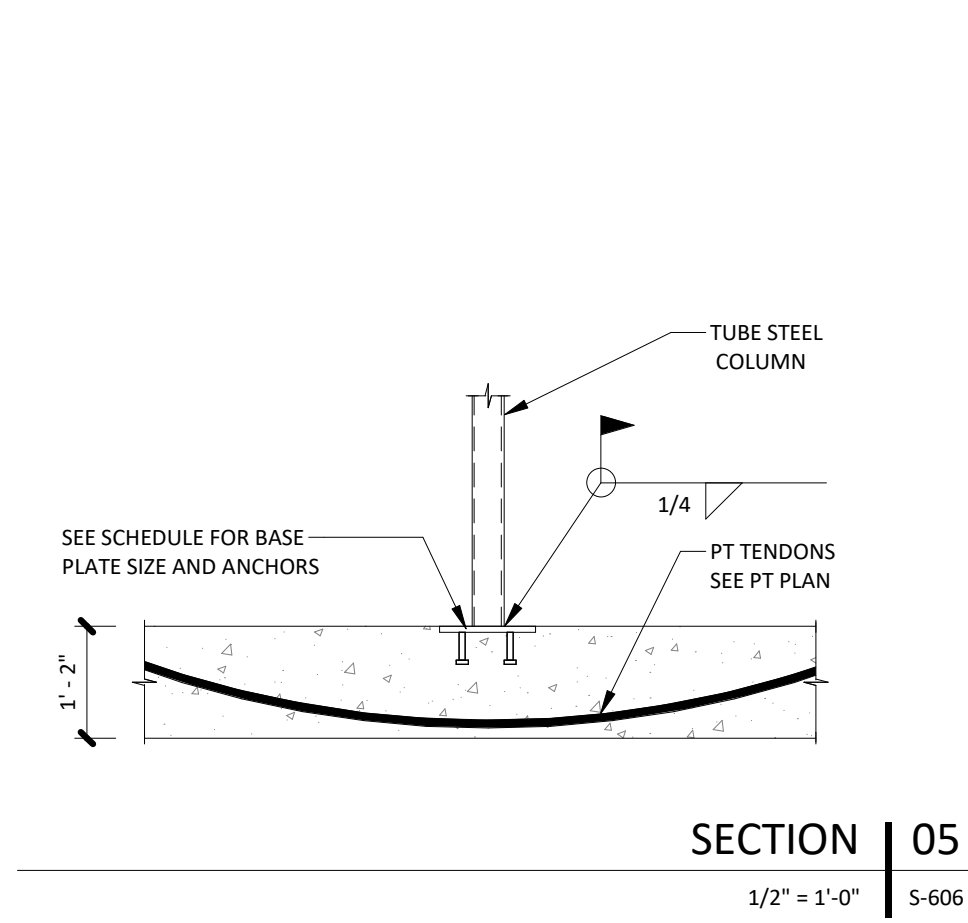
SECTION 02  
1/2" = 1'-0" S-606



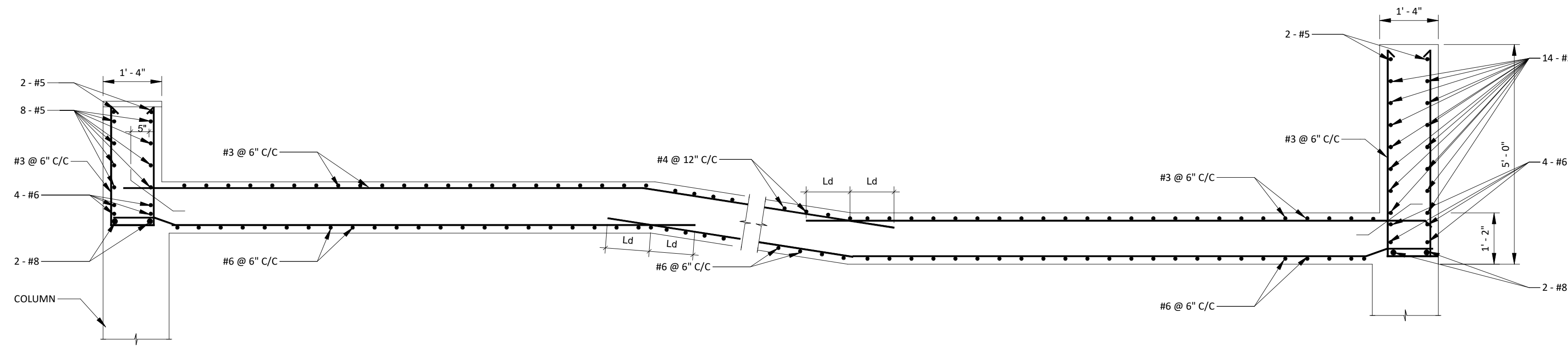
SECTION 03  
1/2" = 1'-0" S-606



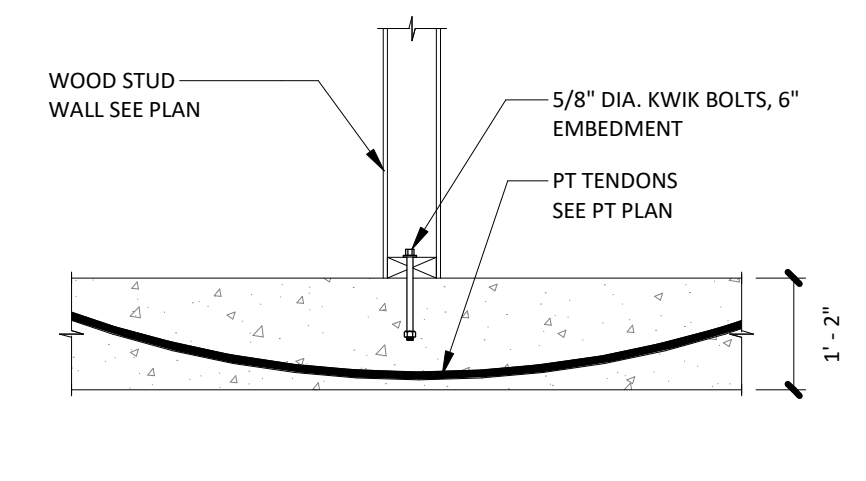
SECTION 04  
1/2" = 1'-0" S-606



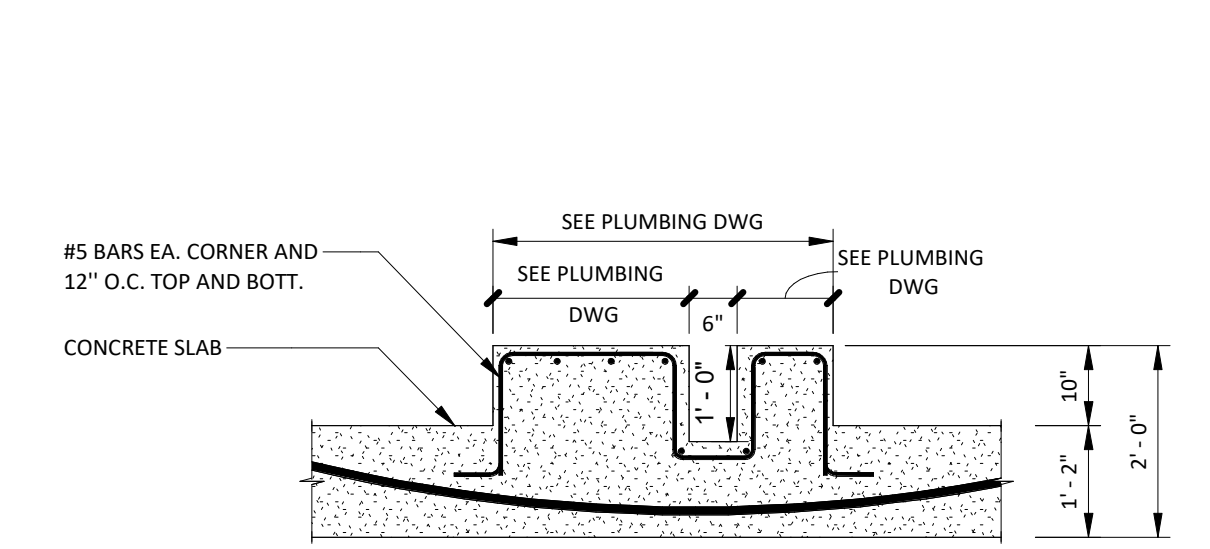
SECTION 05  
1/2" = 1'-0" S-606



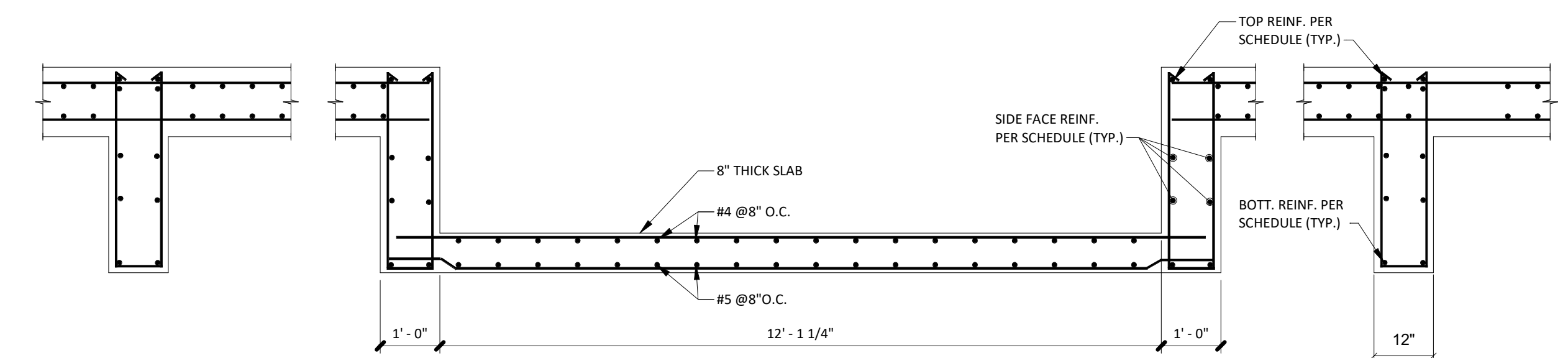
SECTION 06  
1/2" = 1'-0" S-606



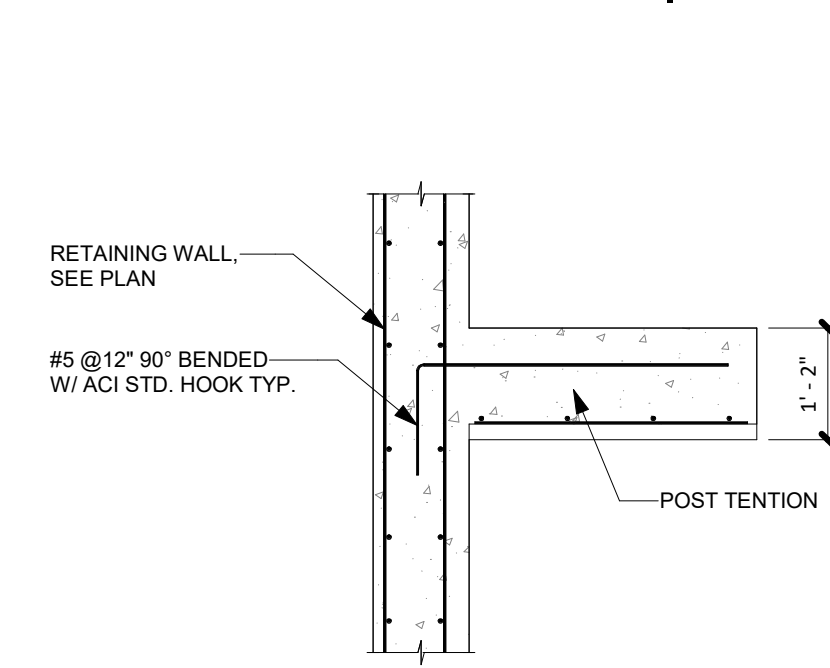
SECTION 07  
1/2" = 1'-0" S-606



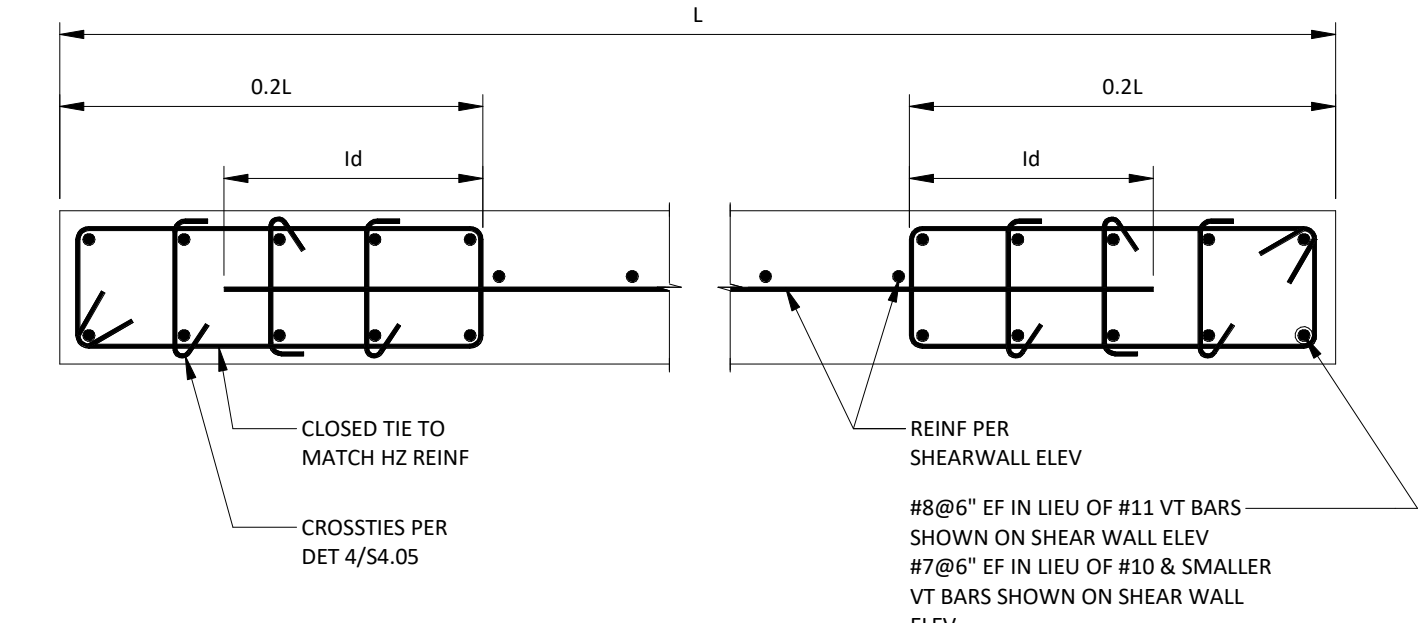
SECTION 08  
1/2" = 1'-0" S-606



SECTION 09  
1/2" = 1'-0" S-606



SECTION 10  
1/2" = 1'-0" S-606



SECTION 11  
1" = 1'-0" S-606

COMPRESSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMAL WEIGHT CONCRETE						
BAR SIZE	CONCRETE COMPRESSIVE STRENGTH					
	3000 PSI		4000 PSI		5000 PSI	
	DEV	SPLICE	DEV	SPLICE	DEV	SPLICE
#3	9	12	8	12	7	12
#4	11	15	10	15	9	15
#5	14	19	12	19	12	19
#6	17	23	15	23	14	23
#7	20	27	17	27	16	27
#8	22	30	19	30	18	30
#9	25	34	22	34	21	34
#10	28	39	25	39	23	39
#11	31	43	27	43	26	43

- NOTES:
- 1.) TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.
  - 2.) COMPRESSION DEVELOPMENT LENGTHS AND COMPRESSION SPLICE LENGTHS ARE BASED ON ACI 318, SECTIONS 12.3 AND 12.16, RESPECTIVELY.
  - 3.) ALL VALUES ARE SHOWN IN INCHES
  - 4.) COMPRESSION SPLICE PERMISSIBLE ONLY WHERE SPECIFICALLY NOTED
  - 5.) TABLE IS NOT APPLICABLE FOR EPOXY-COATED REINFORCEMENT.
  - 6.) "SIDE LAP" ALL LAP SPLICES TO MAINTAIN SPECIFIED CONCRETE COVER.
  - 7.) WHEN BARS OF A DIFFERENT SIZE ARE LAP SPICED, THE SPLICE LENGTH SHALL BE THE LARGER OF THE DEVELOPMENT LENGTH OF THE LARGER BAR, OR THE SPLICE LENGTH OF THE SMALLER BAR.

COMPRESSION DEVEL. & LAP SPLICE TABLE | 12  
1/2" = 1'-0" S-606

TENSION LAP SPLICE LENGTHS (in) GRADE 60 UNCOATED BARS f <sub>c</sub> =4000 psi					
BAR SIZE	LAP CLASS	TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2
#3	A	19	28	15	22
	B	24	36	19	28
#4	A	25	37	19	29
	B	32	48	25	37
#5	A	31	47	24	36
	B	40	60	31	47
#6	A	37	56	29	43
	B	48	72	37	56
#7	A	54	81	42	63
	B	70	106	54	81
#8	A	62	93	48	72
	B	80	121	62	93
#9	A	70	105	54	81
	B	91	136	70	105
#10	A	79	118	61	91
	B	102	153	79	118
#11	A	87	131	67	101
	B	113	170	87	131

- NOTES:
1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL-WEIGHT CONCRETE.
  2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICE LENGTHS ARE BASED ON ACI 318, SECTIONS 12.2.2 AND 12.15, RESPECTIVELY.
  3. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS. LENGTHS ARE IN INCHES.
  4. CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS ARE DEFINED AS:  
  
BEAMS OR COLUMNS:  
CASE 1: COVER AT LEAST (1) BAR DIAMETER AND C.-C. SPACING AT LEAST (2) BAR DIAMETERS  
CASE 2: COVER LESS THAN (1) BAR DIAMETER AND C.-C. SPACING LESS THAN (2) BAR DIAMETERS  
  
ALL OTHERS:  
CASE 1: COVER AT LEAST (1) BAR DIAMETER AND C.-C. SPACING AT LEAST (3) BAR DIAMETERS  
CASE 2: COVER LESS THAN (1) BAR DIAMETER AND C.-C. SPACING LESS THAN (3) BAR DIAMETERS
  5. LAP CLASS A VALUES ARE THE REQUIRED TENSION DEVELOPMENT LENGTHS, L<sub>d</sub>; LAP SPLICE LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS; CLASS A - 1.0l<sub>d</sub> AND CLASS B = 1.3l<sub>d</sub> (ACI 318, SECTION 12.15.1)
  6. LAP CLASS B SHALL BE USED FOR ALL CASES UNLESS APPROVED BY E.O.R
  7. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
  - 8.) LENGTHS SHOWN ARE FOR UNCOATED BARS. LENGTHS SHOWN SHALL BE MULTIPLIED BY 1.2 FOR ALL EPOXY COATED BARS (ACI 318 SECTION 12.2.4)
  - 9.) WHEN BARS OF DIFFERENT SIZES ARE LAP SPICED, THE SPLICE LENGTH FOR THE LARGER BAR SHALL BE USED.

LAP SPLICE LENGTHS f<sub>c</sub>=4000 psi | 13  
1/2" = 1'-0" S-606

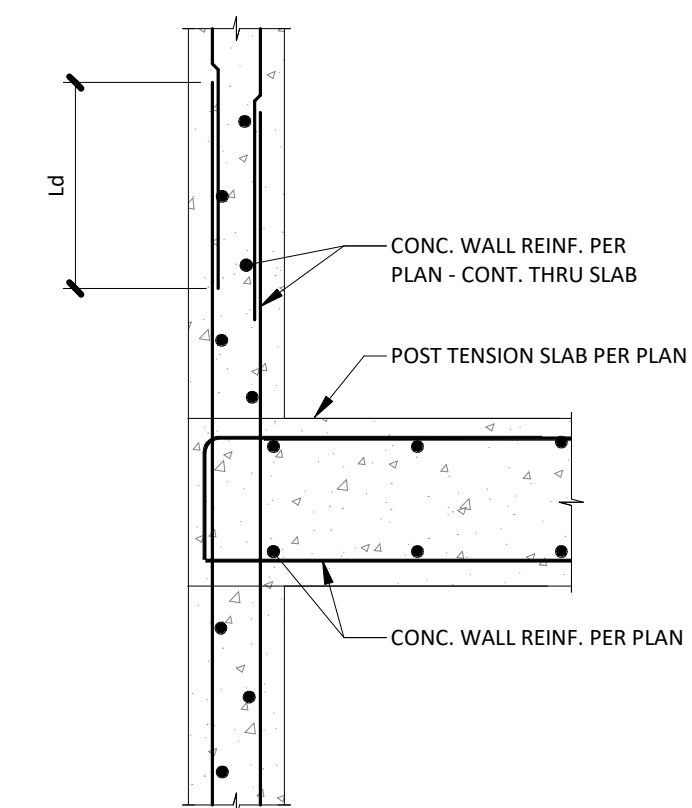
TENSION LAP SPLICE LENGTHS (in) GRADE 60 UNCOATED BARS f <sub>c</sub> =5000 psi					
BAR SIZE	LAP CLASS	TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2
#3	A	17	25	13	19
	B	22	33	17	25
#4	A	22	33	17	26
	B	29	43	22	33
#5	A	28	42	22	32
	B	36	54	28	42
#6	A	33	50	26	38
	B	43	65	33	50
#7	A	49	73	37	56
	B	63	94	49	73
#8	A	55	83	43	64
	B	72	108	55	83
#9	A	63	94	48	72
	B	81	122	63	94
#10	A	70	105	54	81
	B	91	137	70	105
#11	A	78	117	60	90
	B	101	152	78	117

- NOTES:
1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL-WEIGHT CONCRETE.
  2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICE LENGTHS ARE BASED ON ACI 318, SECTIONS 12.2.2 AND 12.15, RESPECTIVELY.
  3. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS. LENGTHS ARE IN INCHES.
  4. CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS ARE DEFINED AS:  
  
BEAMS OR COLUMNS:  
CASE 1: COVER AT LEAST (1) BAR DIAMETER AND C.-C. SPACING AT LEAST (2) BAR DIAMETERS  
CASE 2: COVER LESS THAN (1) BAR DIAMETER AND C.-C. SPACING LESS THAN (2) BAR DIAMETERS  
  
ALL OTHERS:  
CASE 1: COVER AT LEAST (1) BAR DIAMETER AND C.-C. SPACING AT LEAST (3) BAR DIAMETERS  
CASE 2: COVER LESS THAN (1) BAR DIAMETER AND C.-C. SPACING LESS THAN (3) BAR DIAMETERS
  5. LAP CLASS A VALUES ARE THE REQUIRED TENSION DEVELOPMENT LENGTHS, L<sub>d</sub>; LAP SPLICE LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS; CLASS A - 1.0l<sub>d</sub> AND CLASS B = 1.3l<sub>d</sub> (ACI 318, SECTION 12.15.1)
  6. LAP CLASS B SHALL BE USED FOR ALL CASES UNLESS APPROVED BY E.O.R
  7. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
  - 8.) LENGTHS SHOWN ARE FOR UNCOATED BARS. LENGTHS SHOWN SHALL BE MULTIPLIED BY 1.2 FOR ALL EPOXY COATED BARS (ACI 318 SECTION 12.2.4)
  - 9.) WHEN BARS OF DIFFERENT SIZES ARE LAP SPICED, THE SPLICE LENGTH FOR THE LARGER BAR SHALL BE USED.

STANDARD HOOK TABLE	
BAR SIZE	HOOK
#4	8 in.
#5	10 in.
#6	12 in.
#7	14 in.
#8	16 in.

LAP SPLICE LENGTHS f<sub>c</sub>=5000 psi | 14  
1/2" = 1'-0" S-606

STANDARD 90° HOOK TABLE | 15  
12" = 1'-0" S-606



REINFORCING WALL JOINT REIN. 16  
3/4" = 1'-0" S-606

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**DUAL HOTEL DURANGO - AC & HIS**

AC HOTELS

JOB NO: 18-007  
DATE: 2019.03.07

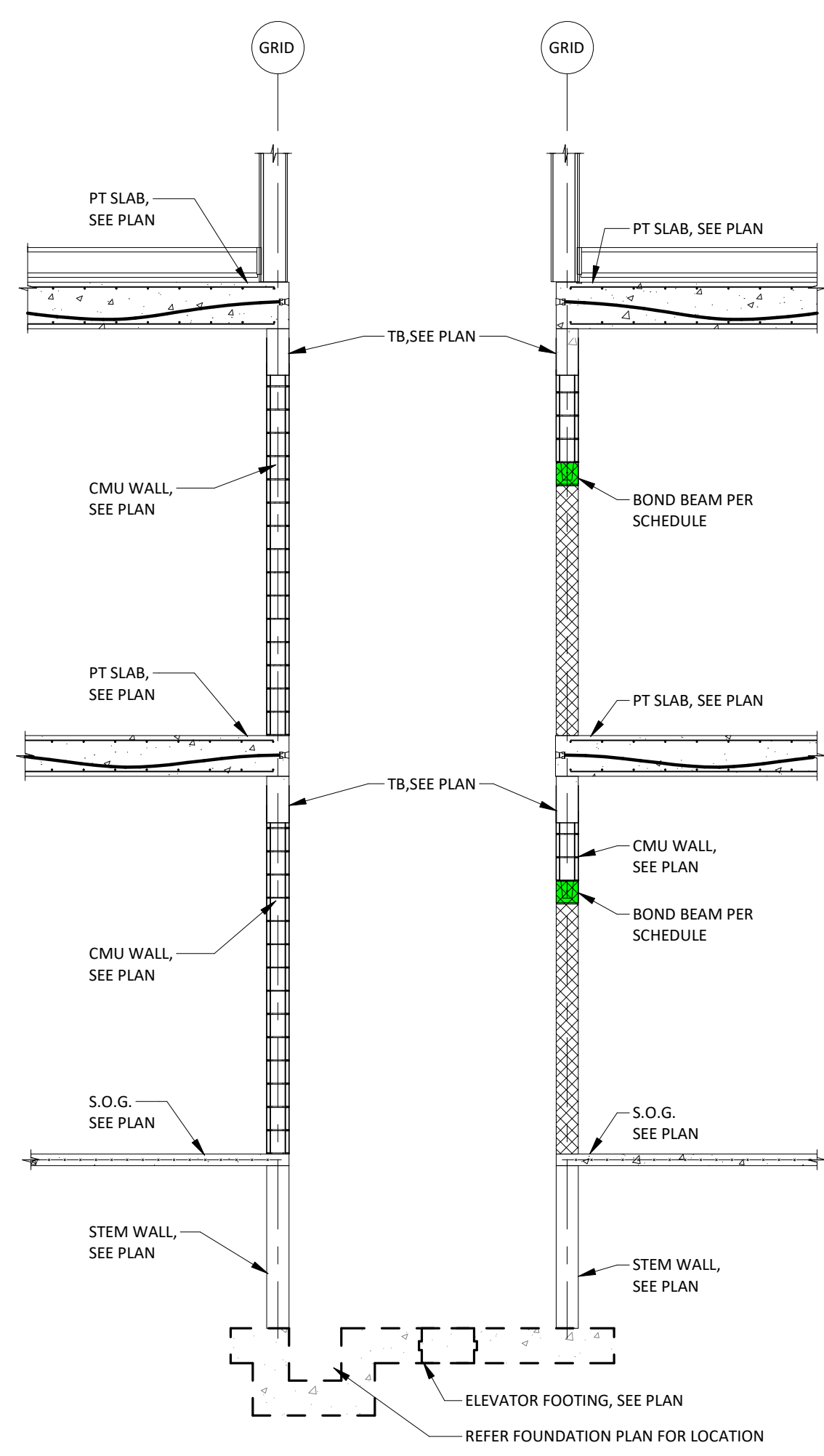
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1 05/17/2019

REVISIONS:  
1 05/17/2019

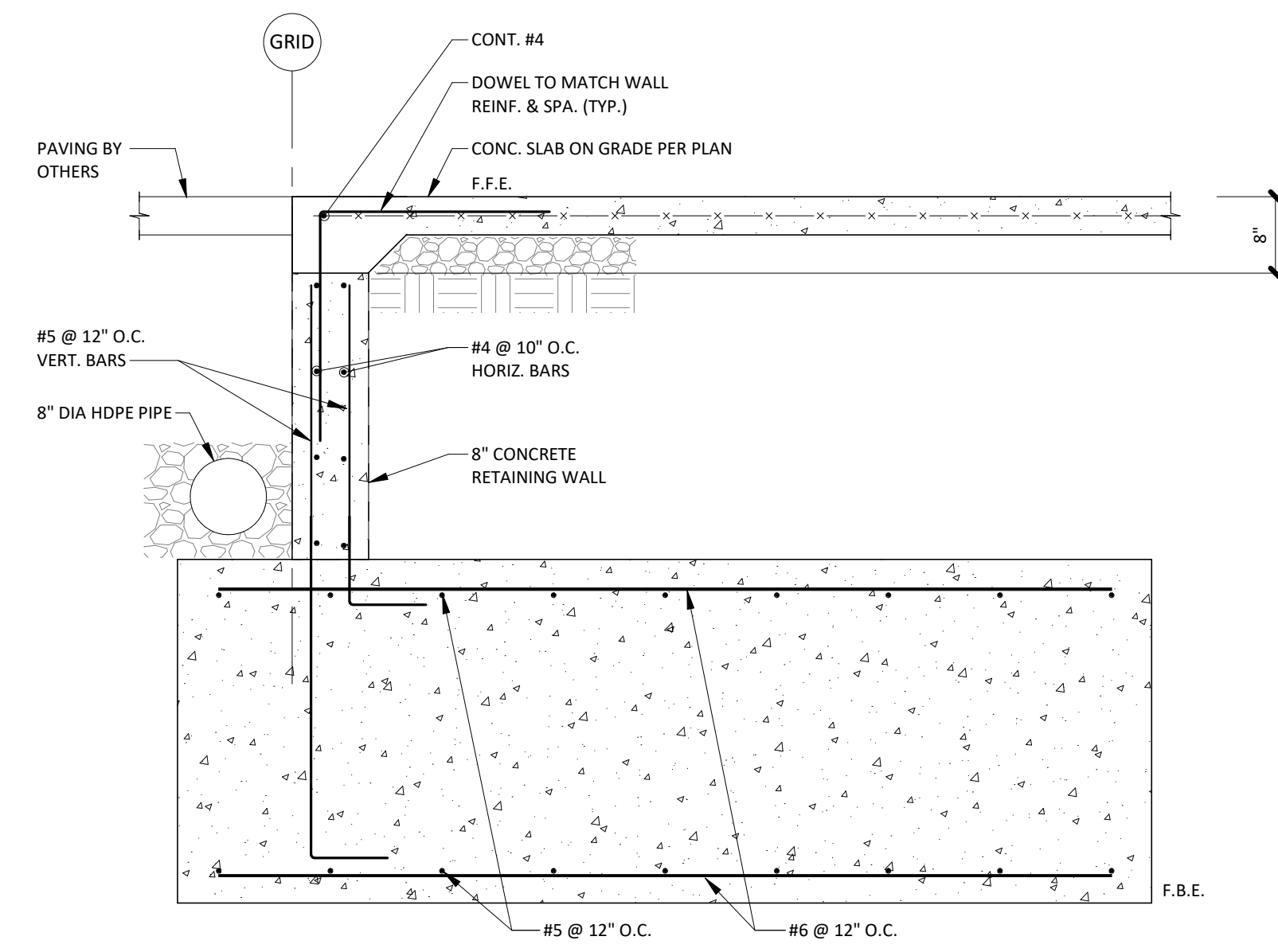
2019.06.14

TYPICAL DETAIL

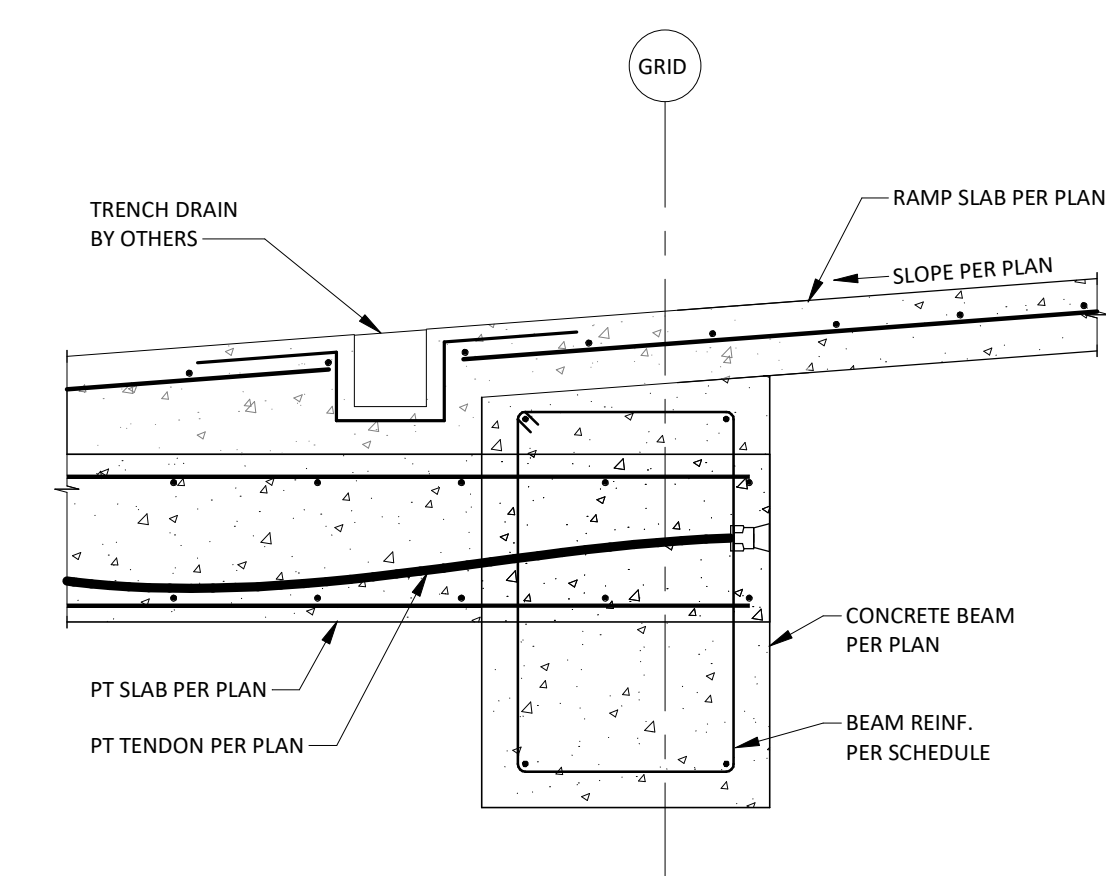
**S-606**



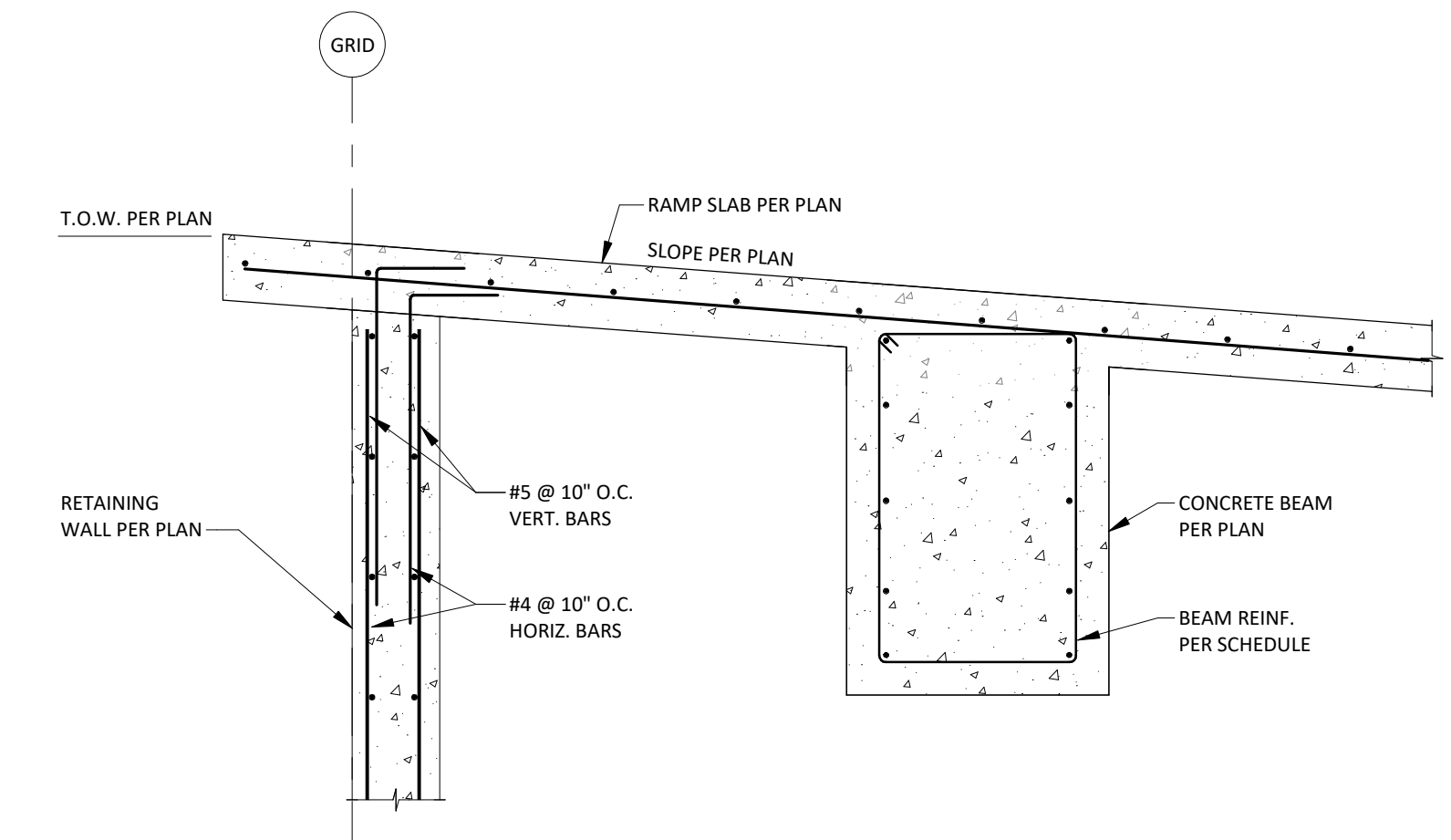
SECTION | 01  
1/4" = 1'-0" S-607



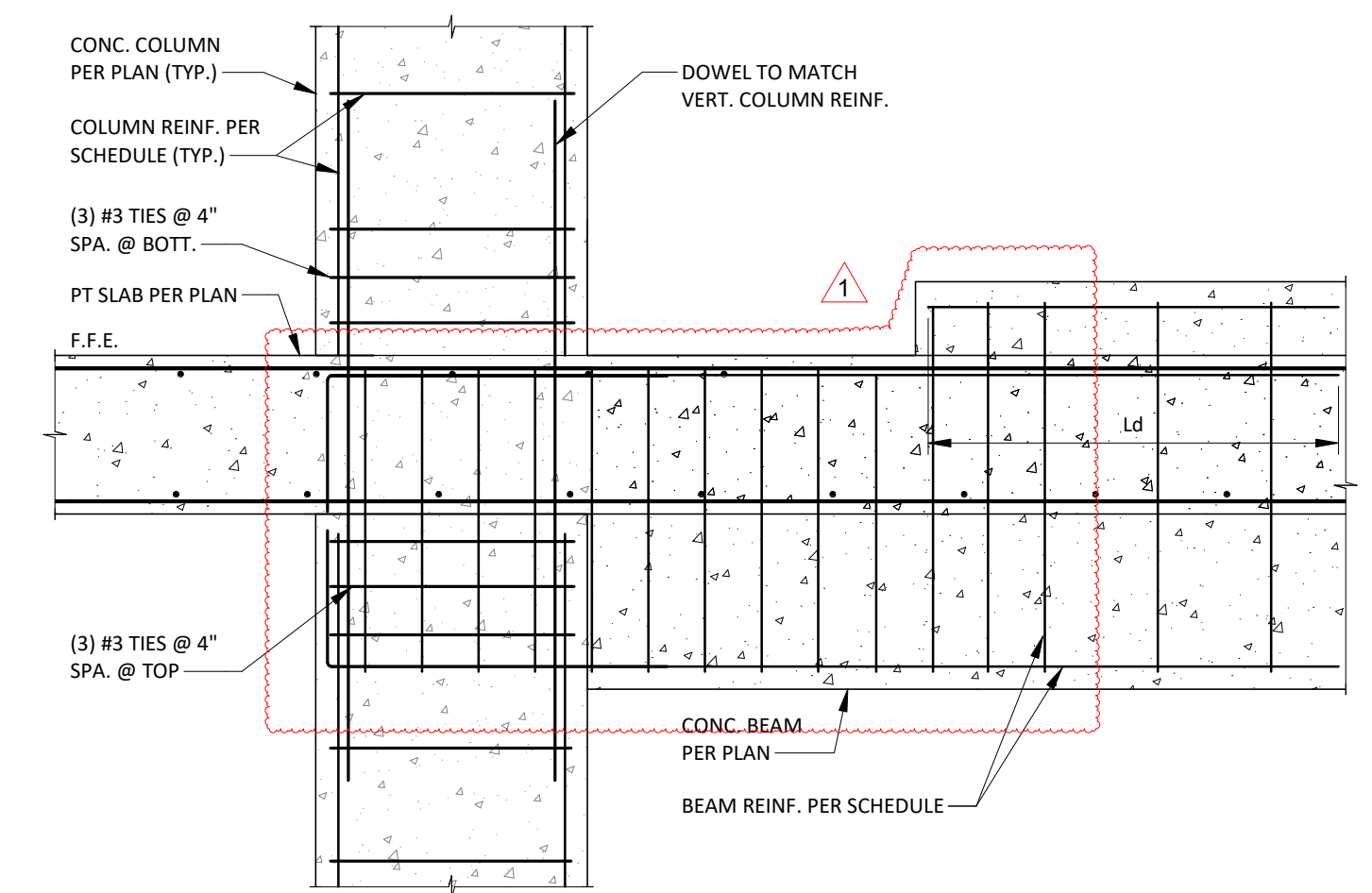
SECTION | 02  
3/4" = 1'-0" S-607



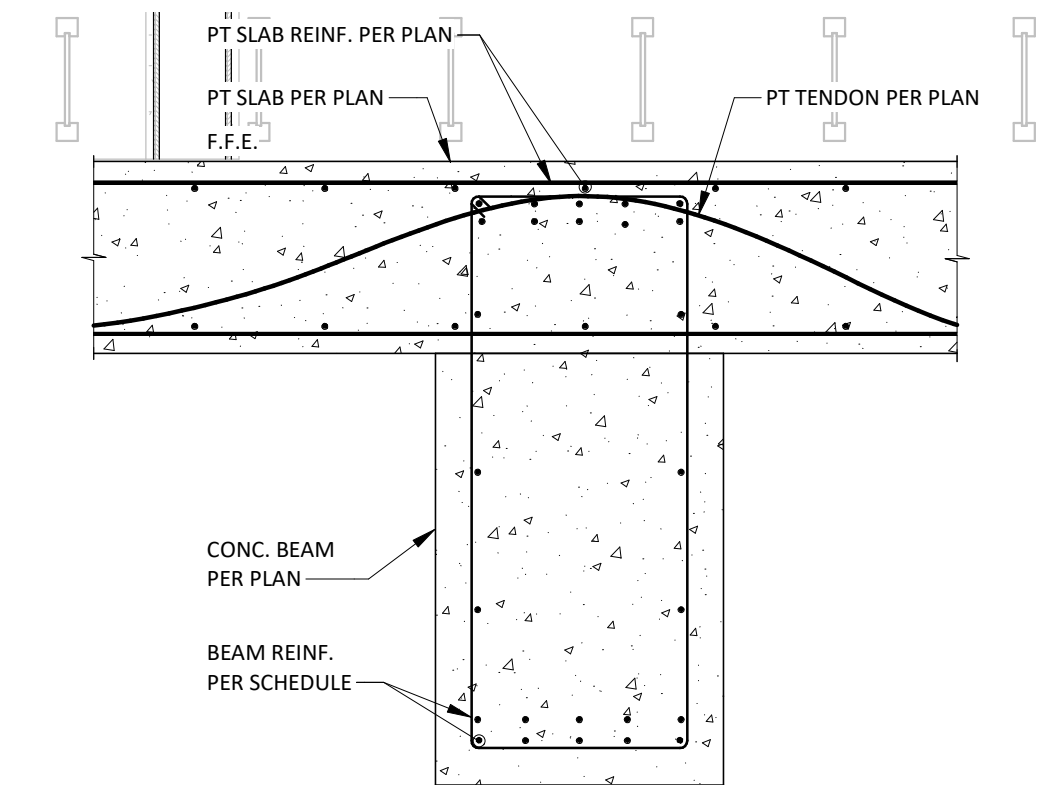
SECTION | 03  
3/4" = 1'-0" S-607



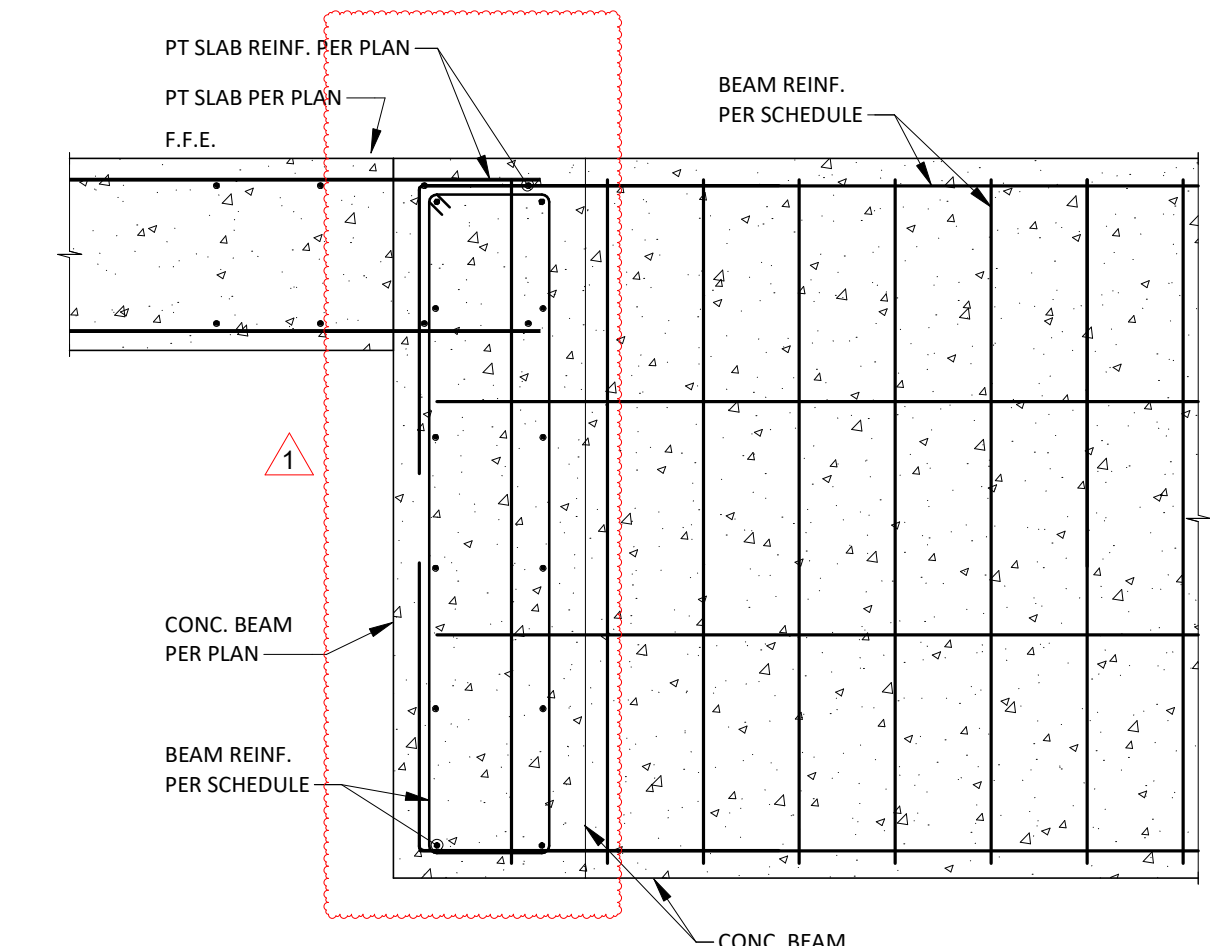
SECTION | 04  
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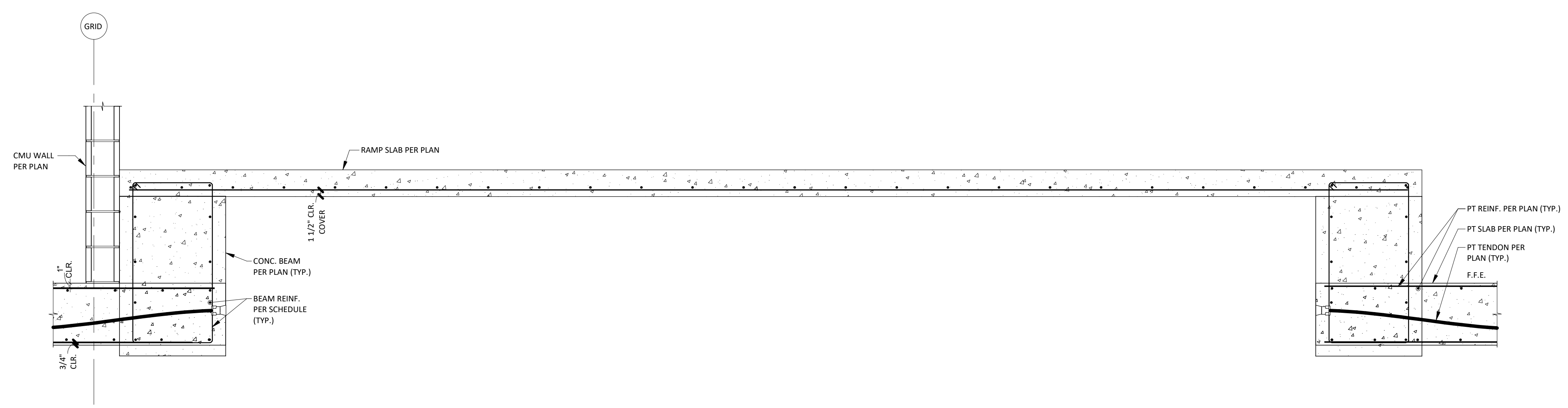
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3/4" = 1'-0" S-607



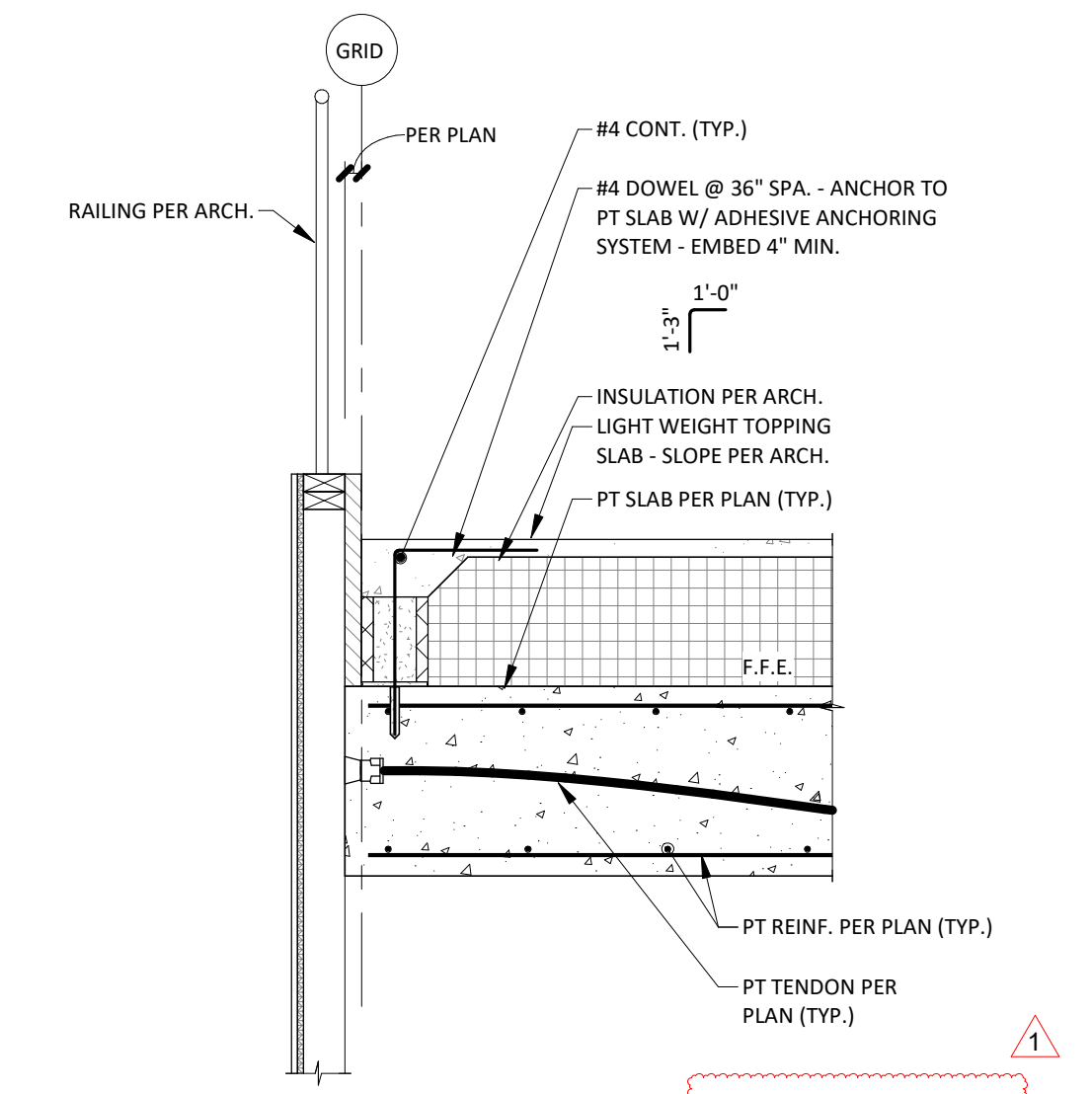
SECTION | 06  
3/4" = 1'-0" S-607



SECTION | 07  
3/4" = 1'-0" S-607



SECTION | 08  
3/4" = 1'-0" S-607



SECTION | 09  
3/4" = 1'-0" S-607

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**DUAL HOTEL DURANGO**  
**- AC & HIS**



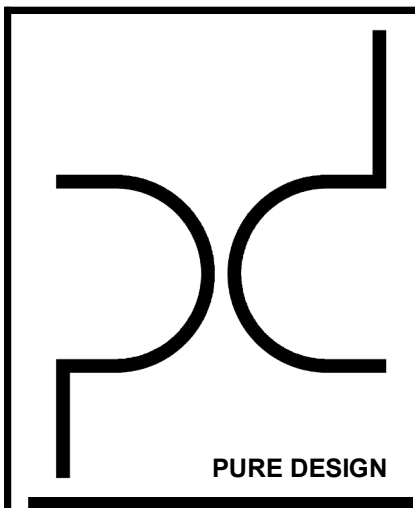
AC HOTELS

JOB NO: 18-007  
DATE: 2018.03.07  
ISSUE RECORD:  
REVISIONS:  
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GARAGE SECTIONS

**S-607**



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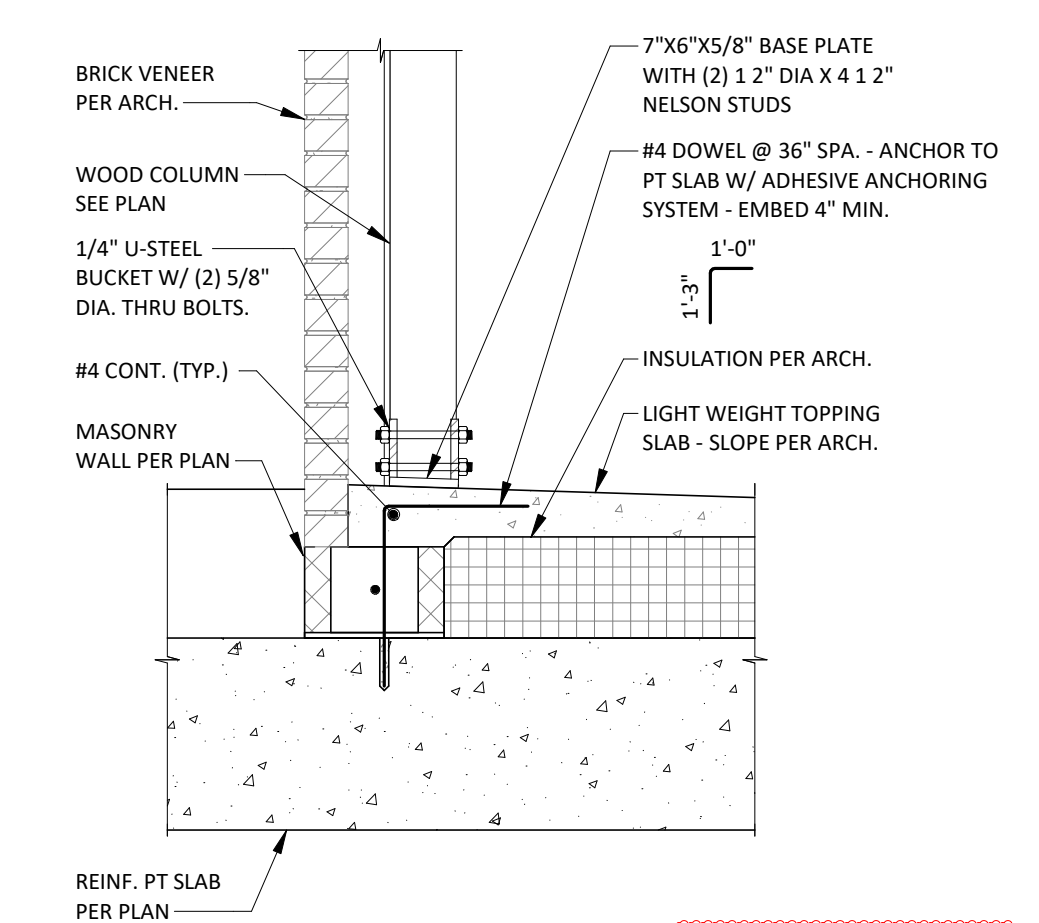
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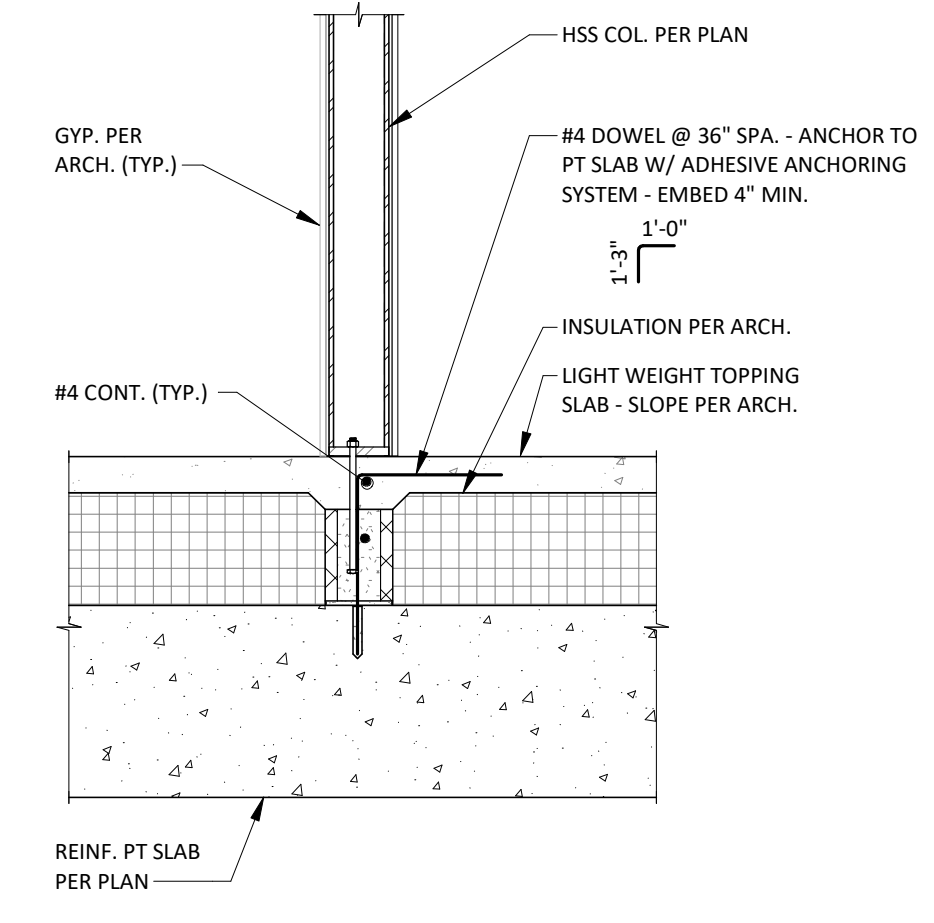
GARAGE SECTIONS

**S-608**

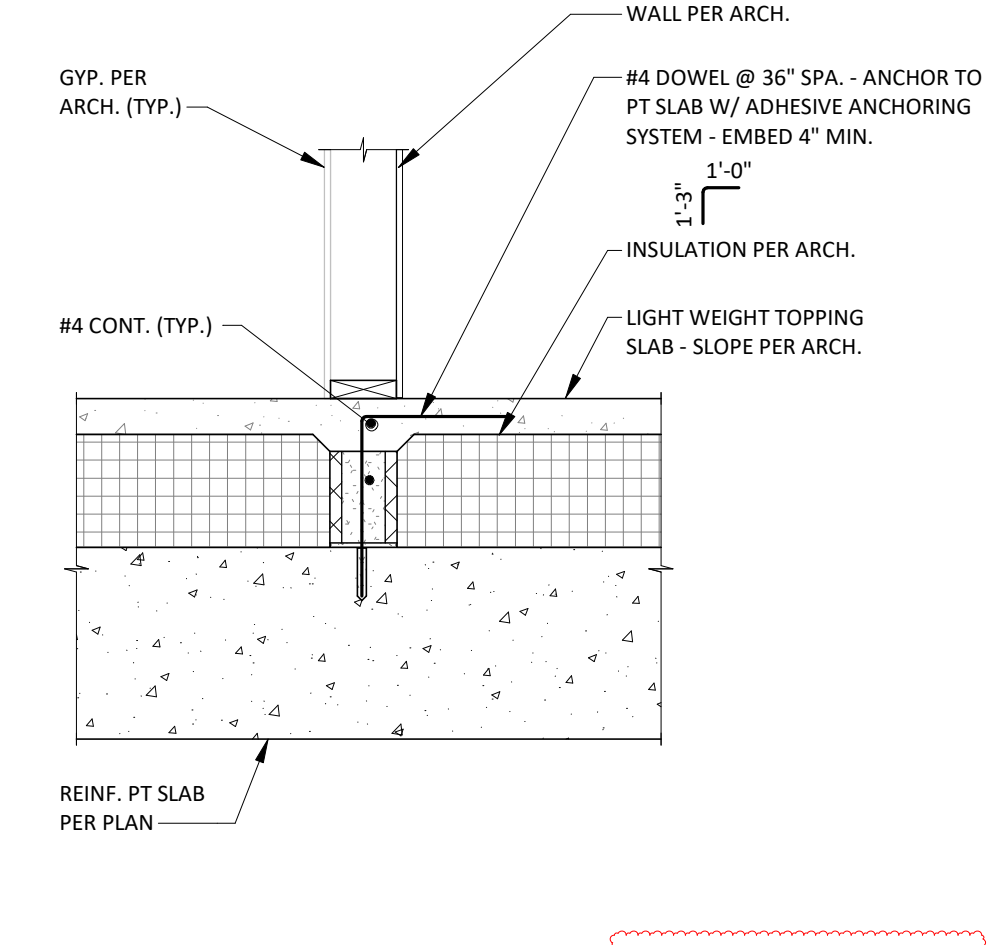
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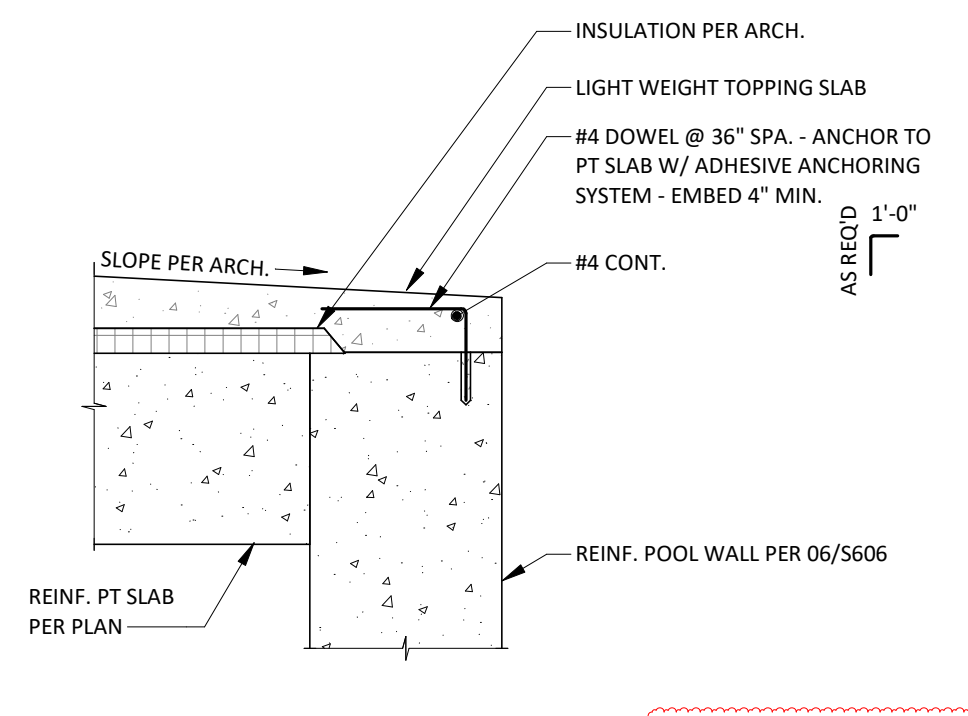
**SECTION 06**  
 3/4" = 1'-0" | 5-608



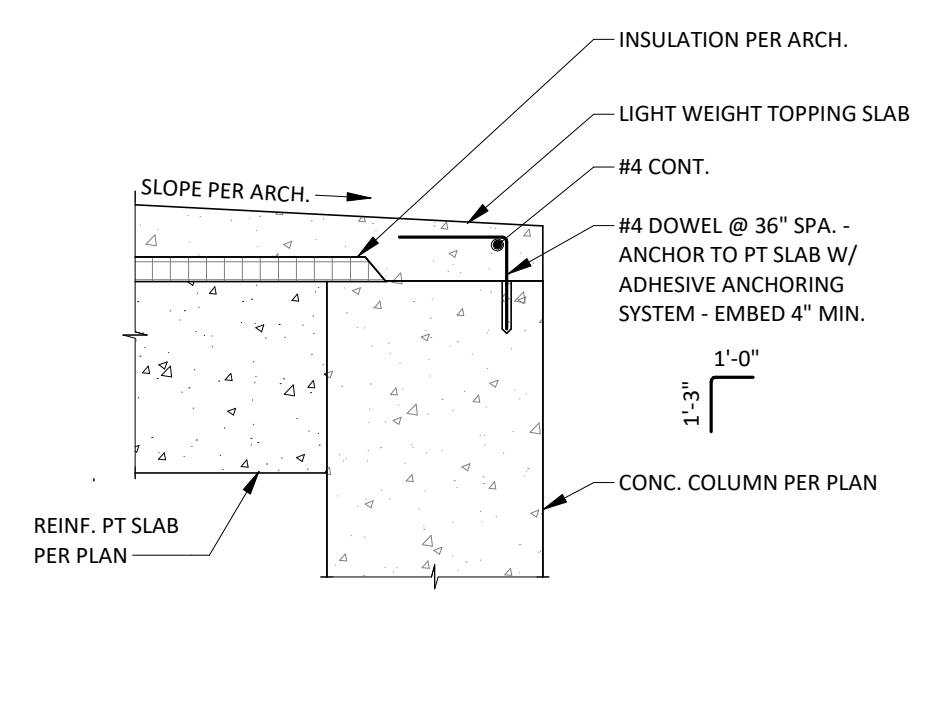
**SECTION 05**  
 3/4" = 1'-0" | 5-608



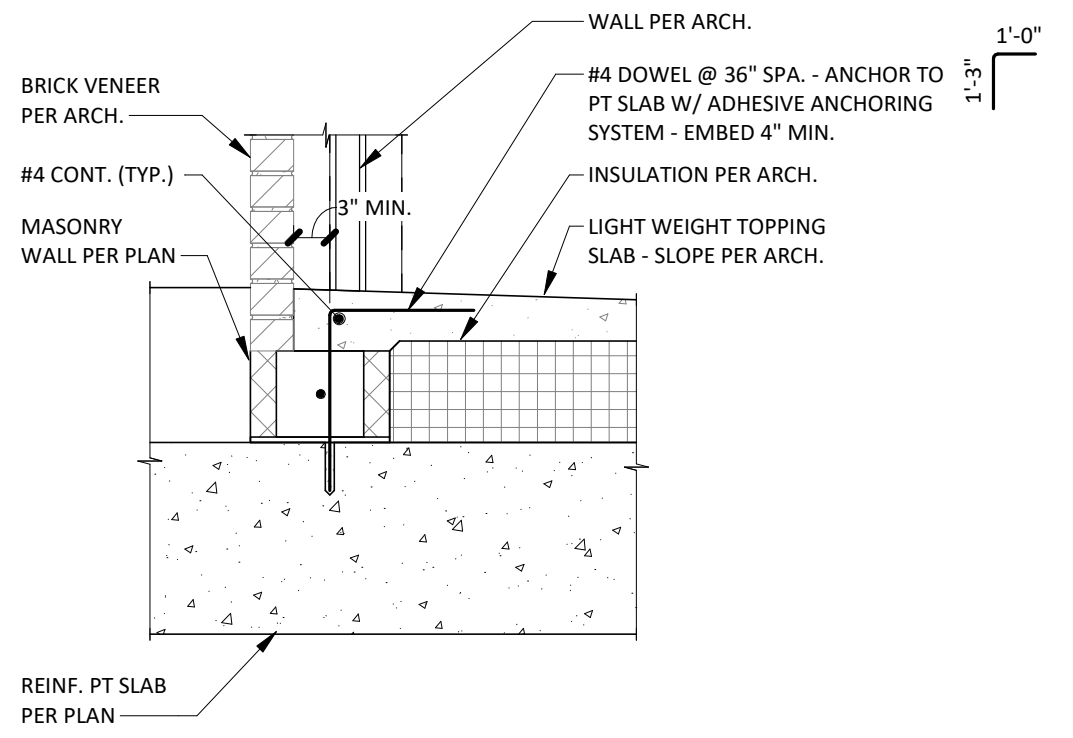
**SECTION 04**  
 3/4" = 1'-0" | 5-608



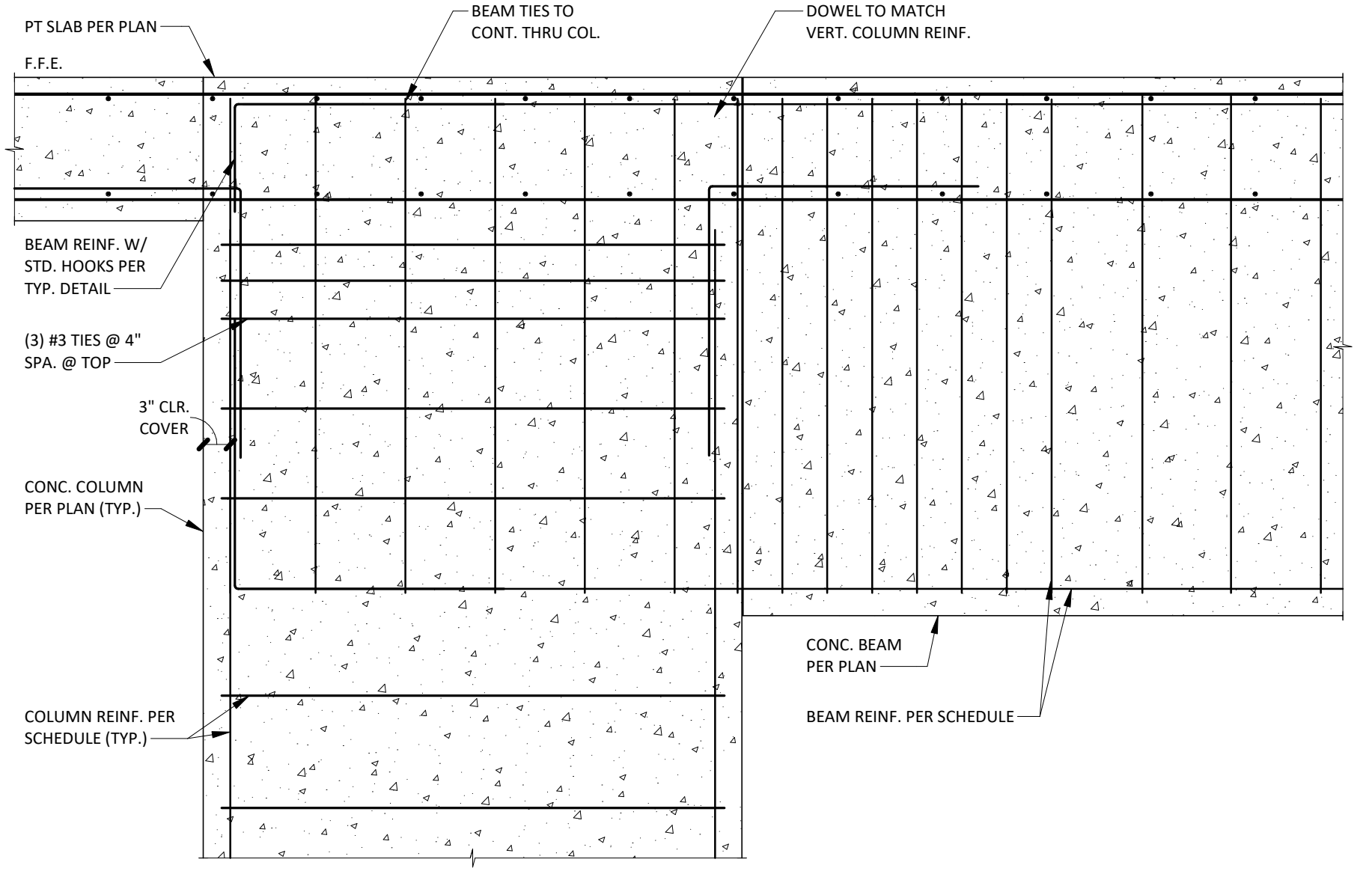
**SECTION 03**  
 3/4" = 1'-0" | 5-608



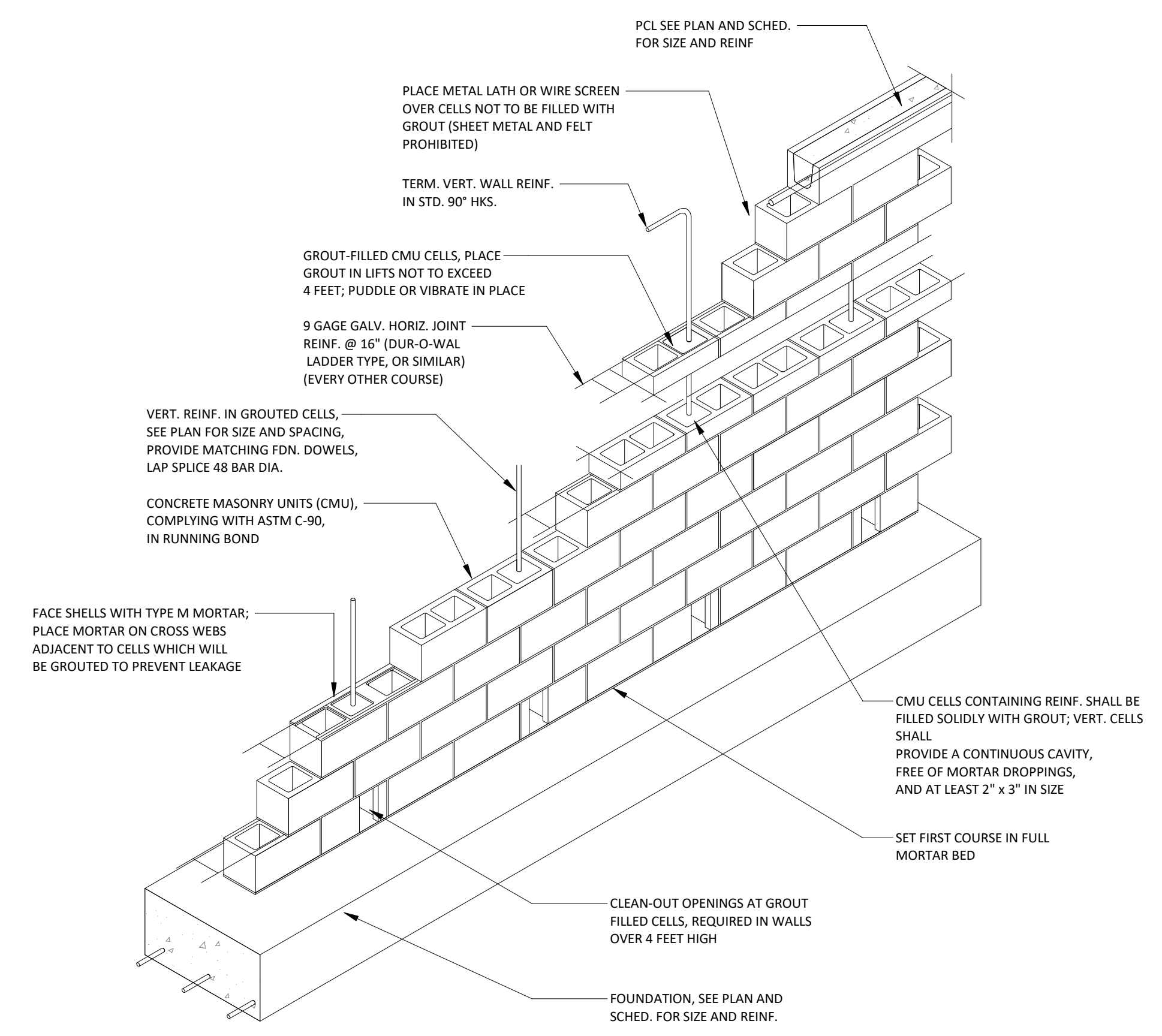
**SECTION 02**  
 3/4" = 1'-0" | 5-608



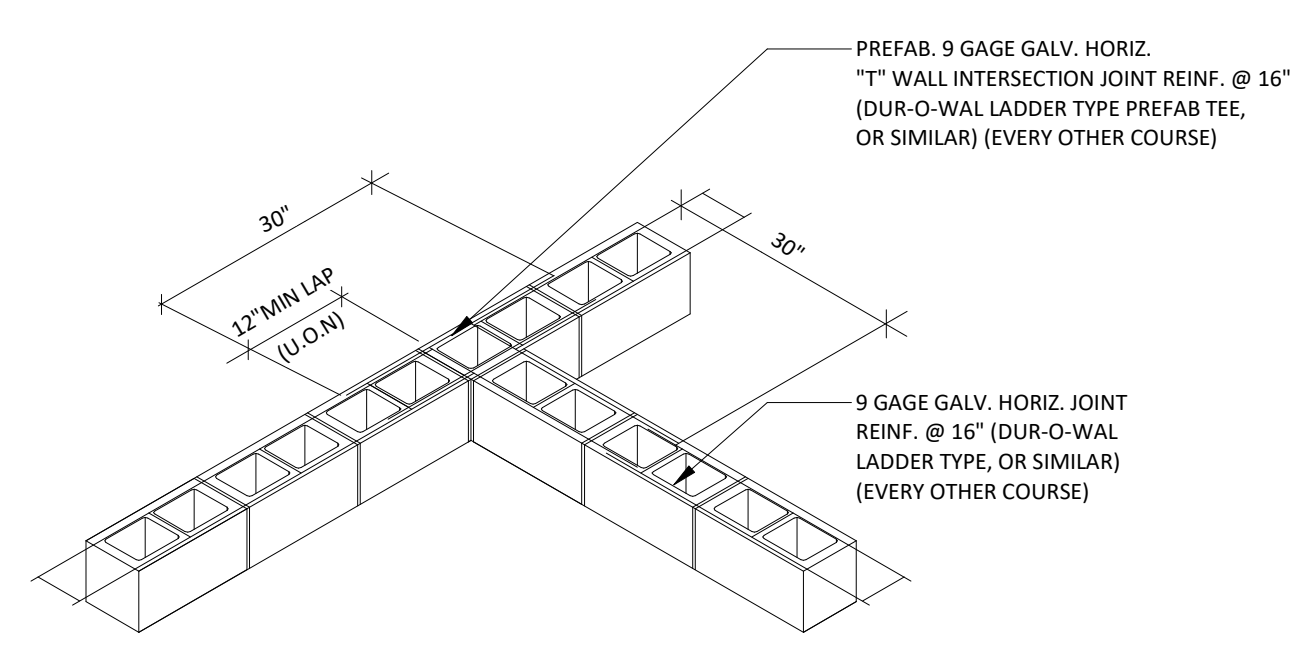
**SECTION 01**  
 3/4" = 1'-0" | 5-608



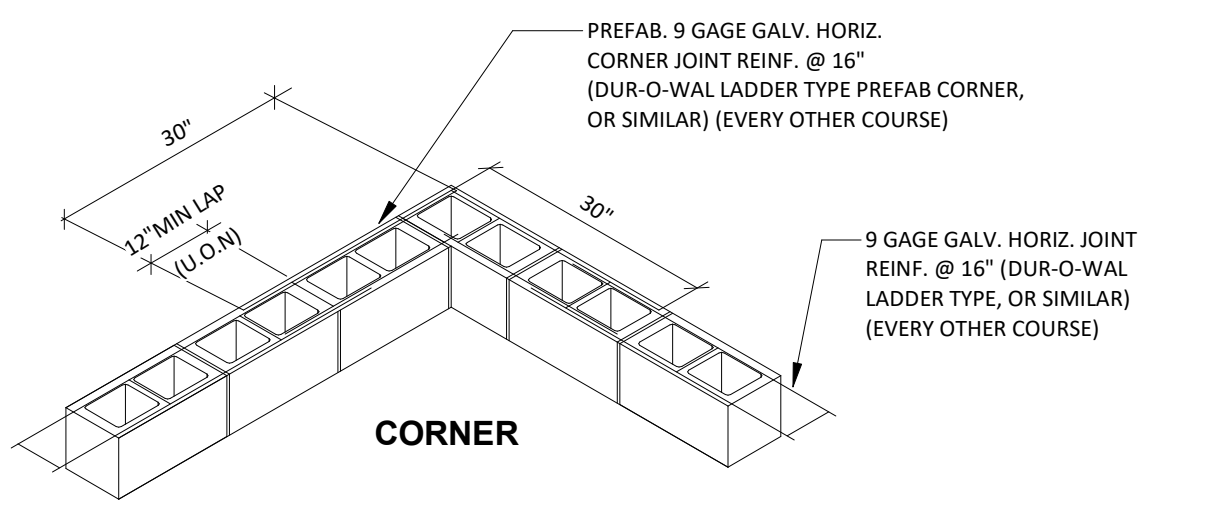
**SECTION 07**  
 3/4" = 1'-0" | 5-608



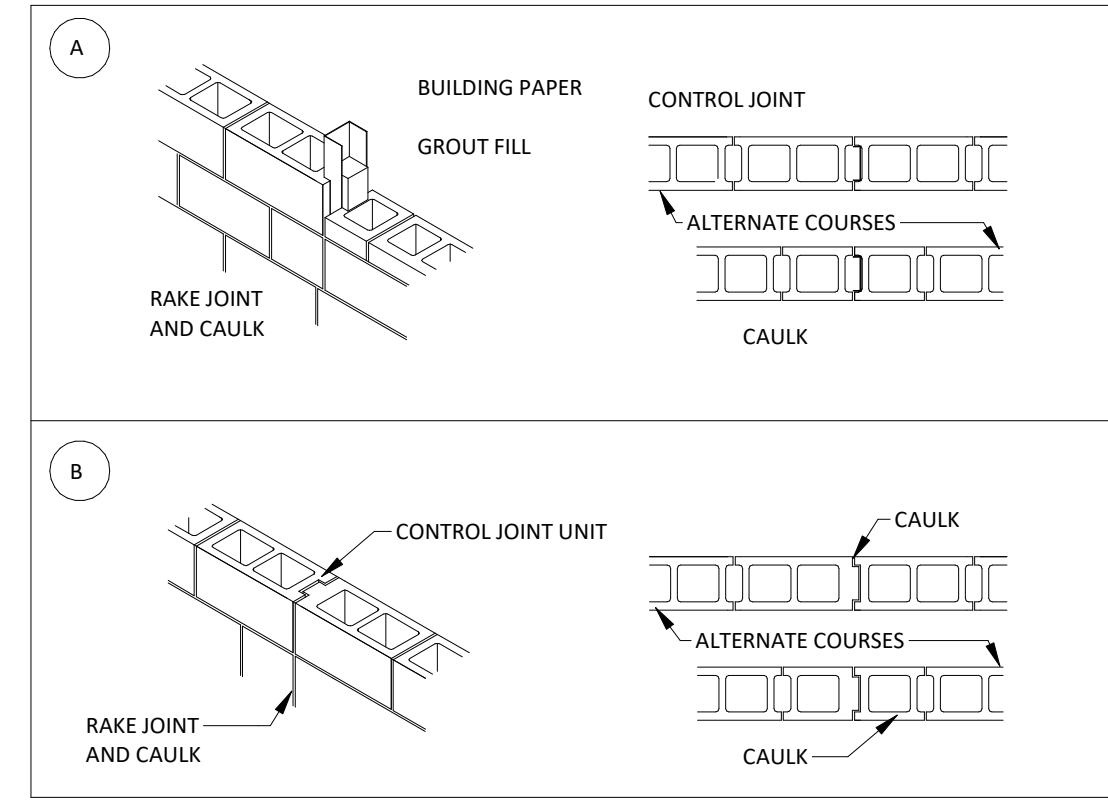
**TYPICAL REINFORCEMENT MASONRY WALL CONSTRUCTION**  
SCALE 1/2"=1'-0"



**TYPICAL TEE WALL INTERSECTION**  
SCALE 1/2"=1'-0"

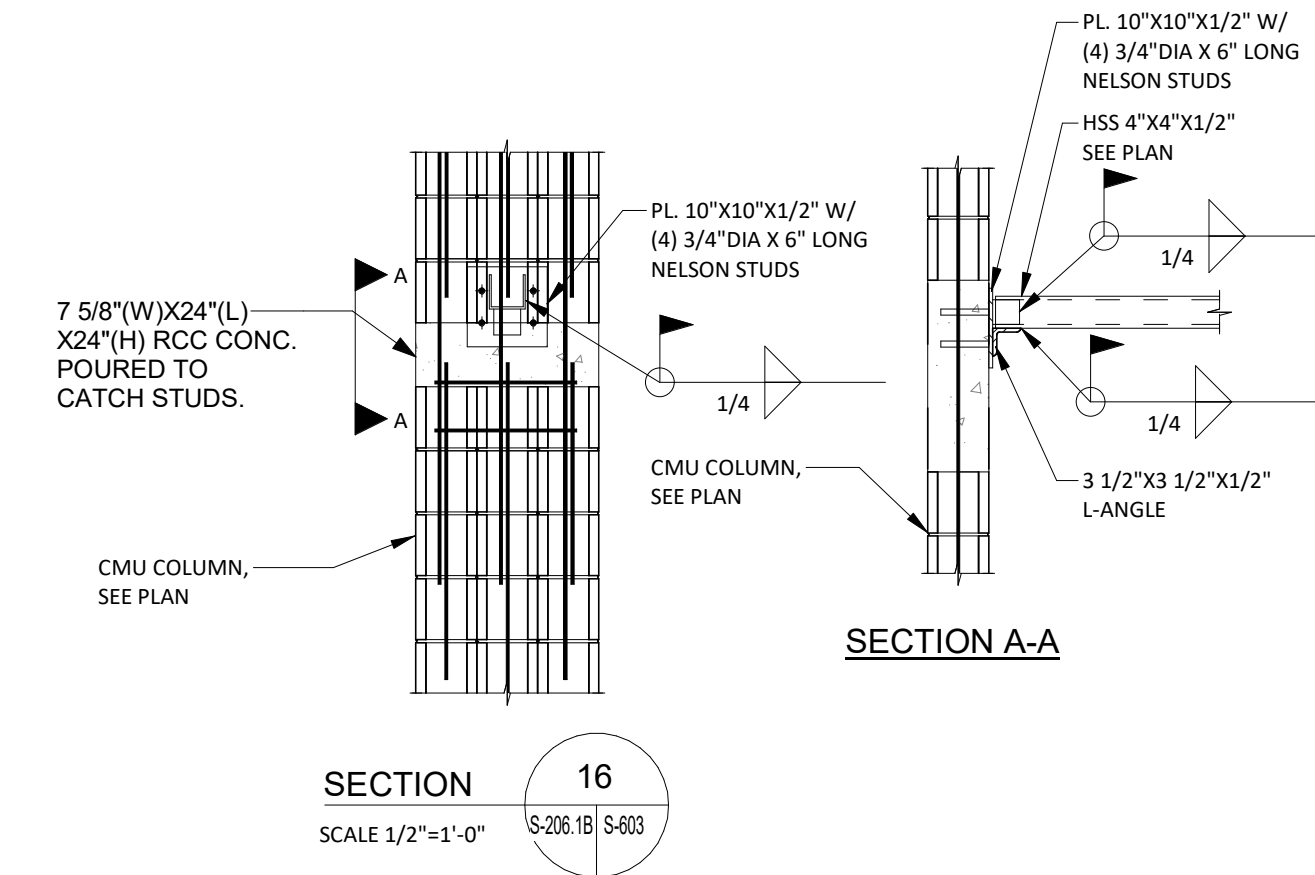


**TYPICAL TEE WALL INTERSECTION CORNER**  
SCALE 1/2"=1'-0"

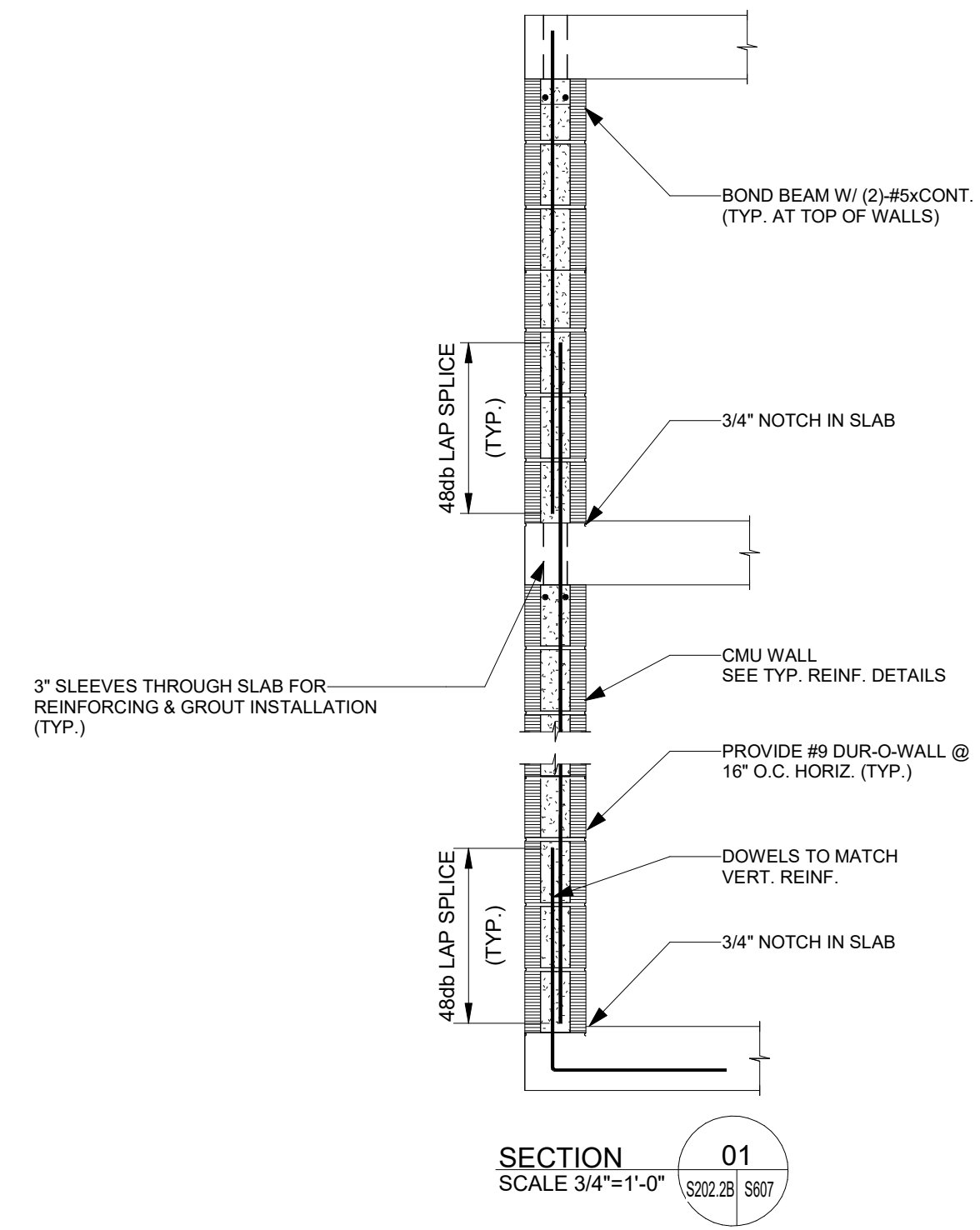


**TYPICAL CMU CONTROL JOINT DETAIL**  
SCALE 1/2"=1'-0"

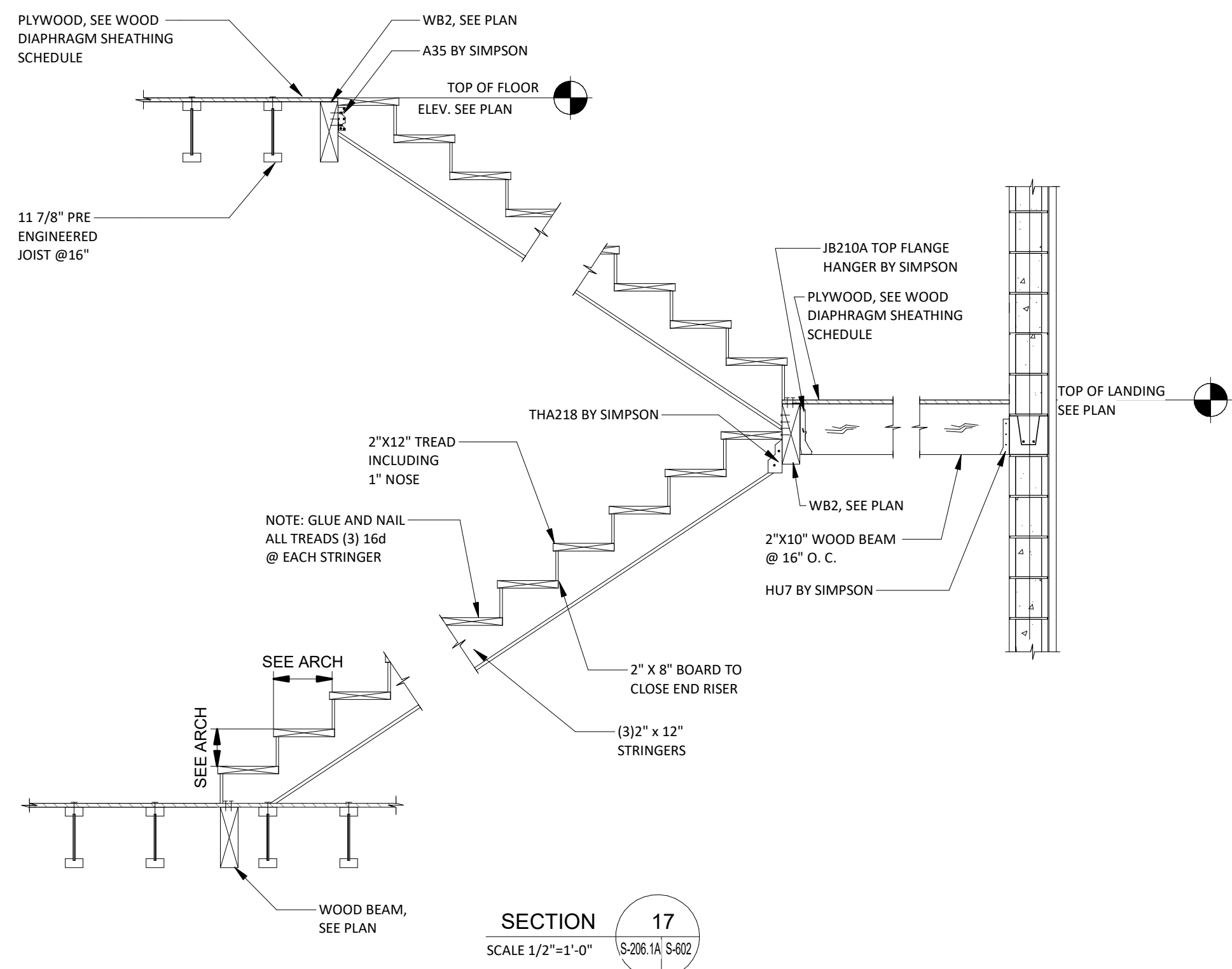
NOTE: PLATE CONTRACTION JOINT A OR B AT RETURNS AND JAMBS OF WALL OPENINGS, MAXIMUM SPACING 25 FT OR 2.5 TIMES WALL HEIGHT, USE ELASTOMERIC SEALANT GRADE NS, CLASS 25, USE M.



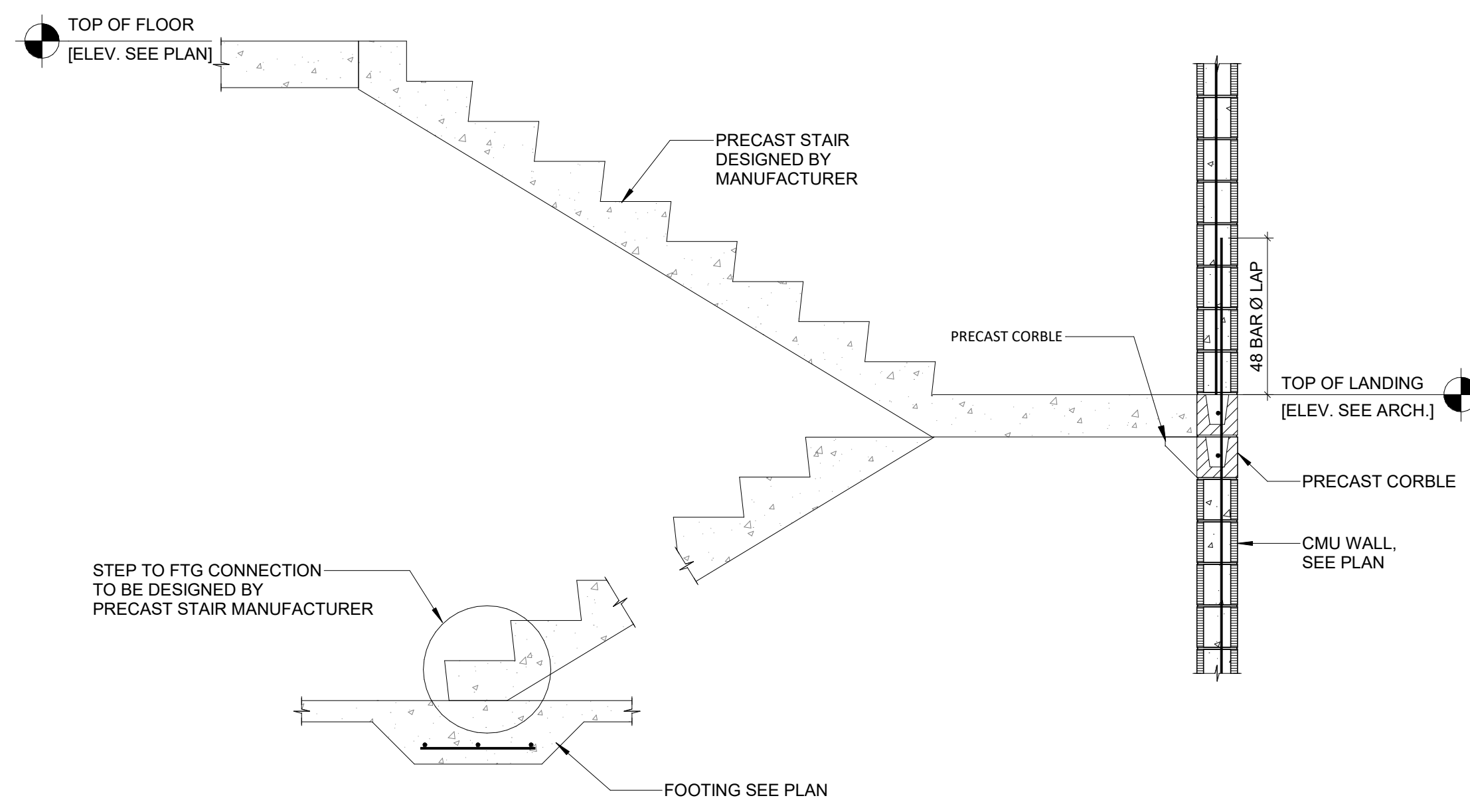
**SECTION 16**  
SCALE 1/2"=1'-0"



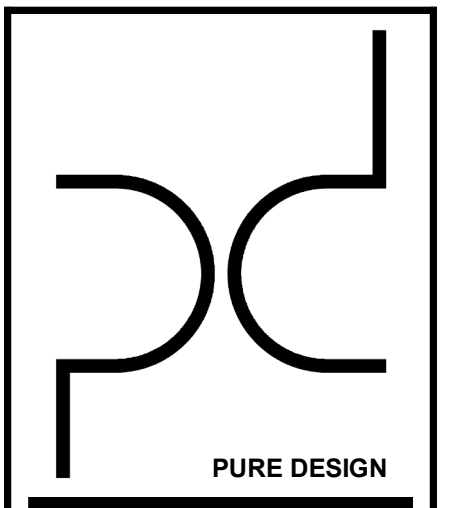
**SECTION 01**  
SCALE 3/4"=1'-0"



**SECTION 17**  
SCALE 1/2"=1'-0"



**TYPICAL STAIR TO FTG. DETAIL**  
SCALE 1/2"=1'-0"



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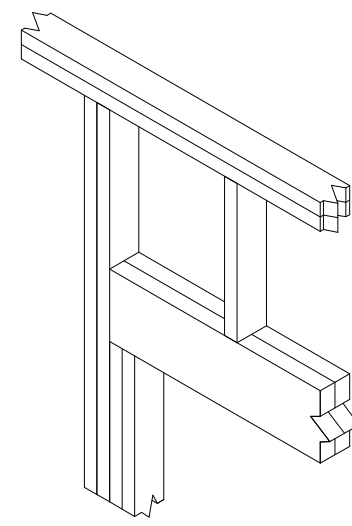
AC HOTELS

JOB NO: 18-007  
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TYPICAL MASONRY DETAILS  
**S-609**



HEADER/BEAM SCHEDULE (MARRIOTT & HAMPTON HOTELS)			
MARK	SIZE	BEARING STUDS	JAMB STUDS
1	(2) 2x8	(1) 2x6	(1) 2x6
2	(3) 2x8	(1) 2x6	(1) 2x6
3	(3) 2x10	(2) 2x6	(1) 2x6
4	(3) 1 3/4 x 9 1/2 2.0E -2600Fb LVL	(2) 2x6	(1) 2x6
5	(3) 2x10	(2) 2x6	-----
6	(4) 1 3/4 x 10 1/4 2.0E -2600Fb LVL	(3) 2x6	-----

**HEADER SCHEDULE | 01**

1/2" = 1'-0" S-610

SHEAR WALL SCHEDULE						
SHEAR WALL MARK	SHEATHING MATERIAL	PANEL EDGE BLOCKING	PANEL FASTENING	SILL PL. TO RIM CONN. & SLEEPER TO SUPPORT CONN.	SHEAR TRANSFER CLIPS	SILL PLATE ANCHORS
A	15/32" OSB PLYWOOD (1) SIDE	BLOCKED	8d NAILS @ 6" SPA. PERIMETER & 12" SPA. FIELD	(1) SIMPSON SDWH19400DB (OR EQ.) @ 16" SPA.	SIMPSON A35 CLIP @ 18" SPA.	1/2"Ø CONC. SCREW ANCHOR @ 32" SPA. -EMBED 3 1/2" MIN.
B	15/32" OSB PLYWOOD (1) SIDE	BLOCKED	8d NAILS @ 4" SPA. PERIMETER & 12" SPA. FIELD	(1) SIMPSON SDWH19400DB (OR EQ.) @ 12" SPA.	SIMPSON A35 CLIP @ 12" SPA.	1/2"Ø CONC. SCREW ANCHOR @ 32" SPA. -EMBED 3 1/2" MIN.
C	15/32" OSB PLYWOOD (1) SIDE	BLOCKED	8d NAILS @ 2" SPA. PERIMETER & 12" SPA. FIELD	(1) SIMPSON SDWH19400DB (OR EQ.) @ 6" SPA. (STAGGERED)	SIMPSON A35 CLIP @ 8" SPA.	1/2"Ø CONC. SCREW ANCHOR @ 24" SPA. -EMBED 3 1/2" MIN.

- NOTES:  
 1.) PANEL FASTENING AT ALL OTHER LOCATIONS SHALL BE 8d NAILS @ 6" SPA. PERIMETER & 12" SPA. FIELD  
 2.) RIM BOARD TO PLATE CONNECTION AT ALL OTHER LOCATIONS TO BE (1) SIMPSON SDWH19400DB (OR EQ.) @ 10" SPA.  
 3.) SHEAR TRANSFER CLIPS AT ALL OTHER LOCATIONS TO BE SIMPSON A35 CLIP @ 18" SPA.  
 4.) SILL PLATE AT ALL OTHER LOCATIONS TO BE 1/2"Ø CONC. SCREW ANCHOR @ 32" SPA. -EMBED 3 1/2" MIN.  
 5.) \* = SHEAR WALL TRANSFERS TO STEEL MOMENT FRAME ON GROUND FLOOR.  
 6.) † = SHEAR WALL IS OFFSET AT THE GROUND FLOOR.

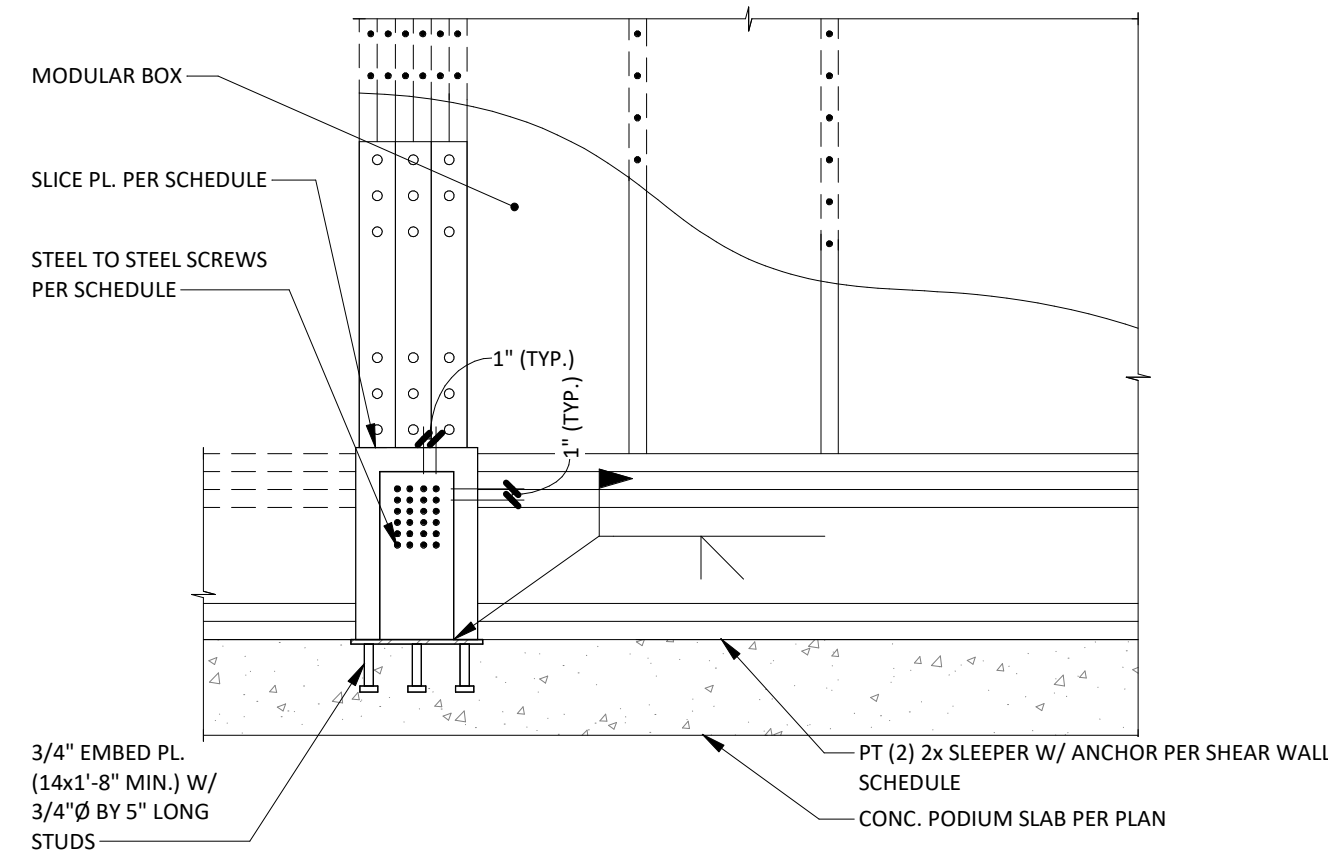
**SHEAR WALL SCHEDULE - AC MARRIOTT & HAMPTON INN | 02**

3/4" = 1'-0" S-610

HOLD DOWN SCHEDULE				
HOLD DOWN MARK	HOLD DOWN TYPE	ANCHOR ROD (36ksi)	MIN. POST/STUD PACK (REF. MANUF. FOR ADD'L REQ'TS)	REFERENCE DETAIL
A	(1) SIMPSON CMST14	N/A	(2) 2x4 or (2) 2x6	12/S3.1, 11/S4.1, 16/S6.7
B	(1) SIMPSON CMST12	N/A	(3) 2x6	12/S3.1, 11/S4.1, 16/S6.7
C	(1) SIMPSON CMST12	N/A	(4) 2x4	12/S3.1, 11/S4.1, 16/S6.7
D	(1) SIMPSON HDU5-SDS2.5	5/8"Ø A. ROD - EMBED 6" MIN.	(4) 2x4 or (2) 2x6	13/S-609
E	(1) SIMPSON HDU8-SDS2.5	7/8"Ø A. ROD - EMBED 6" MIN.	(4) 2x4 or (3) 2x6	13/S-609

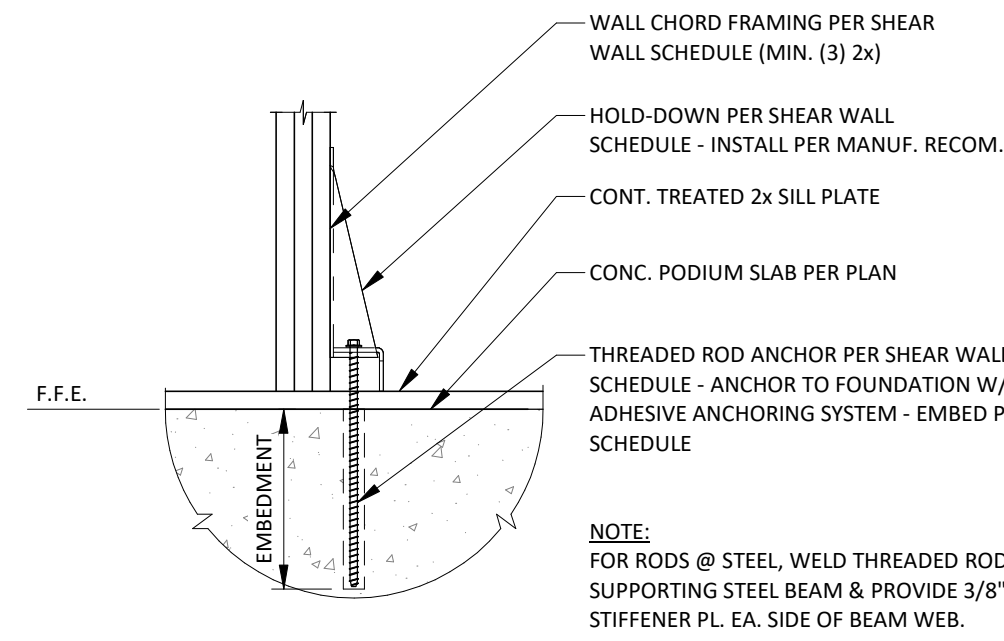
**HOLD DOWN SCHEDULE - AC MARRIOTT & HAMPTON INN | 03**

3/4" = 1'-0" S-610



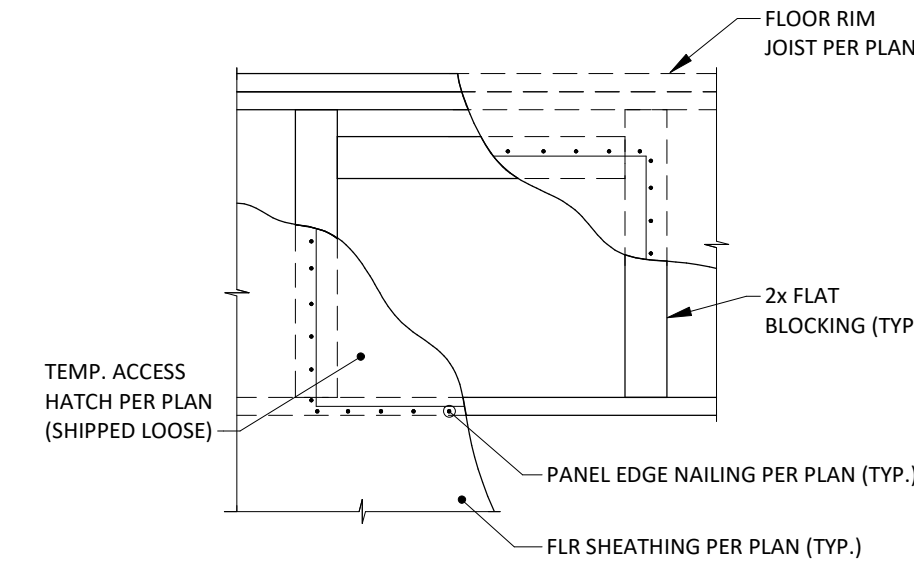
**TYP. HOLD-DOWN DETAIL @ MODULAR WALLS | 04**

3/4" = 1'-0" S-610



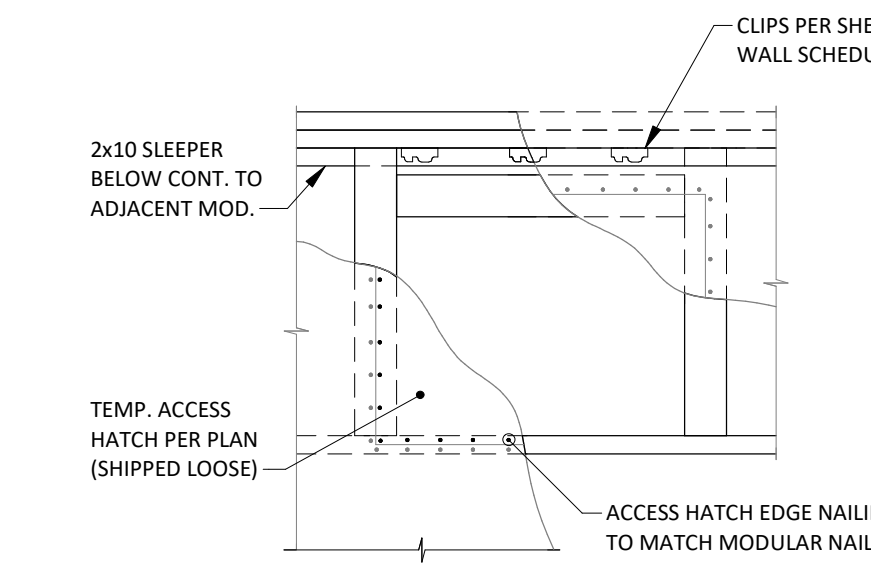
**TYP. HOLD-DOWN DETAIL @ SITE BUILT WALLS | 05**

3/4" = 1'-0" S-610



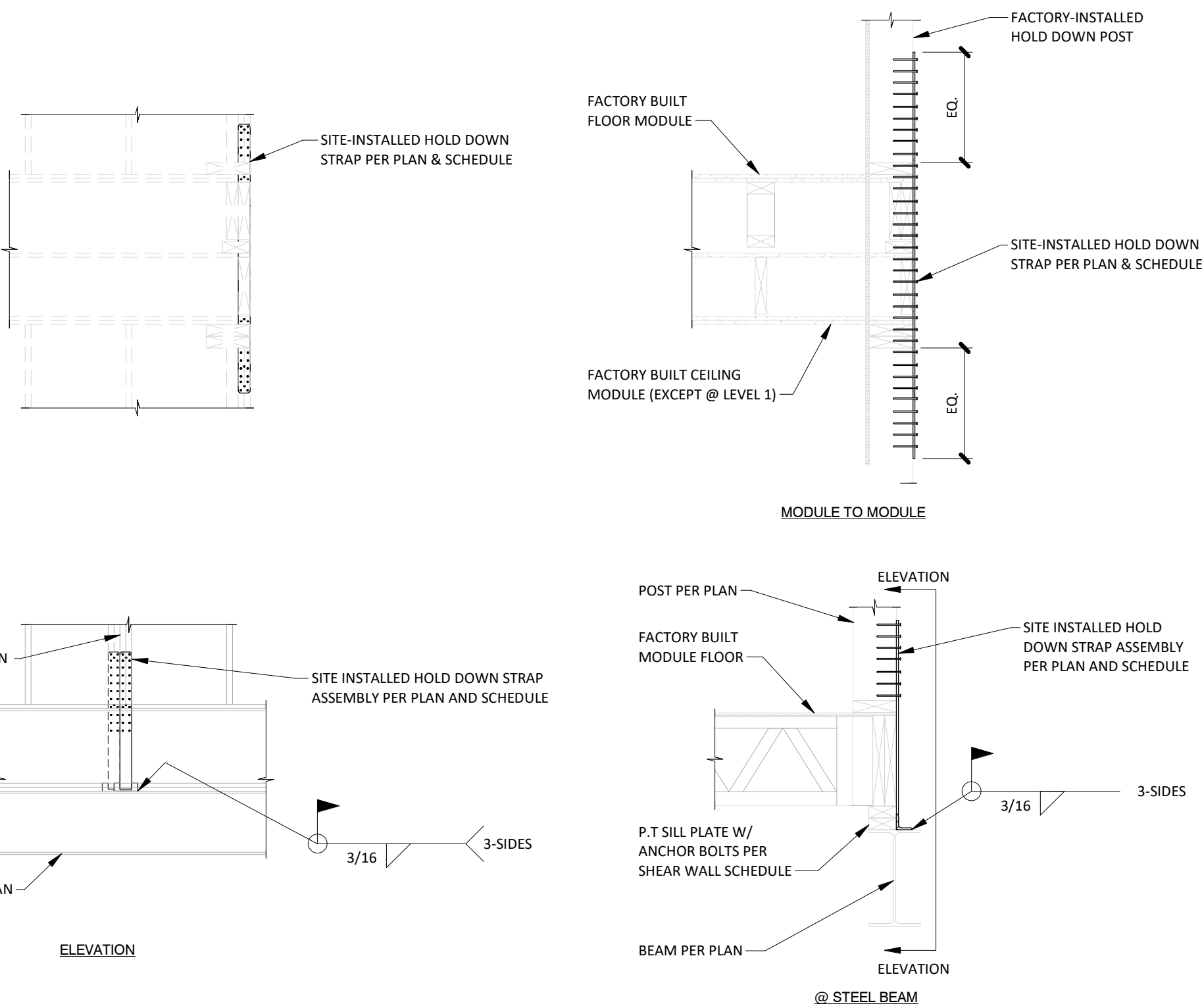
**TYP. SHEAR WALL ACCESS PANEL | 06**

3/4" = 1'-0" S-610



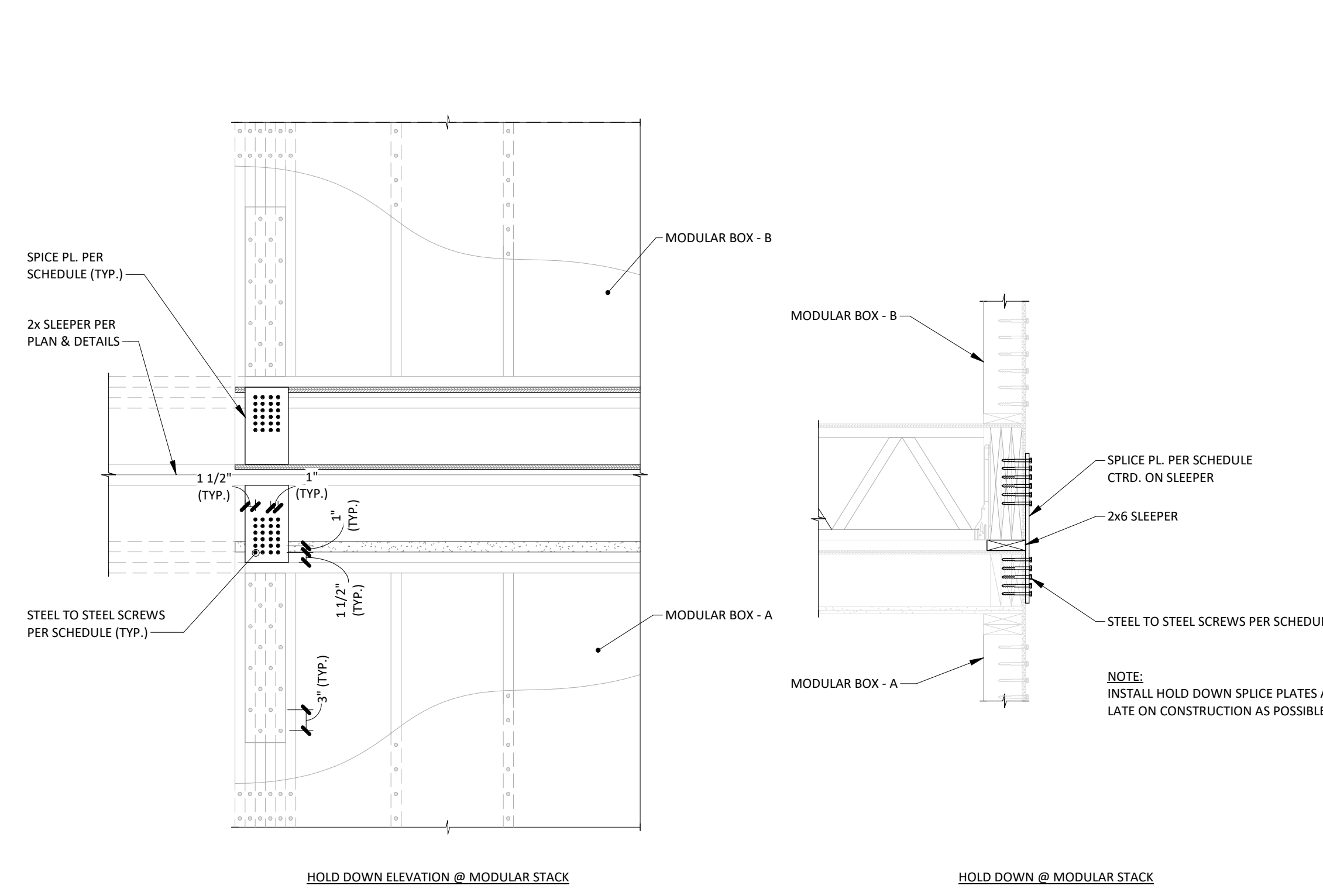
**TYP. SHEAR WALL ACCESS PANEL INSTALLATION | 07**

3/4" = 1'-0" S-610



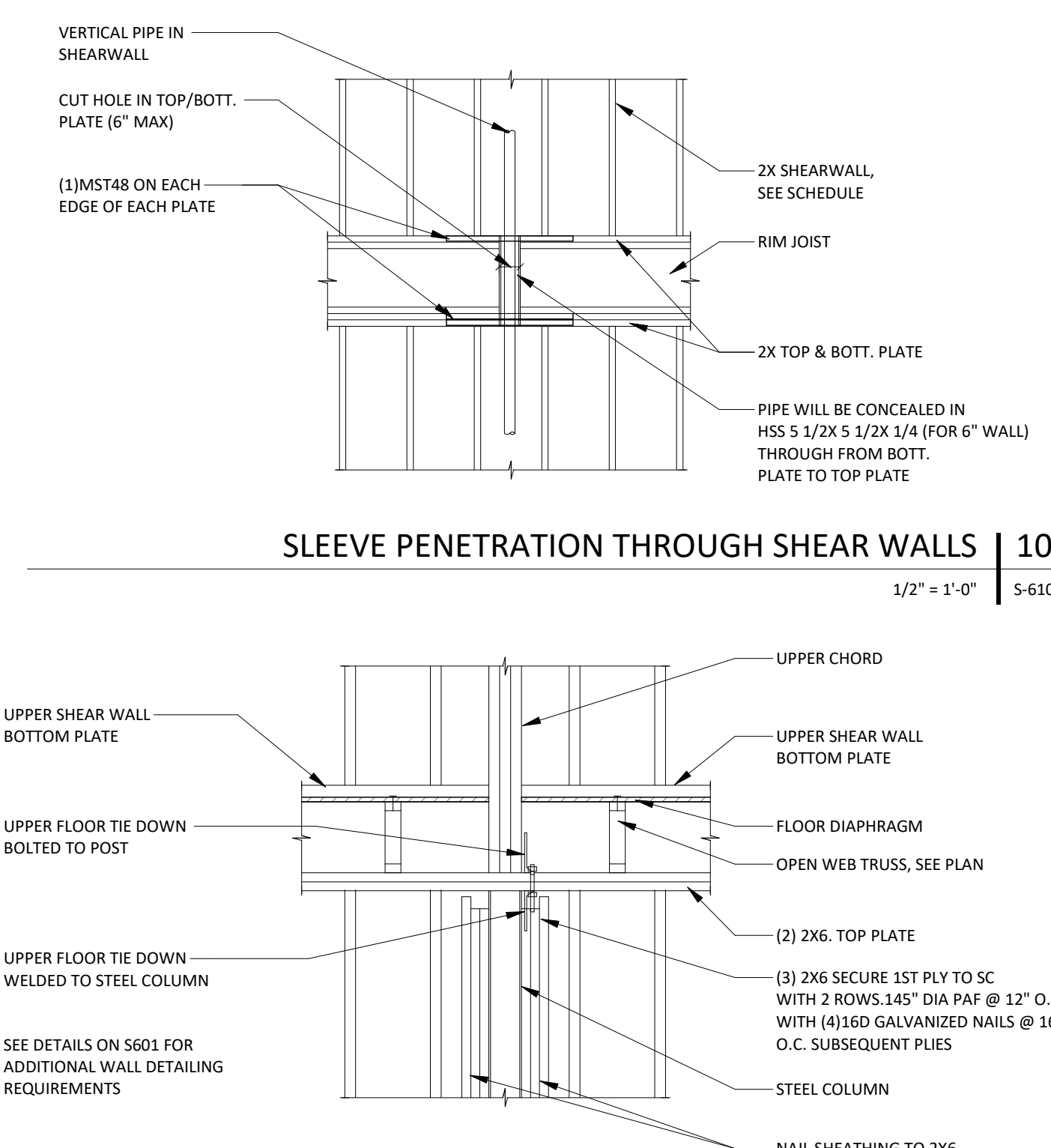
**INTERIOR HOLD DOWNS | 08**

3/4" = 1'-0" S-610



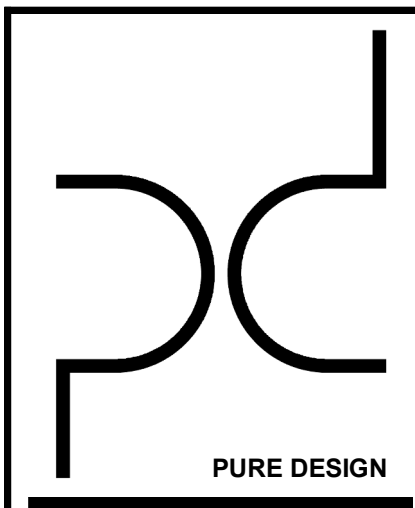
**EXTERIOR HOLD DOWNS | 09**

3/4" = 1'-0" S-610



**TIMBER CHORD TO STEEL CHORD CONNECTION | 11**

1/2" = 1'-0" S-610



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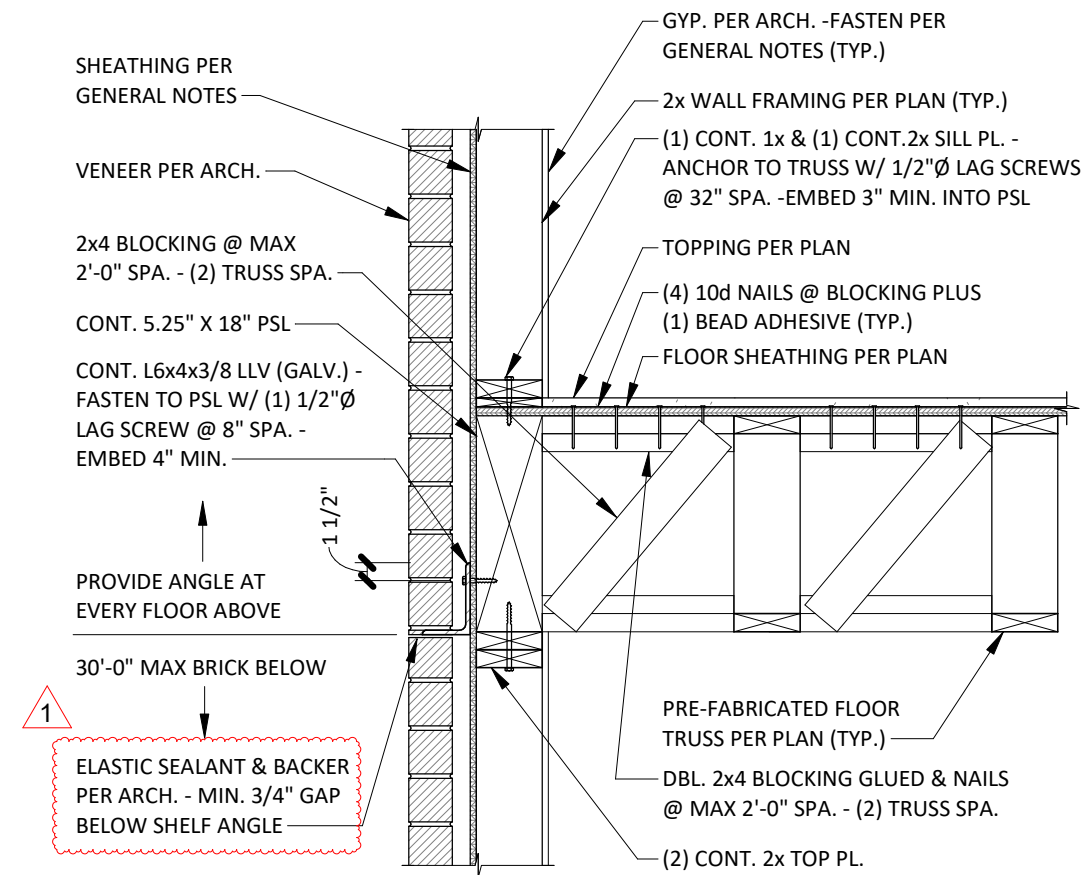


JOB NO: 18-007  
 DATE: 2018.03.07  
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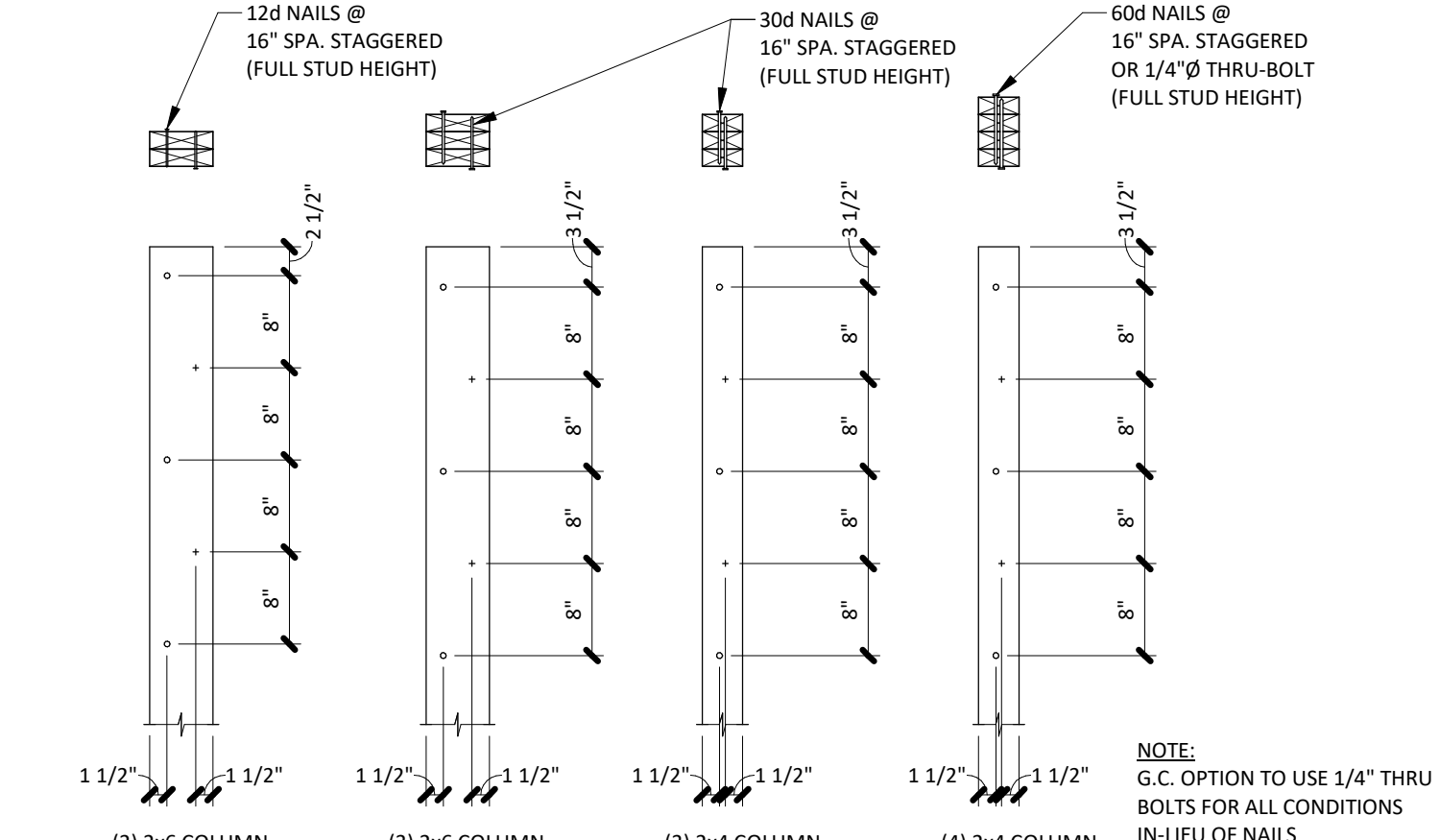
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 Lenexa, Kansas 66214  
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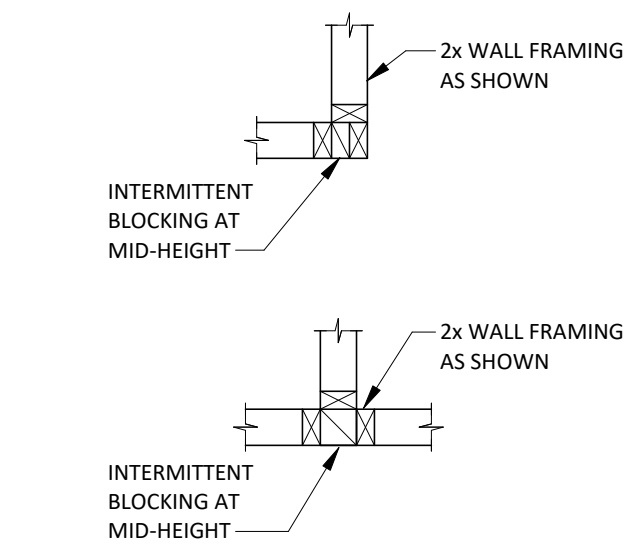
TYPICAL WOOD FRAMING DETAILS  
**S-610**



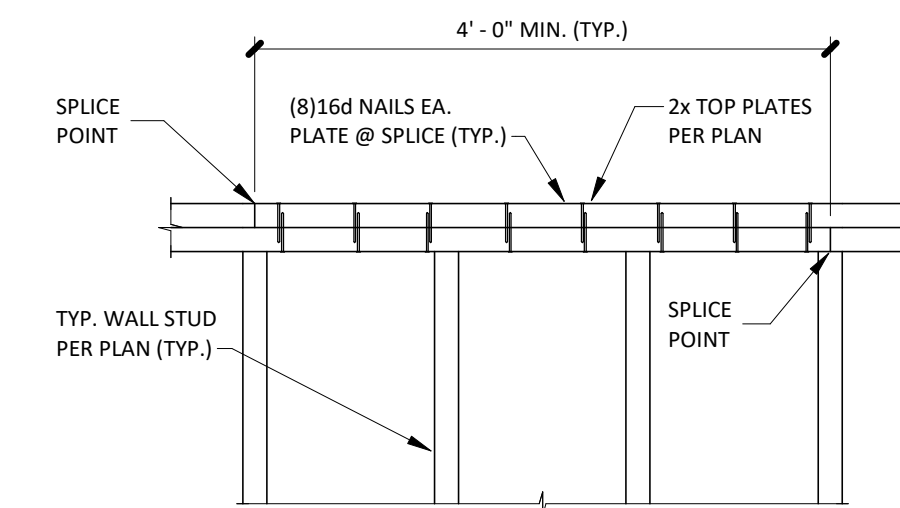
01 BRICK SUPPORT WHEN HEIGHT > 30'  
3/4" = 1'-0"



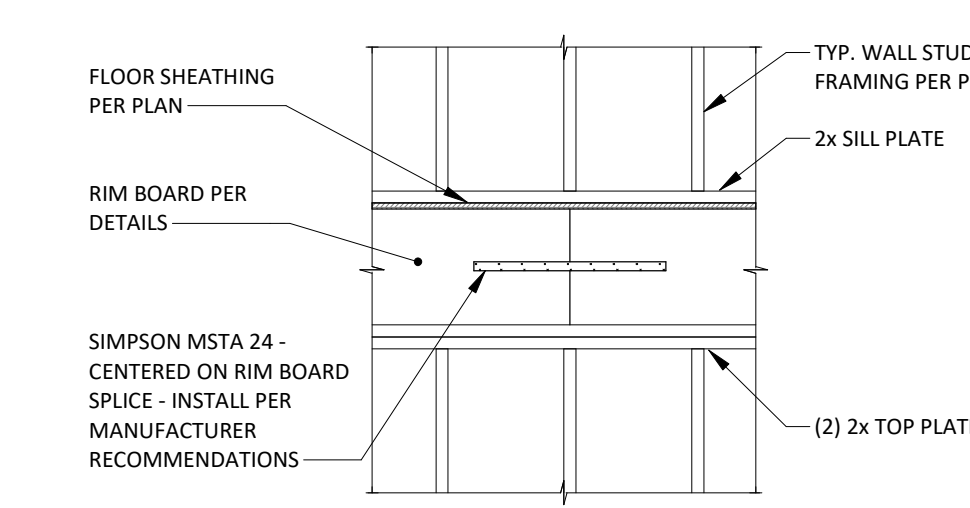
02 TYP. BUILT UP COLUMN DETAIL  
3/4" = 1'-0"



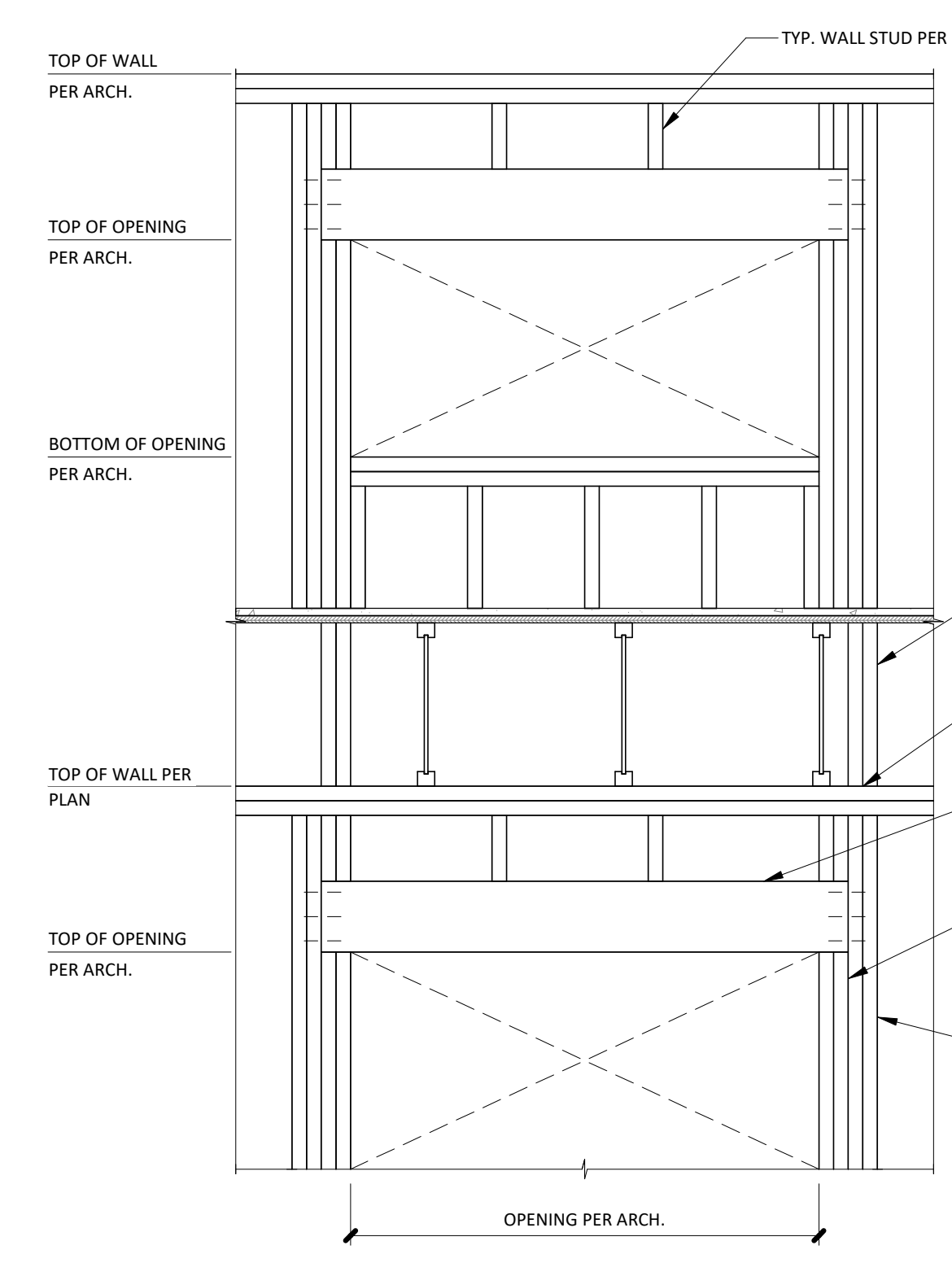
03 TYP. STUD WALL INTERSECT DETAIL  
3/4" = 1'-0"



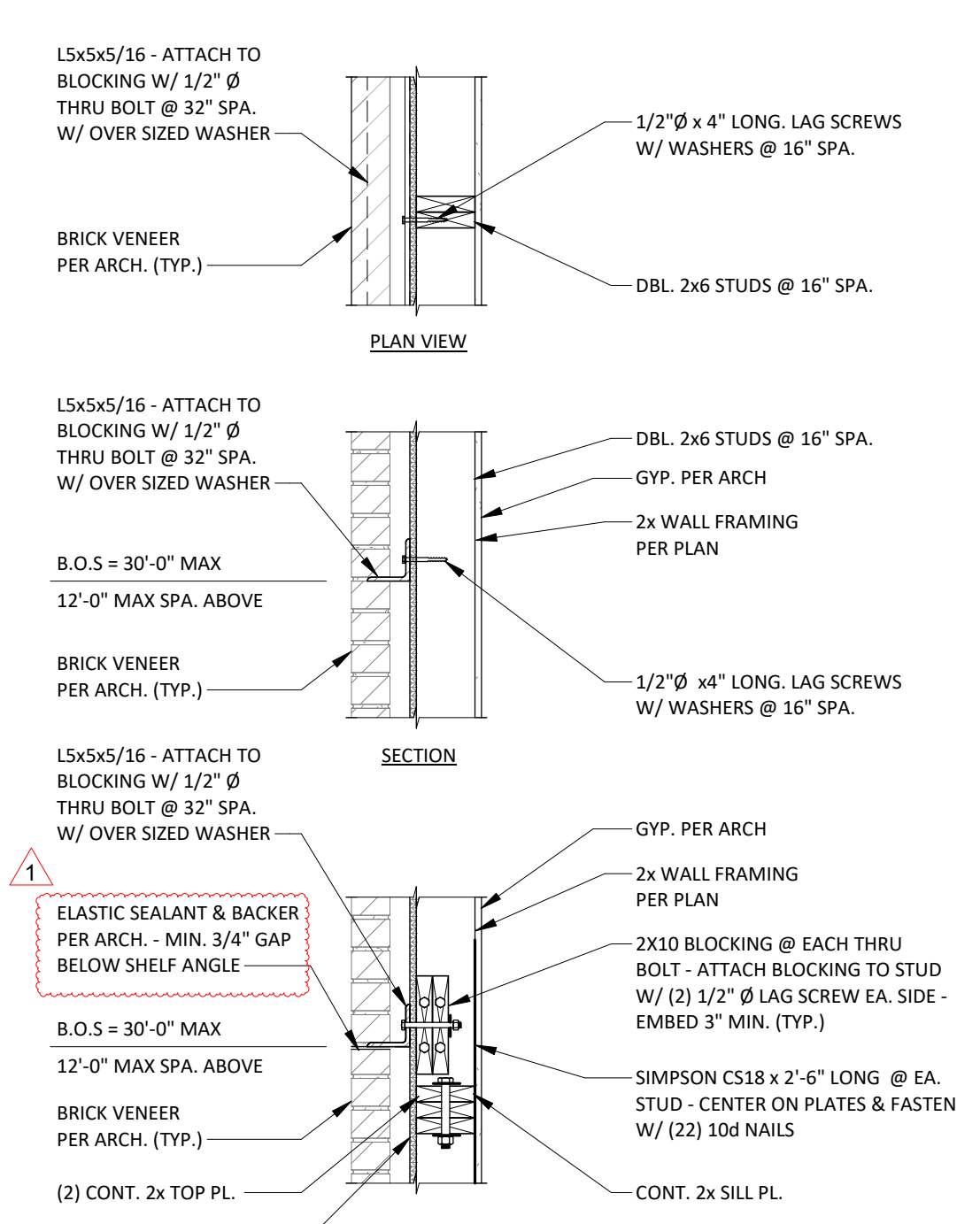
04 TYP. TOP CHORD SPLICE DETAIL  
3/4" = 1'-0"



05 TYP. RIM BOARD SPLICE  
1/2" = 1'-0"



06 TYP. HEADER/WALL OPENING FLOOR-FLOOR DETAIL  
3/4" = 1'-0"

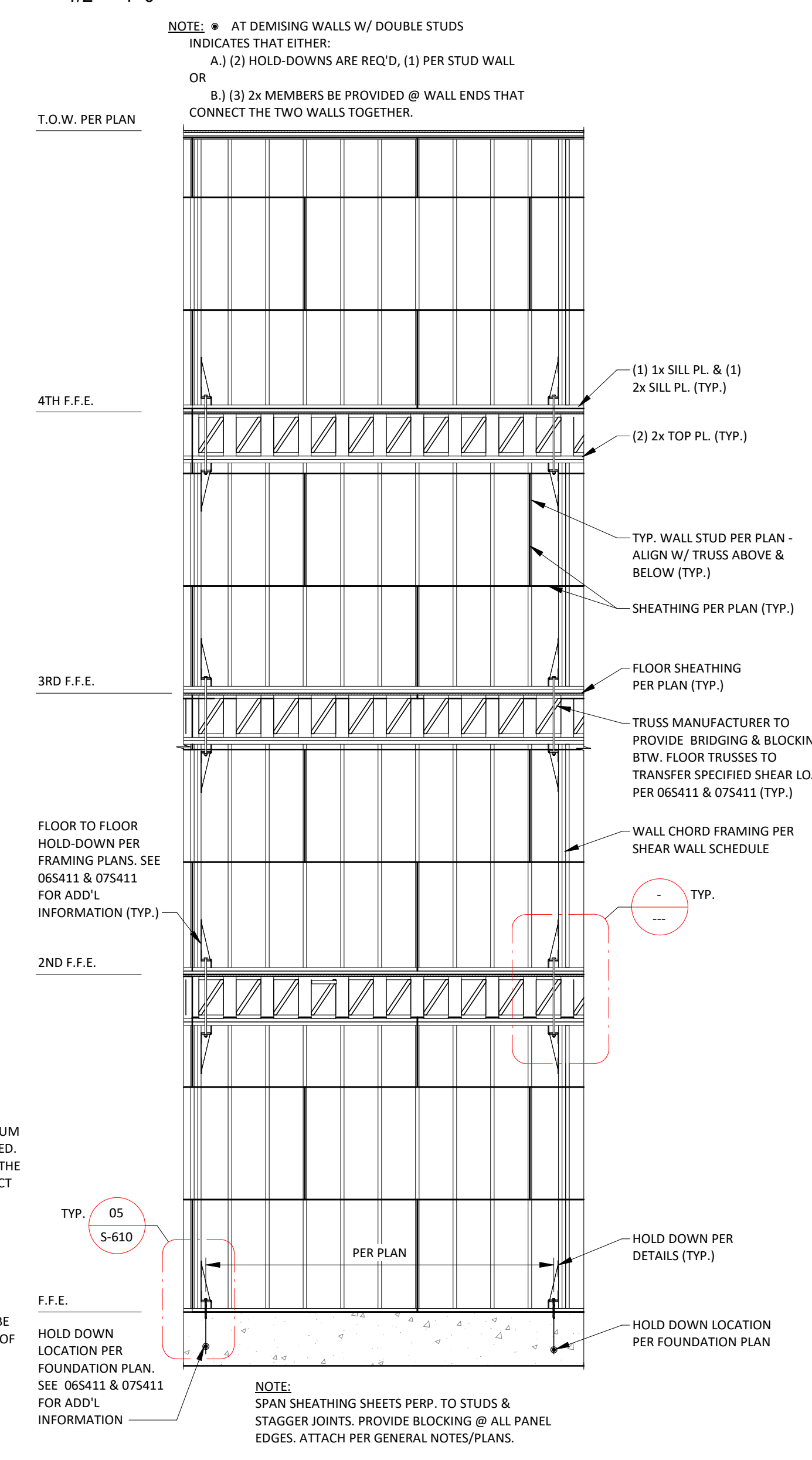


08 TYPICAL LEDGER ANGLE CONN. @ STAIR DETAIL  
3/4" = 1'-0"

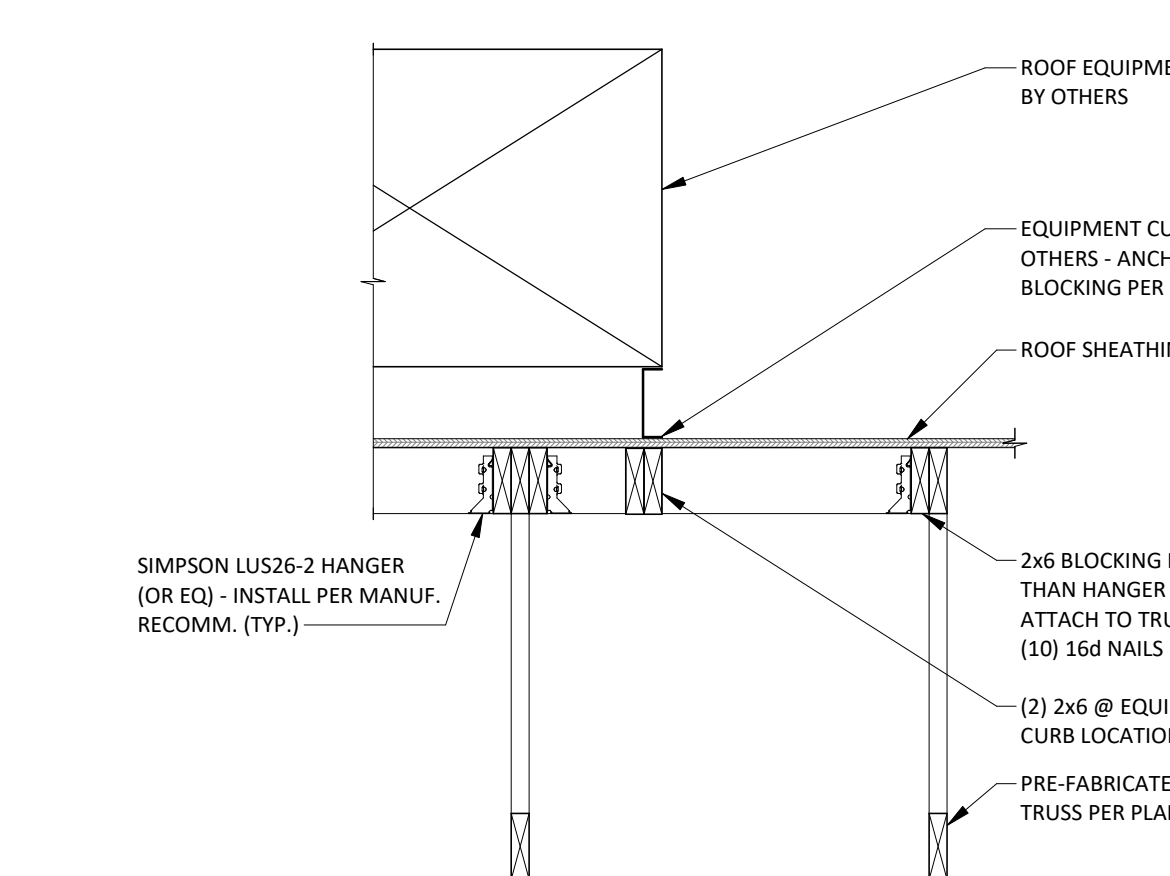
FASTENING SCHEDULE PER IBC 2015, TABLE 2304.10.1		
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
BRIDGING OR BLOCKING BETWEEN DIMENSIONAL JOISTS OR RAFTERS	2-8d COMMON (2 1/2" x 0.131"); OR 2-10d BOX (3" x 0.128"); OR 2-3" x 0.131" NAILS	EACH END, TOENAIL
FLAT BLOCKING TO TRUSS AND WEB FILLER	2-16d COMMON (3 1/2" x 0.162"); OR 3-3" x 0.131" NAILS	END NAIL
RAFTER OR ROOF TRUSS TO TOP PLATE (SEE SECTION IBC 2015, 2308.7.5, TABLE 2308.7.5)	3-10d COMMON (3" x 0.148"); OR 3-16d BOX (3 1/2" x 0.135"); OR 4-10d BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS	TOENAIL
BRIDGING, BLOCKING, & MEMBER - TO - MEMBER FOR PRE-FABRICATED JOIST, RAFTER OR TRUSS	PER FABRICATOR	PER FABRICATOR
STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS	16d COMMON (3 1/2" x 0.162"); OR 16d BOX (3 1/2" x 0.135"); OR 3" x 0.131" NAILS	16" O.C. FACE NAIL 12" O.C. FACE NAIL
BUILT-UP HEADER (2" TO 2" HEADER)	16d COMMON (3 1/2" x 0.162"); OR 16d BOX (3 1/2" x 0.135")	16" O.C. EA. EDGE, FACE NAIL 12" O.C. EA. EDGE, FACE NAIL
BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS	20d COMMON (4" x 0.192") AND: 2-20d COMMON (4" x 0.192"); OR 3-10d BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	32" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES 24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
CONTINUOUS HEADER TO STUD	4-8d COMMON (2 1/2" x 0.131"); OR 4-10d BOX (3" x 0.128")	TOENAIL
HEADER TO KING STUD	4-8d COMMON (2 1/2" x 0.131"); OR 4-10d BOX (3" x 0.128")	TOENAIL
BOTTOM PLATE TO JOIST, RIM JOISTS, BAND JOIST OR BLOCKING (NOT AT SHEAR WALL PANELS)	16d COMMON (3 1/2" x 0.162"); OR 16d BOX (3 1/2" x 0.135"); OR 3" x 0.131" NAILS	16" O.C. FACE NAIL 12" O.C. FACE NAIL
STUD TO TOP OR BOTTOM PLATE	2-16d COMMON (3 1/2" x 0.162"); OR 3-10d BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	END NAIL
TOP PLATE TO TOP PLATE	16d COMMON (3 1/2" x 0.162") 10d BOX (3" x 0.128"); OR 3" x 0.131" NAILS	16" O.C. FACE NAIL 12" O.C. FACE NAIL
TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	2-16d COMMON (3 1/2" x 0.162"); OR 3-10d BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	FACE NAIL
TOP PLATE SPLICE	PER TYP. DETAILS	FACE NAIL
RIM JOISTS, BAND JOISTS, OR BLOCKING TO TOP PLATE, SILL OR OTHER FRAMING BELOW	8d COMMON (2 1/2" x 0.131"); OR 10d BOX (3" x 0.128"); OR 3" x 0.131" NAILS	6" O.C., TOENAIL
LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	3-16" COMMON (3 1/2" x 0.162"); OR 4-10d BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS	EACH STUD, FACE NAIL

10 TYP. FASTENING SCHEDULE  
3/4" = 1'-0"

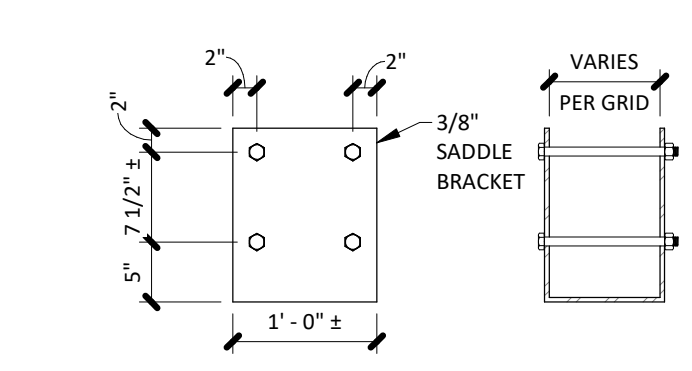
NOTES:  
1.) CONNECTIONS SHOWN ARE THE MINIMUM REQUIREMENTS FOR THE CONDITIONS LISTED. THESE CONNECTIONS DO NOT SUPERCEDE THE CONNECTIONS SHOWN WITHIN THE PROJECT DETAILS.  
2.) CONTACT THE ENGINEER OF RECORD REGARDING CONNECTIONS NOT SHOWN WITHIN THIS SCHEDULE OR THE PROJECT DETAILS.  
3.) ALL ALTERNATE CONNECTIONS ARE TO BE REVIEWED & APPROVED BY THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION.  
4.) CONNECTIONS SHOWN ARE THE MOST COMMON. REFER TO IBC 2015, TABLE 2304.10.1 FOR ADDITIONAL CONNECTION INFORMATION.



11 TYP. SHEAR WALL ELEVATION FLOOR-FLOOR W/ FLOOR TRUSS  
1/4" = 1'-0"



07 ROOF EQUIPMENT SUPPORT DETAIL  
3/4" = 1'-0"



09 SADDLE BRACKET DETAIL  
3/4" = 1'-0"

PURE DESIGN

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DUAL HOTEL DURANGO  
- AC & HIS

JOB NO: 18-007  
DATE: 2018.03.07  
ISSUE RECORD:  
REVISIONS:  
1 05/17/2019

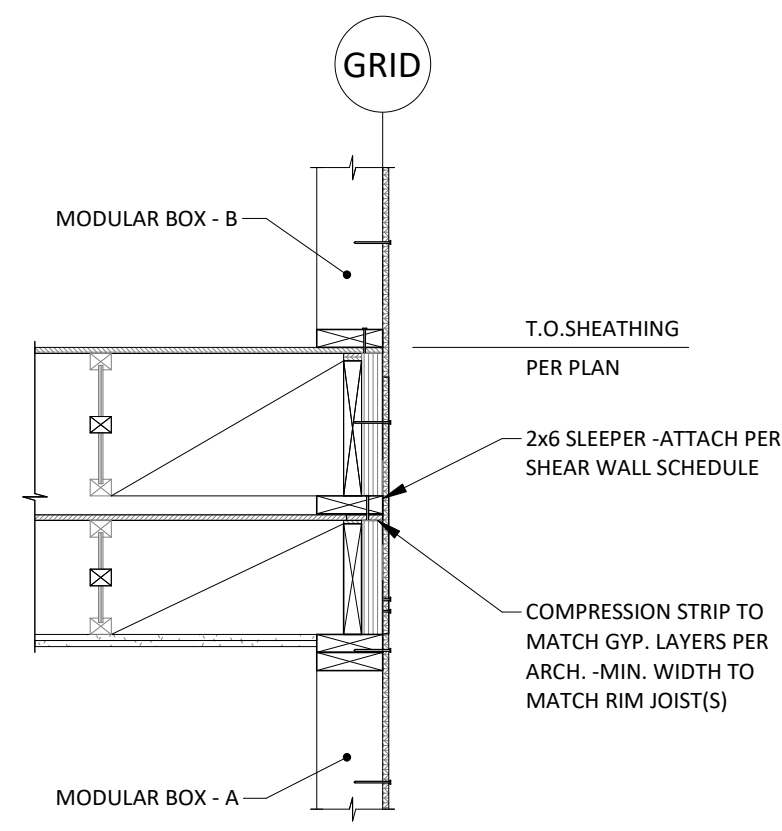
2019.06.14

AC HOTELS

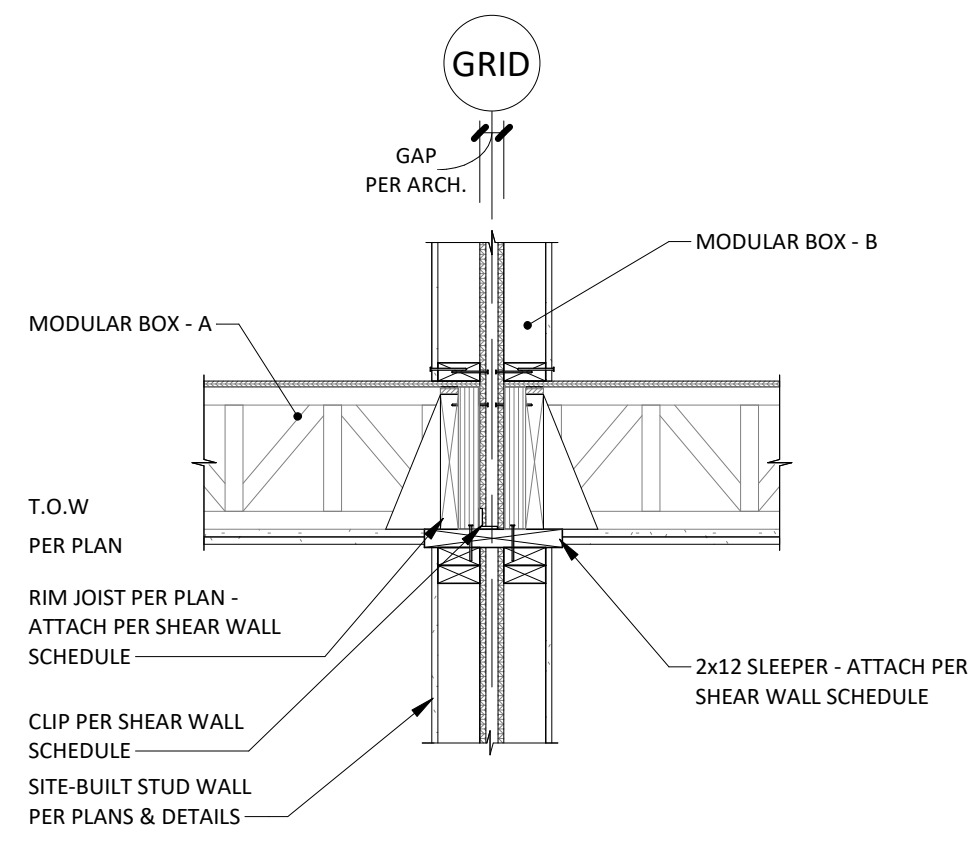
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TYPICAL WOOD FRAMING DETAILS

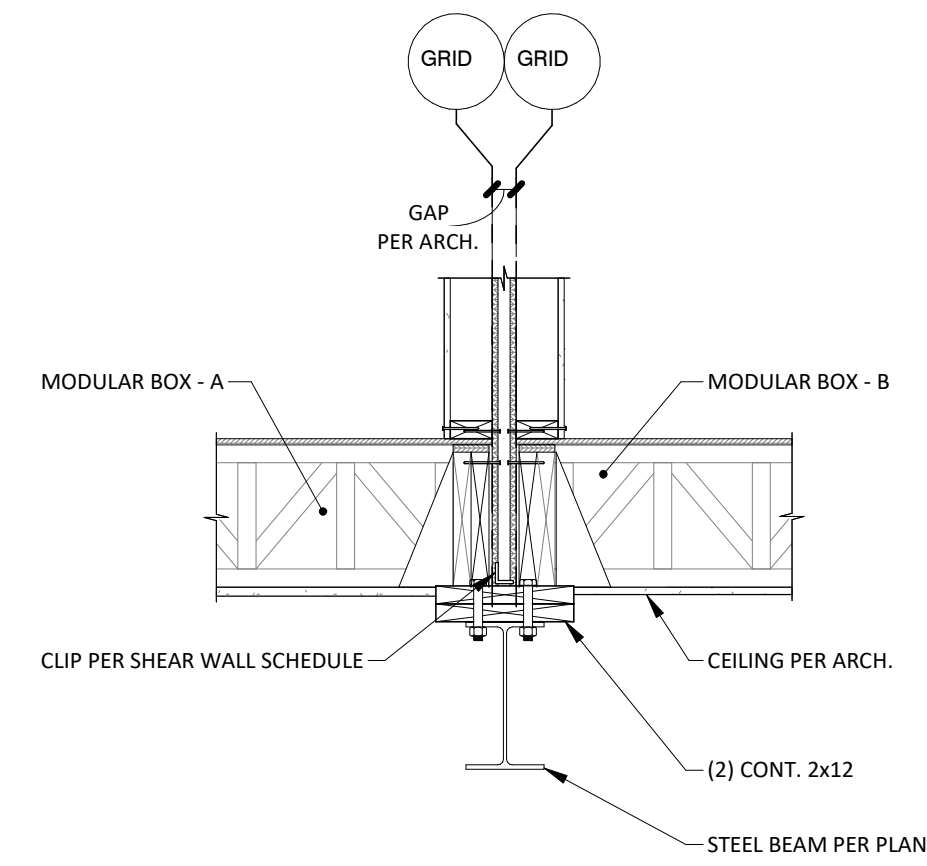
S-611



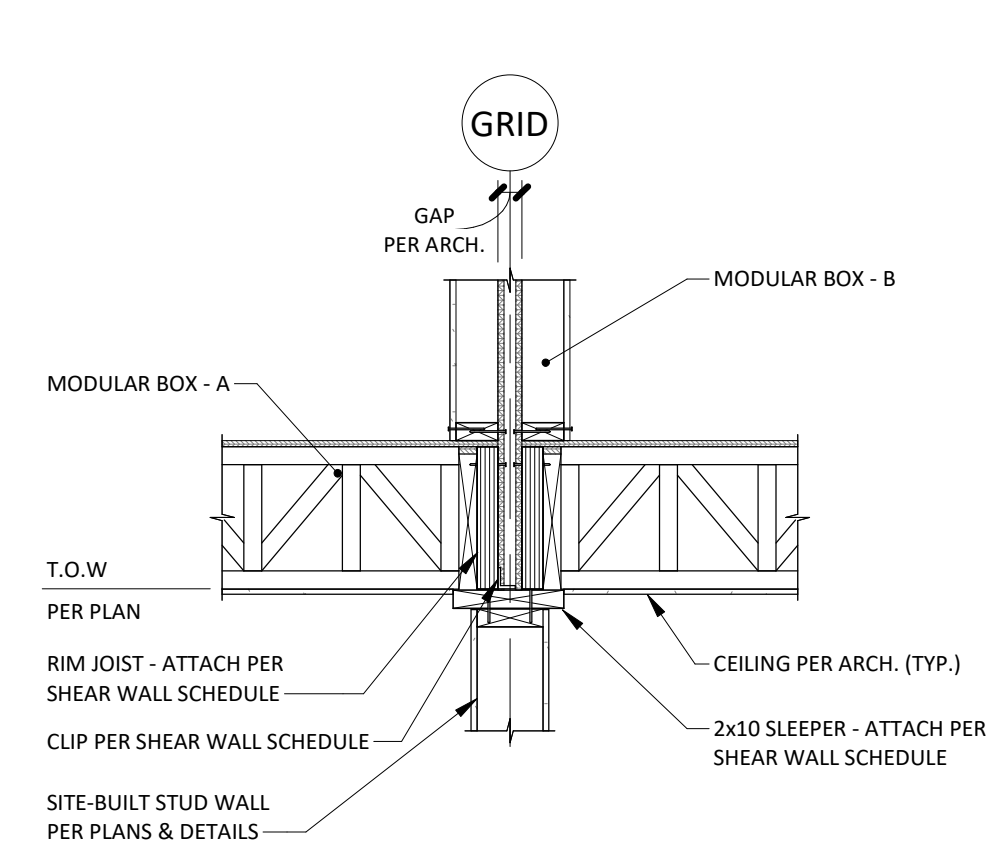
1 TYP. CORRIDOR @ MODULAR STACK  
3/4" = 1'-0"



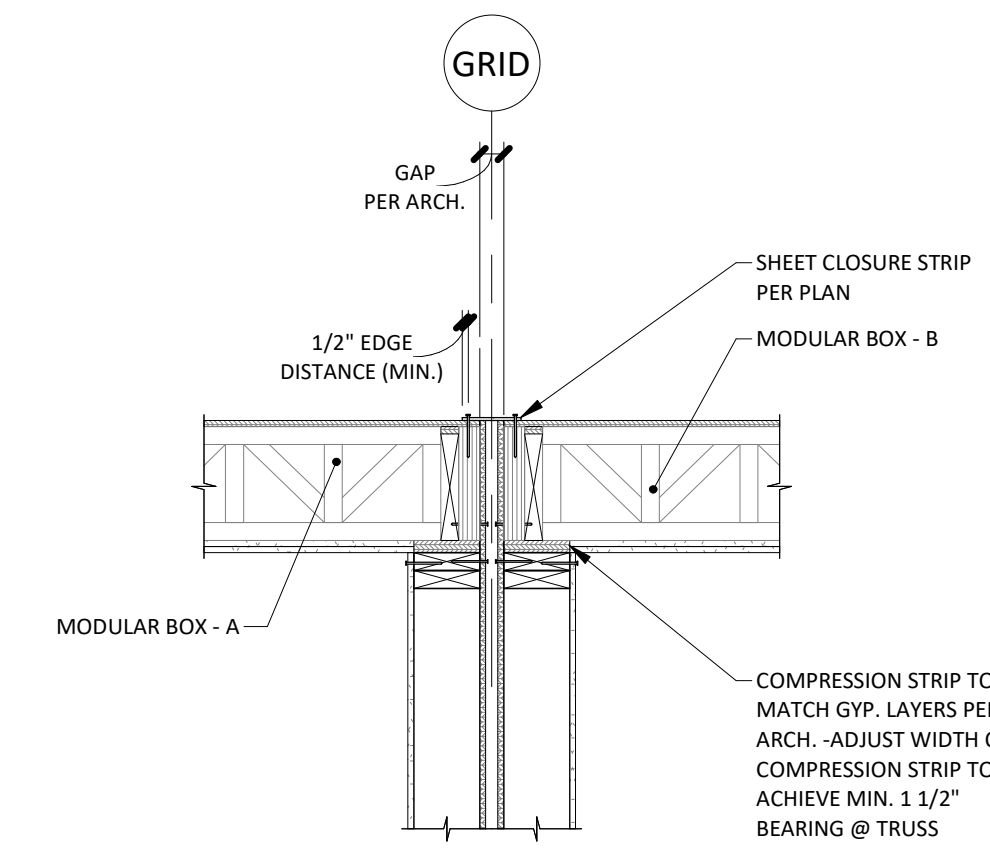
2 TYP. MODLINE @ SITE-BUILT DOUBLE WALL  
3/4" = 1'-0"



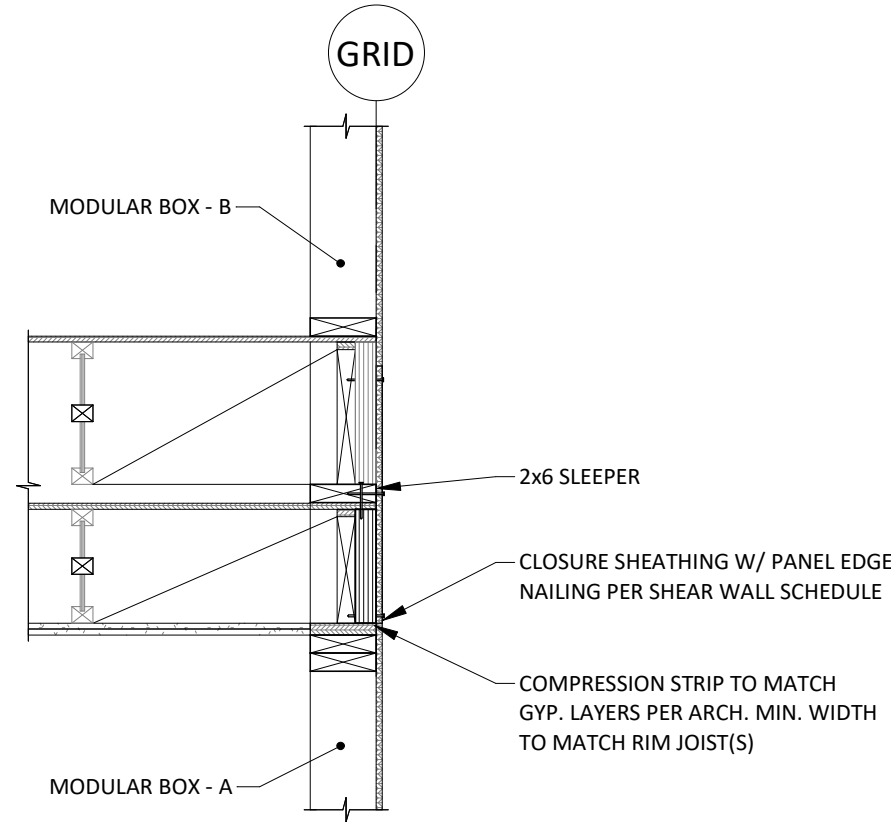
3 TYP. INTERIOR @ STEEL BEAM  
3/4" = 1'-0"



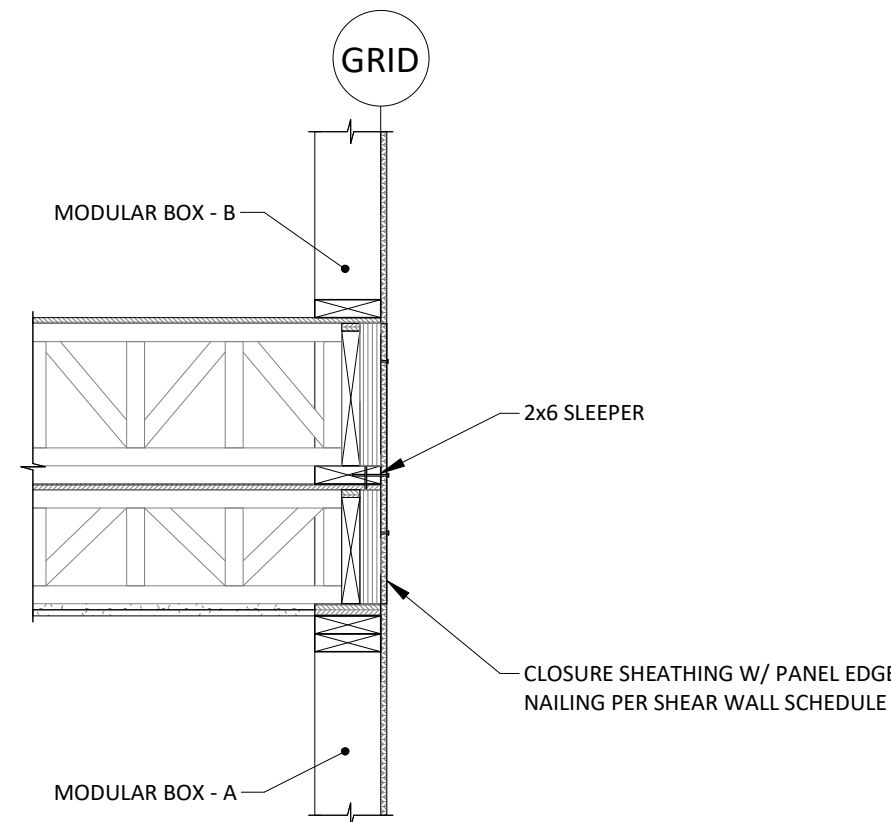
4 TYP. MODLINE @ SITE-BUILT WALL  
3/4" = 1'-0"



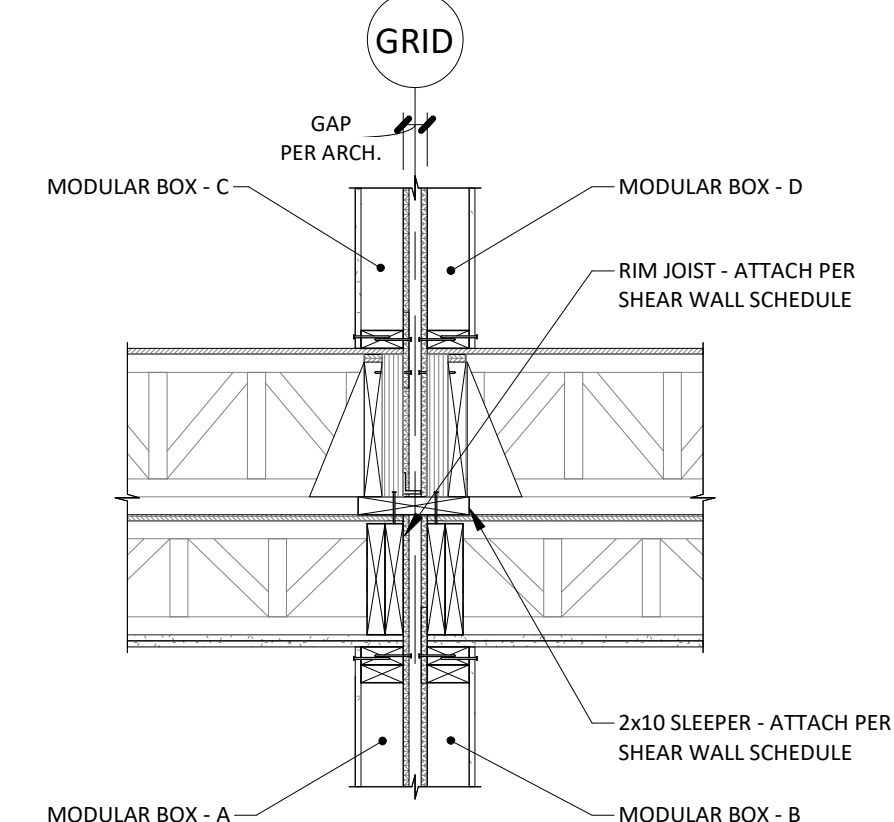
5 TYP. MODLINE @ MODULAR ROOF  
3/4" = 1'-0"



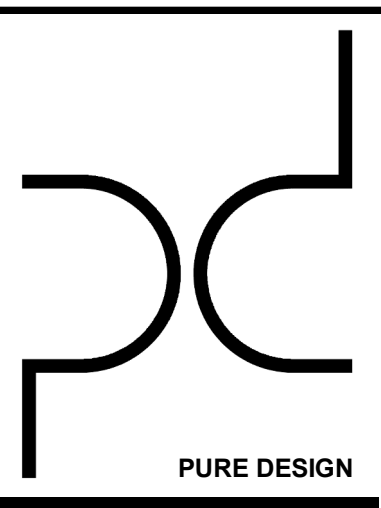
6 TYP. EXTERIOR @ MODULAR STACK - PARALLEL  
3/4" = 1'-0"



7 TYP. EXTERIOR @ MODULAR STACK - PERPENDICULAR  
3/4" = 1'-0"



8 TYP. MODLINE @ MODULAR STACK  
3/4" = 1'-0"



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AC HOTELS

JOB NO: 18-007  
DATE: 2018.03.07

ISSUE RECORD:

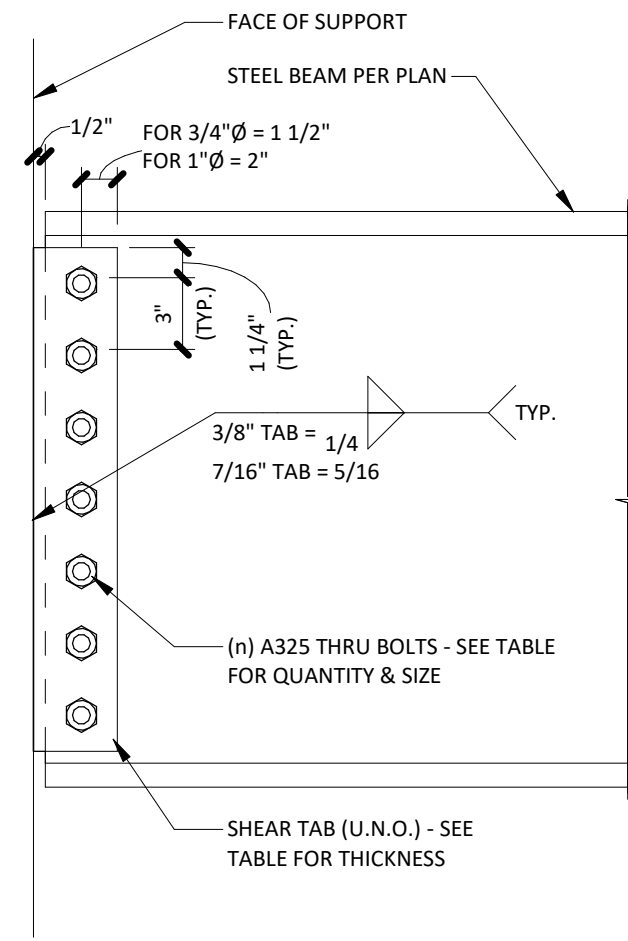
REVISIONS:

NEW 2019.06.14

TYPICAL MODULAR DETAILS

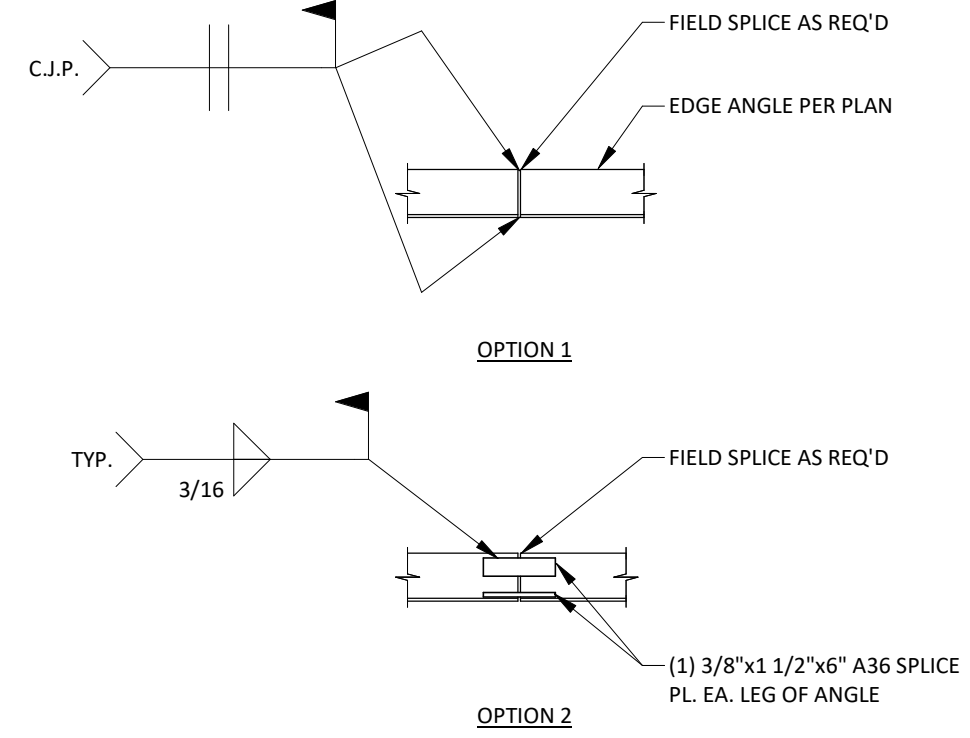
S-612

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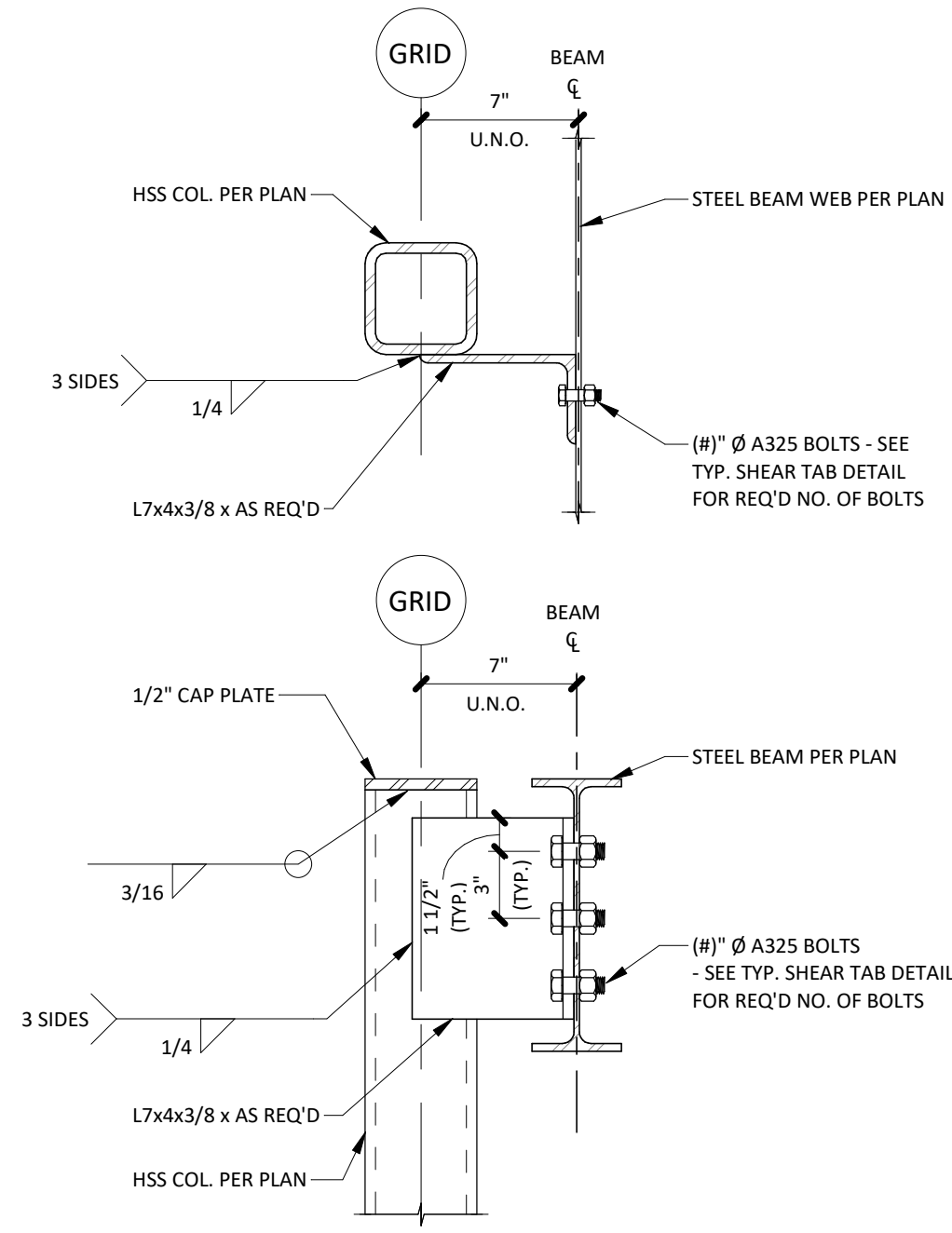


BEAM DEPTH	# OF (n) A325 BOLTS	BOLT Ø	SHEAR TAB THICKNESS
12"	3	3/4"	3/8"
14"	3	3/4"	3/8"
16"	4	3/4"	3/8"
18"	5	3/4"	3/8"
21"	6	1"	7/16"
24"	7	1"	7/16"
27"	8	1"	7/16"
30"	9	1"	7/16"
34"	10	1"	7/16"
36"	11	1"	7/16"

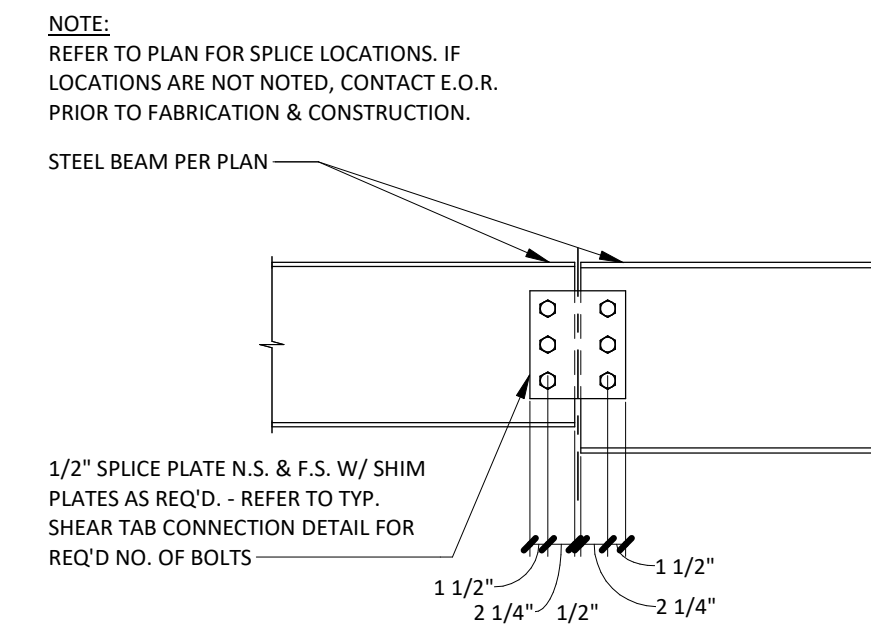
TYP. SHEAR TAB CONNECTION DETAIL | 01  
1 1/2" = 1'-0" | S-613



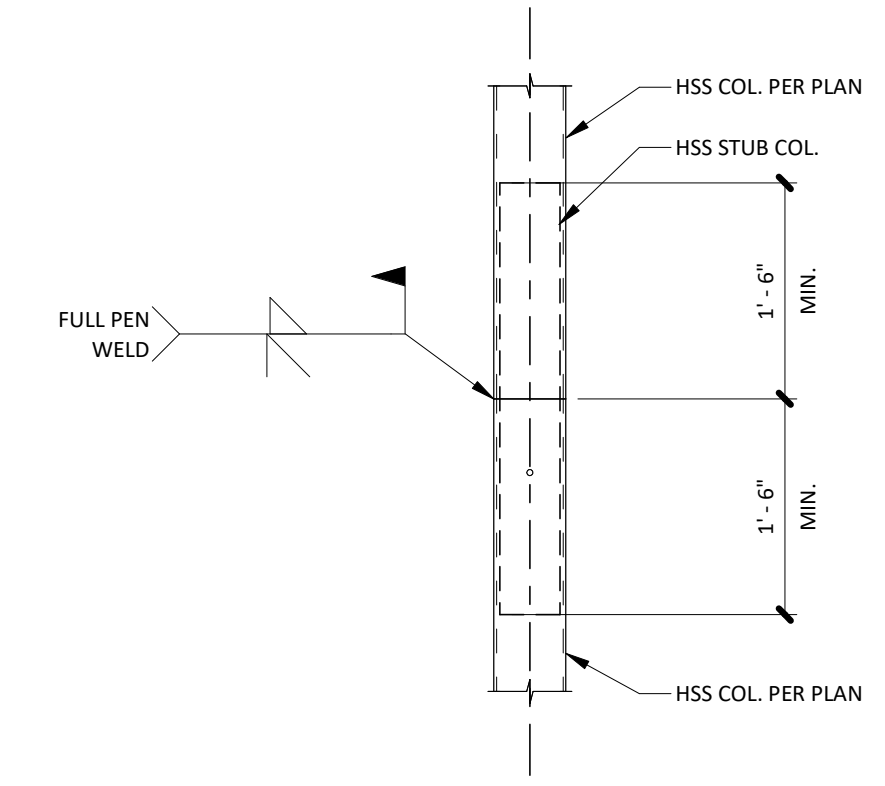
EDGE ANGLE SPlice DETAIL | 02  
3/4" = 1'-0" | S-613



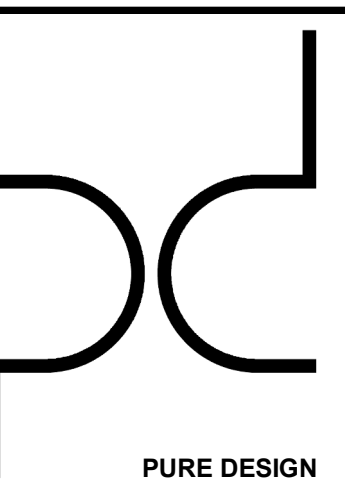
BEAM TO COL. - ECCENTRIC CONN. DETAIL | 03  
1 1/2" = 1'-0" | S-613



TYP. BEAM END SPlice CONNECTION | 04  
3/4" = 1'-0" | S-613



TYP. HSS COLUMN SPlice | 05  
3/4" = 1'-0" | S-613



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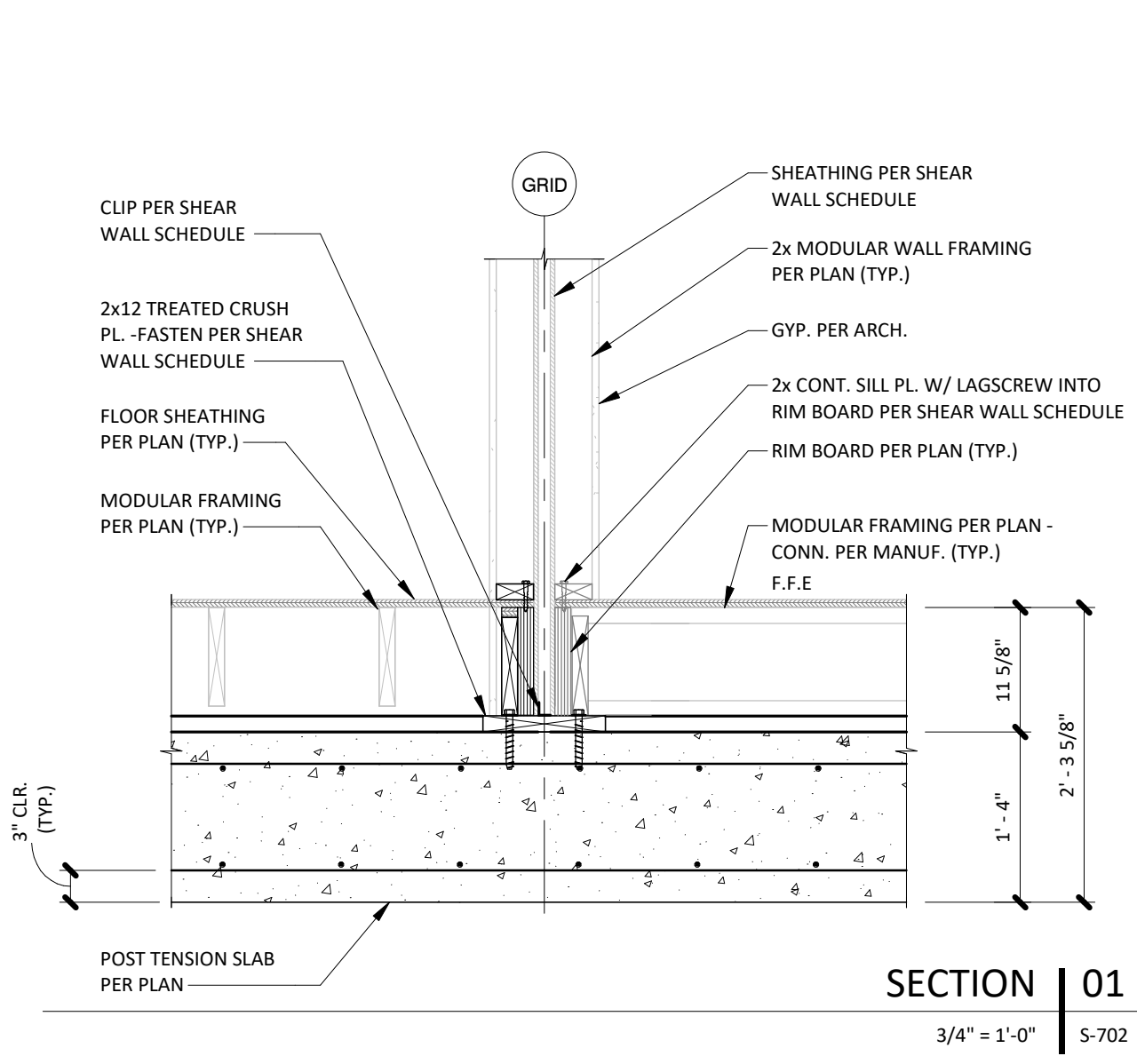
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TYPICAL STEEL DETAILS

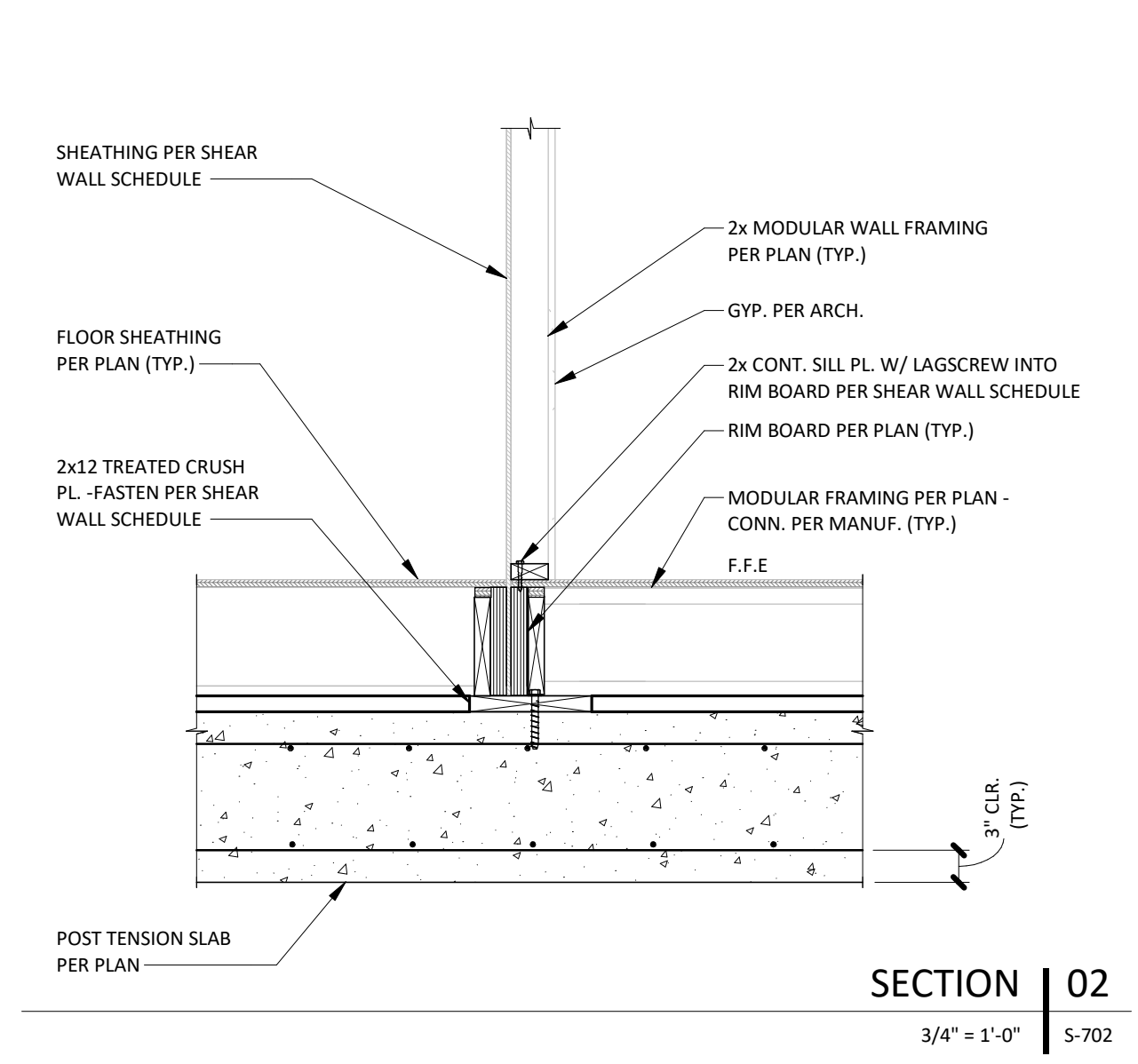
S-613



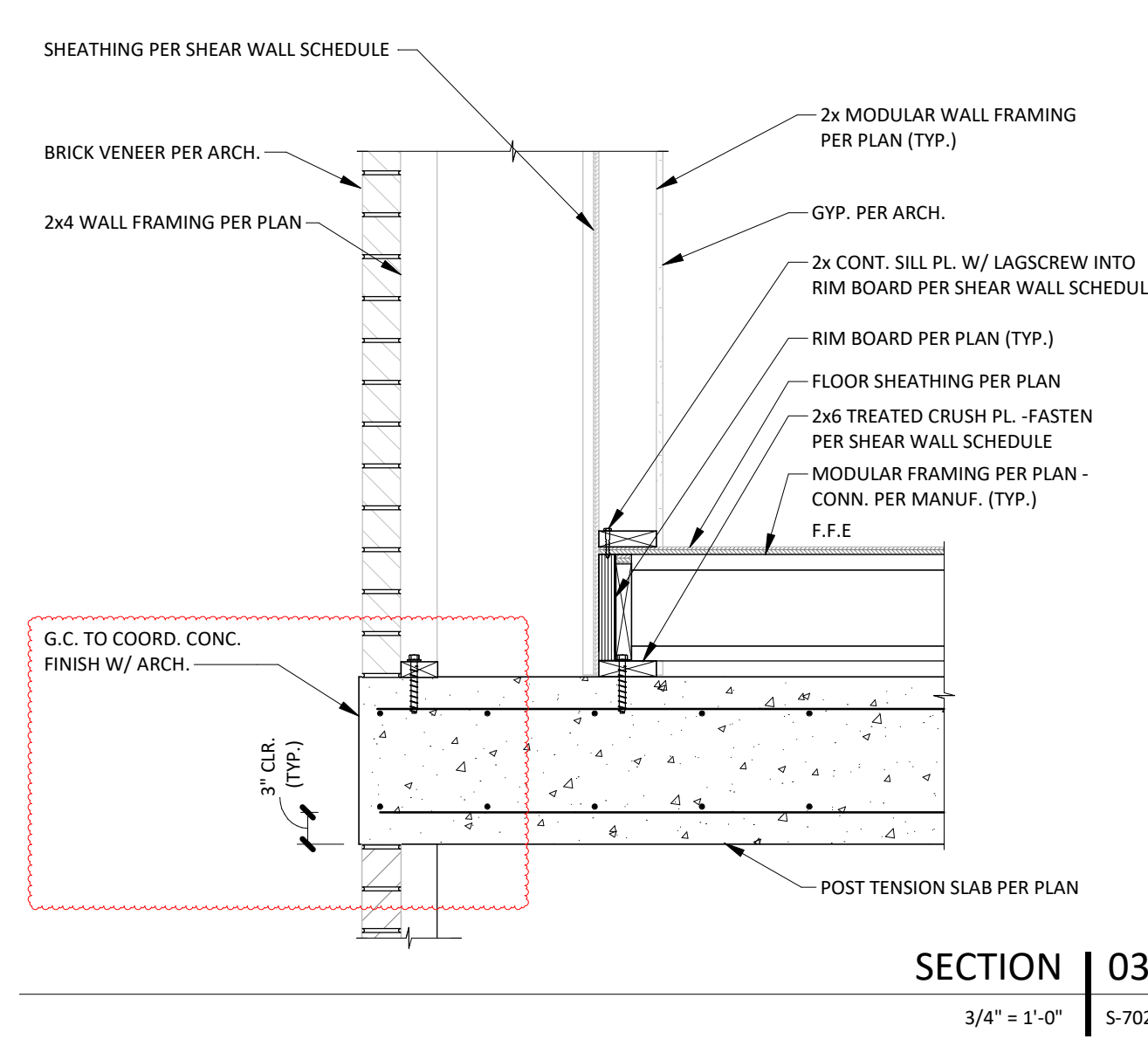




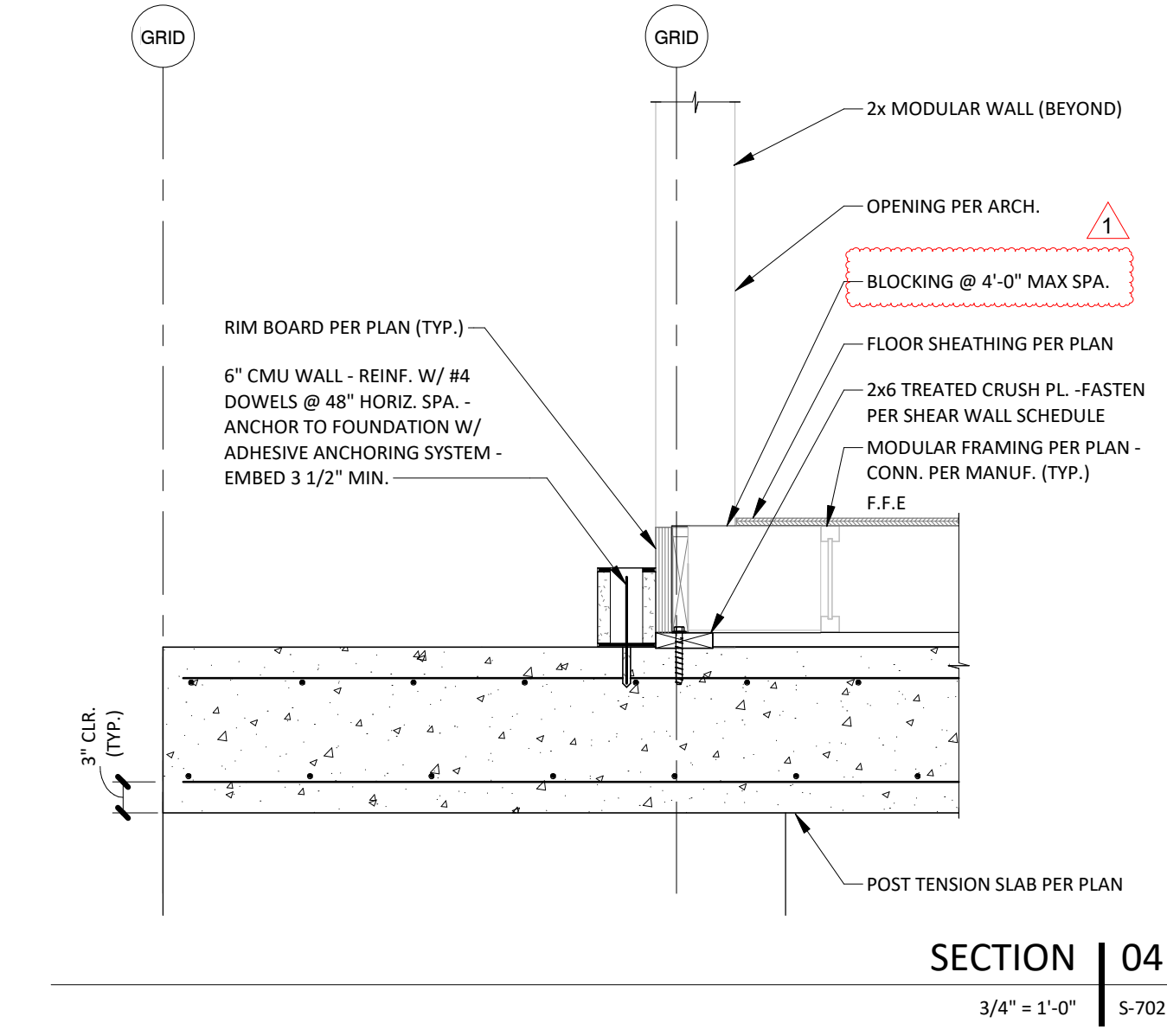
SECTION | 01  
3/4" = 1'-0" S-702



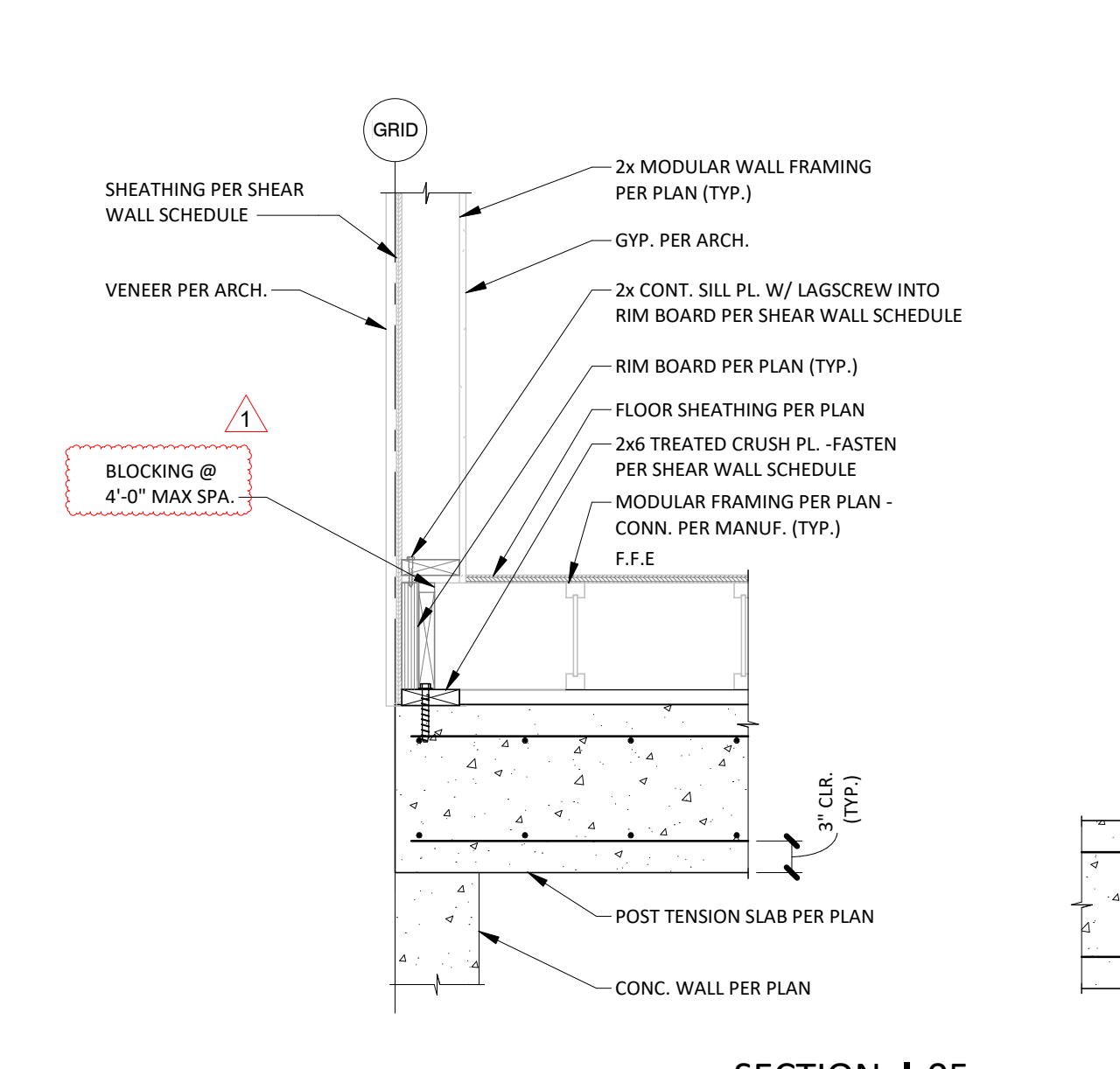
SECTION | 02  
3/4" = 1'-0" S-702



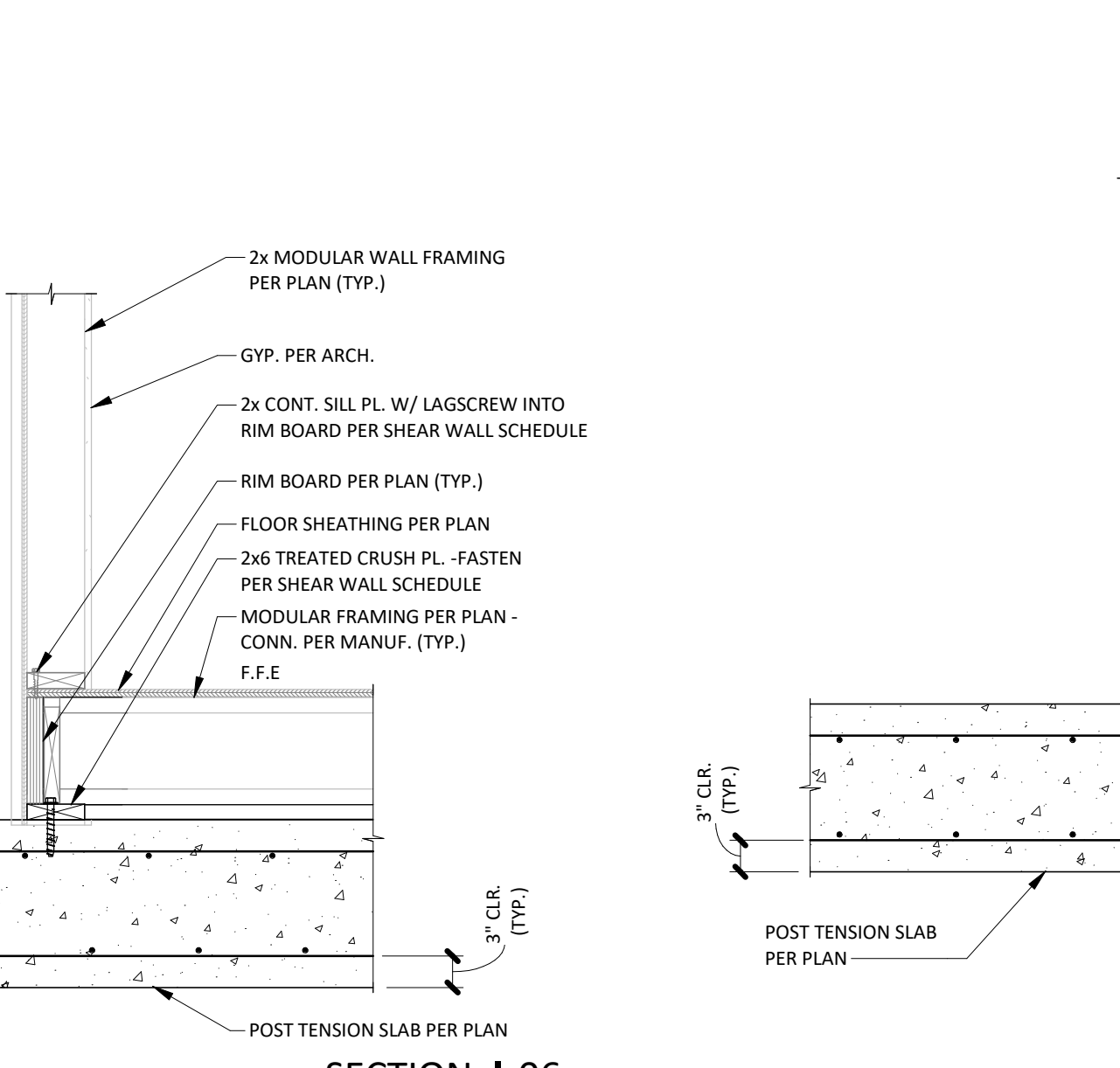
SECTION | 03  
3/4" = 1'-0" S-702



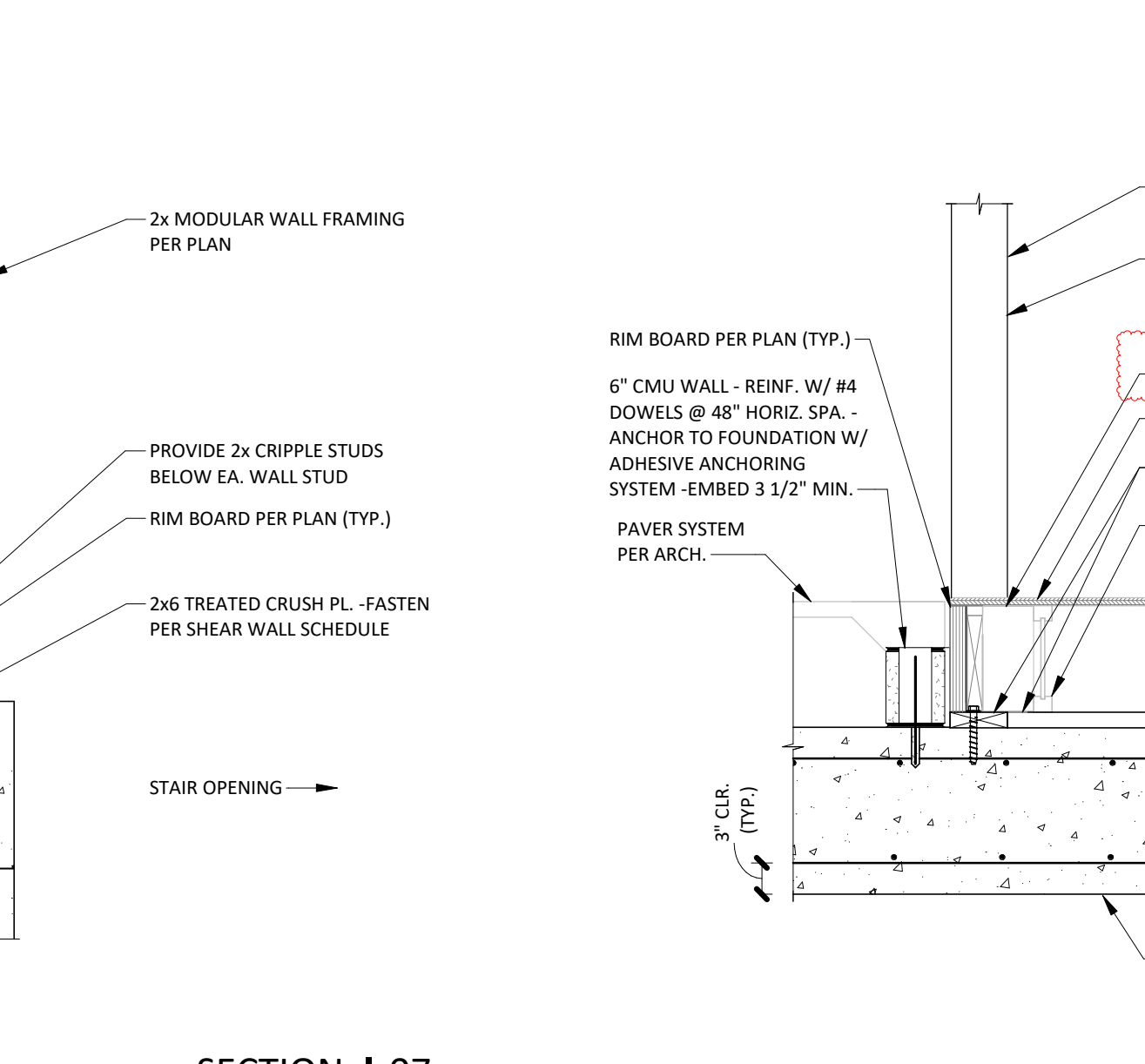
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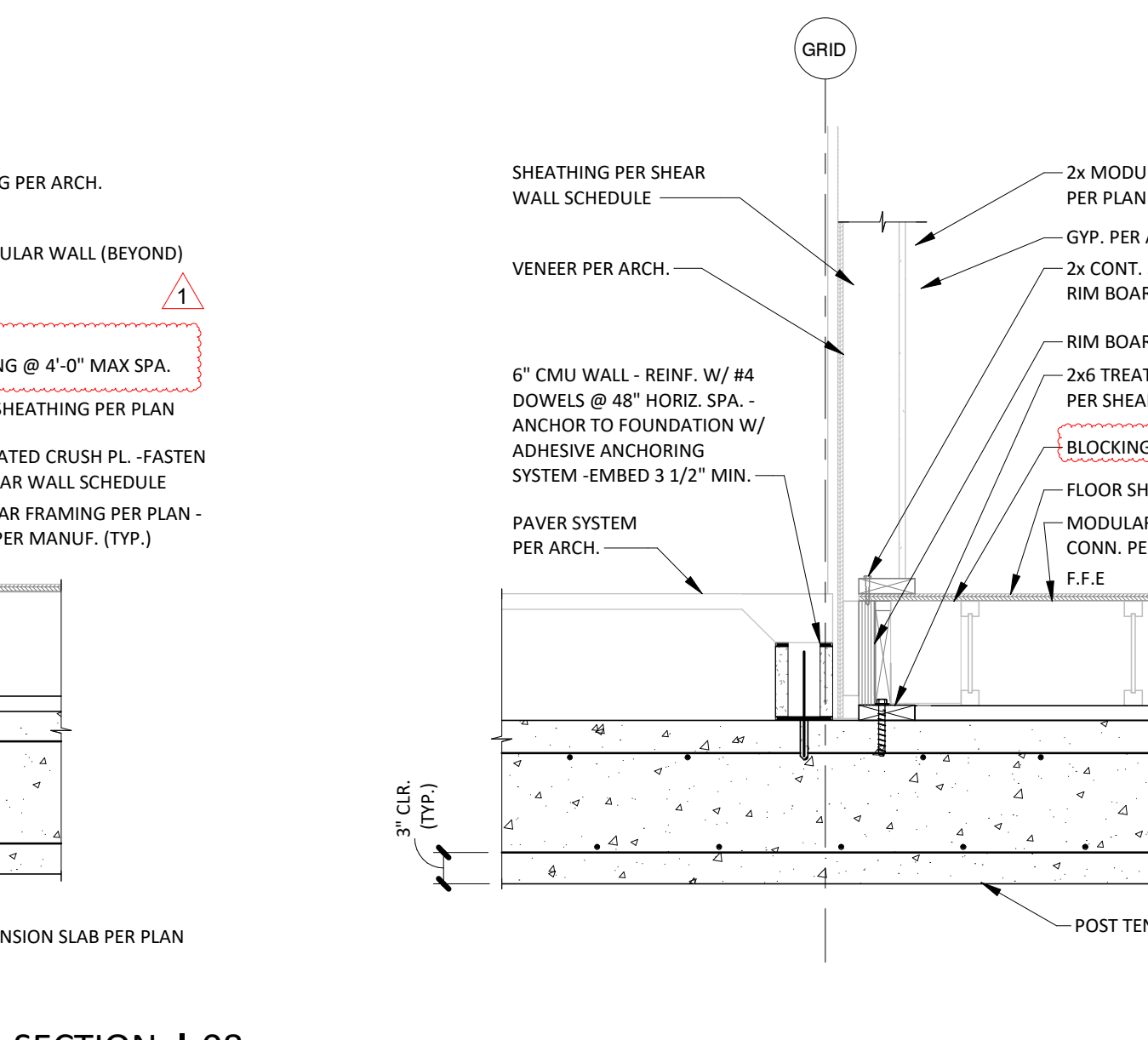
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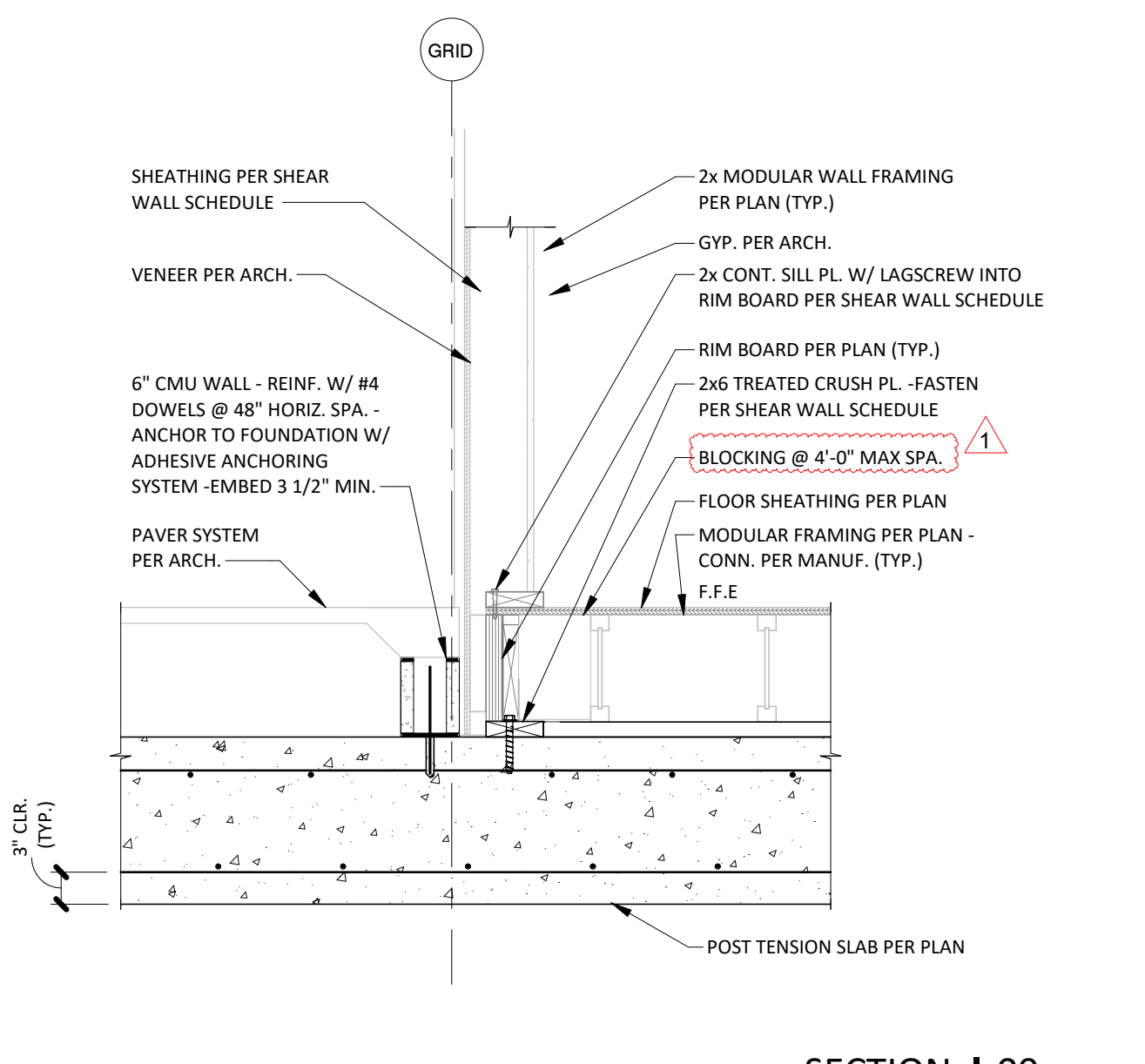
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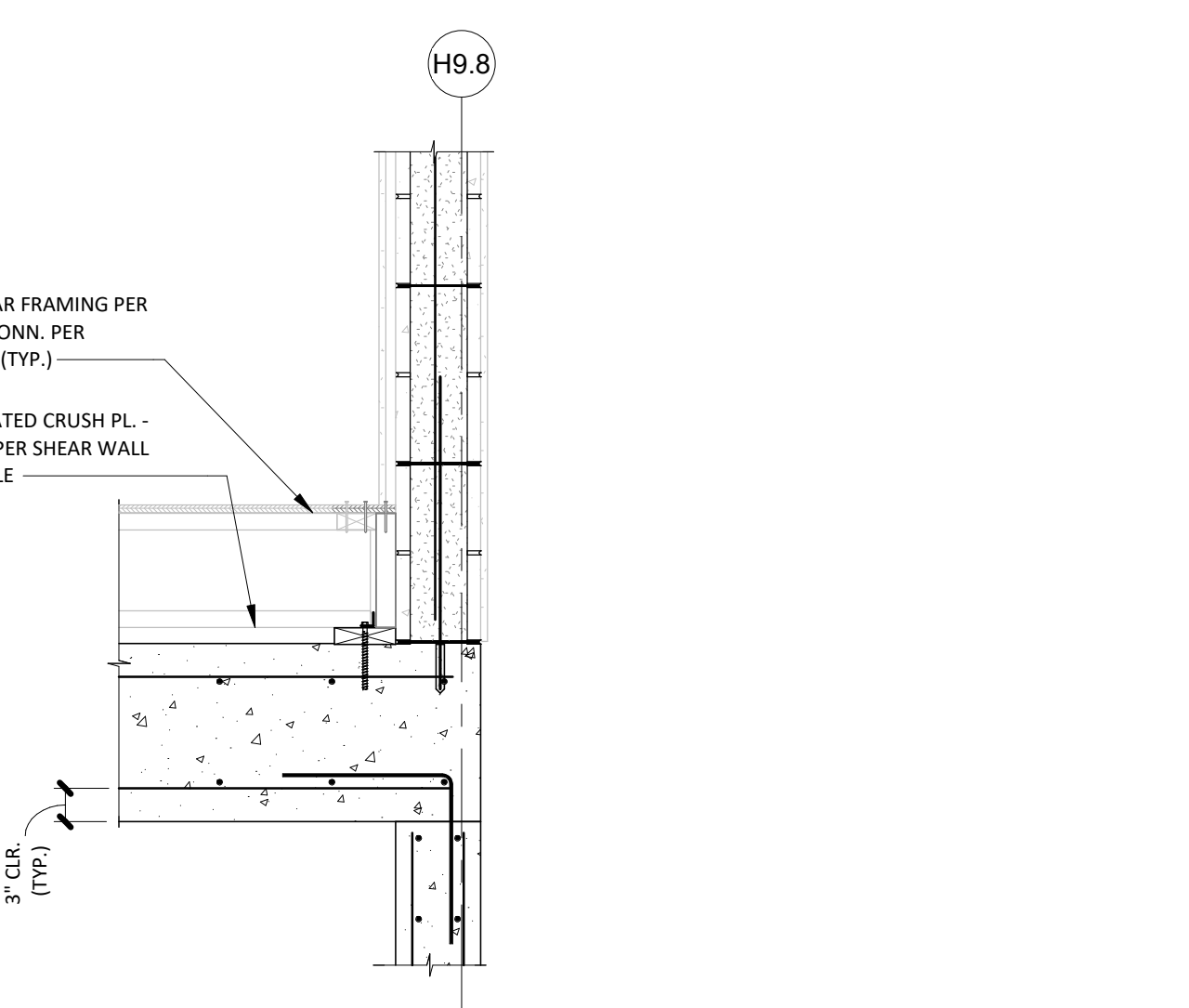
SECTION | 07  
3/4" = 1'-0" S-702



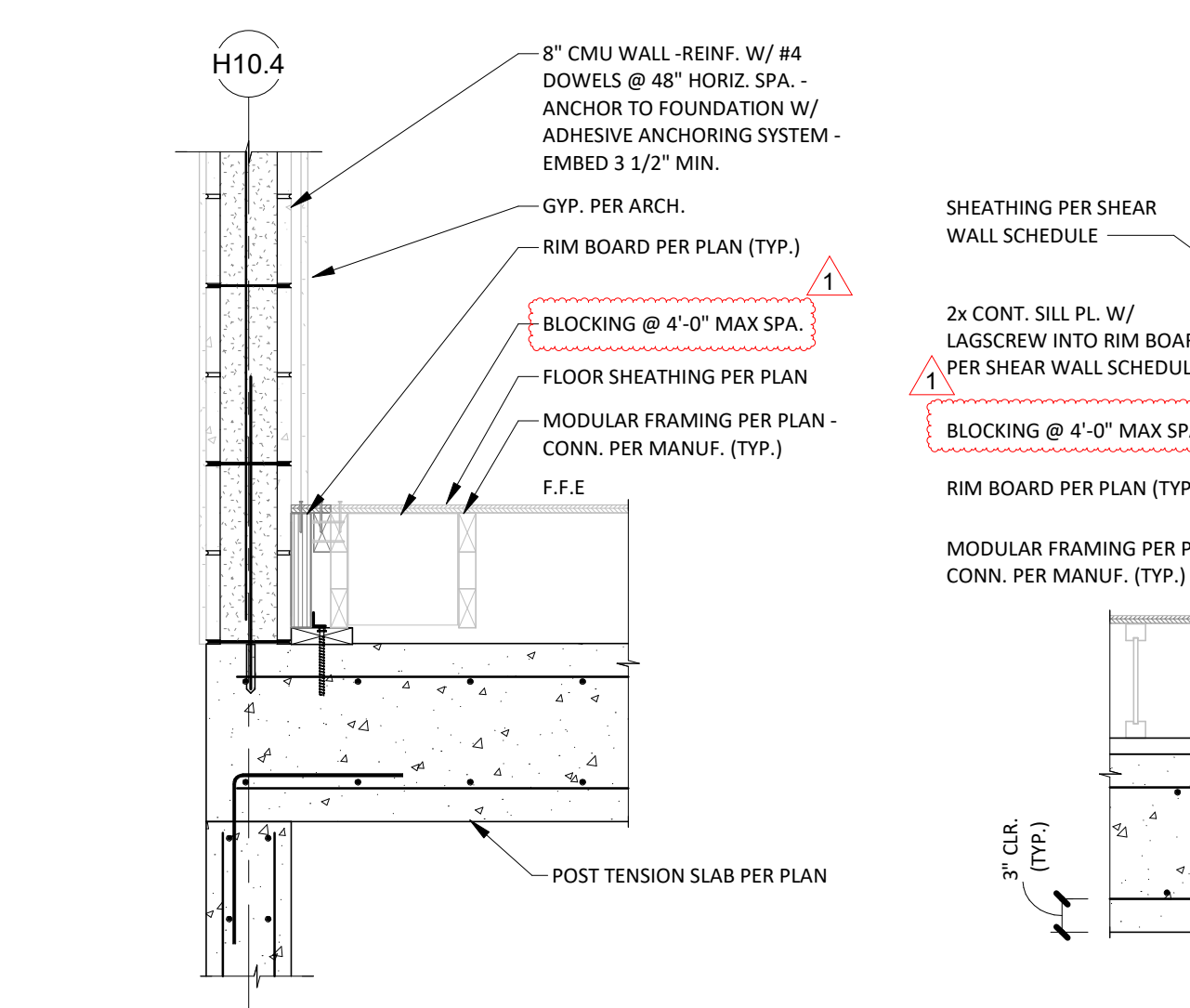
SECTION | 08  
3/4" = 1'-0" S-702



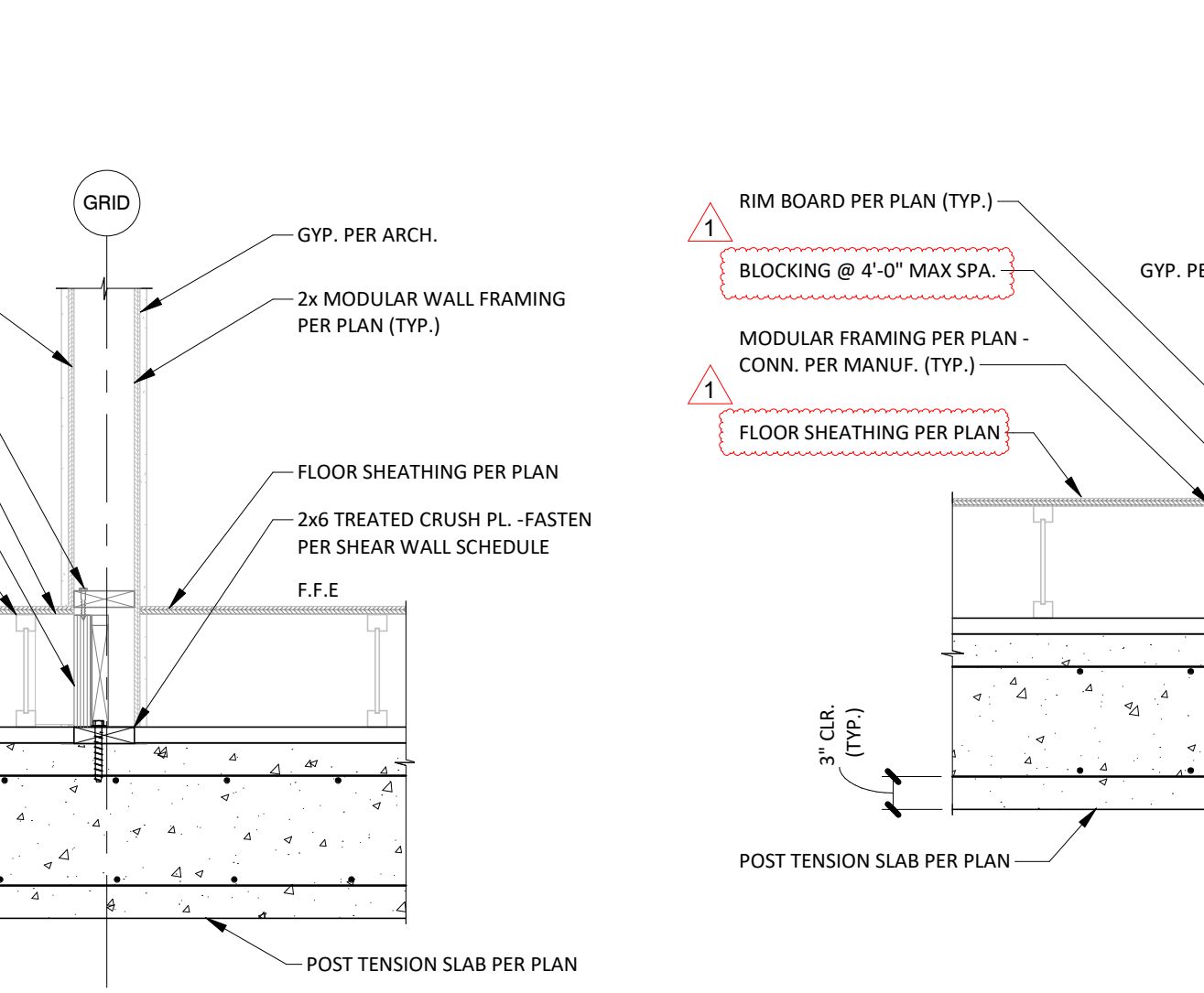
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3/4" = 1'-0" S-702



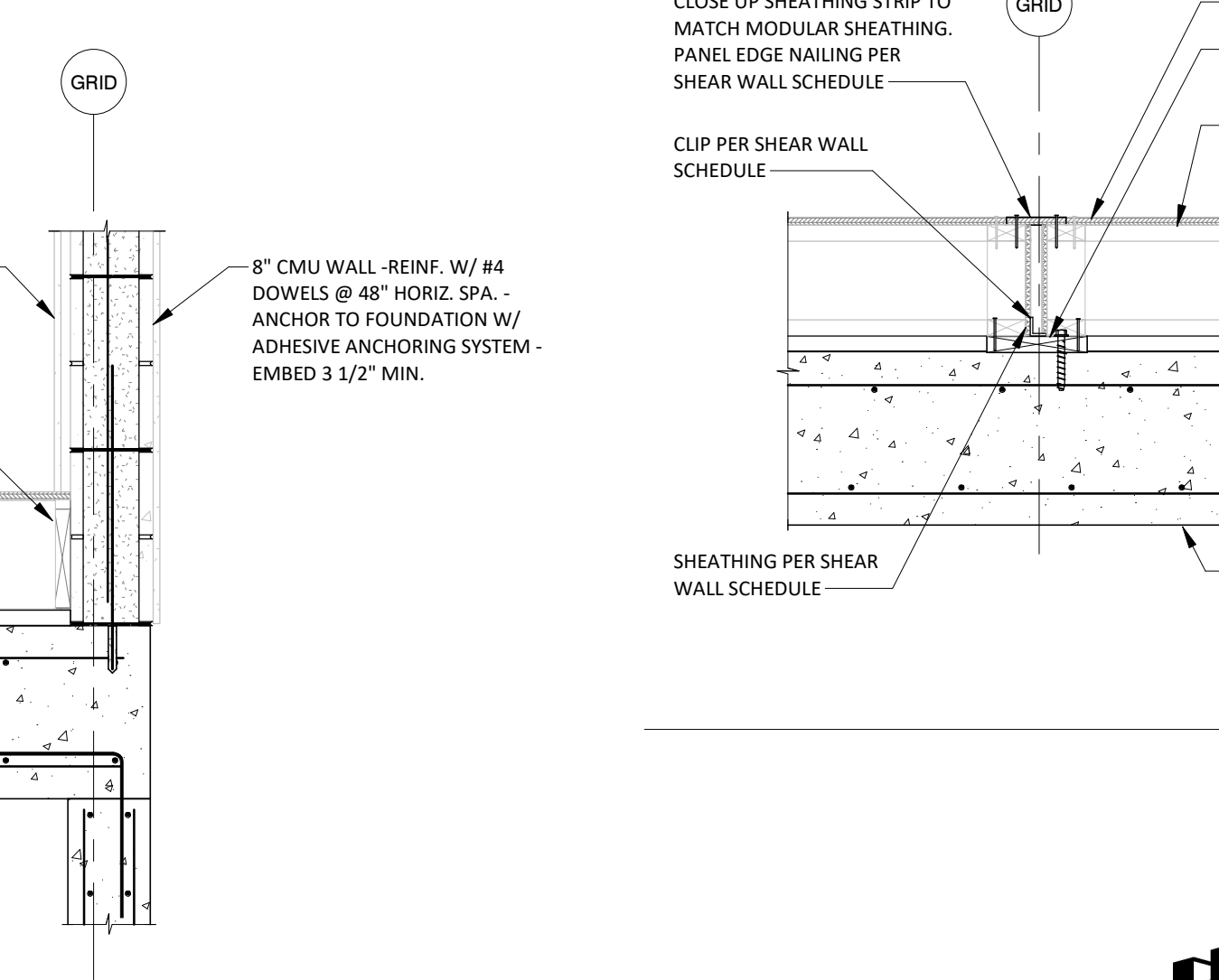
SECTION | 10  
3/4" = 1'-0" S-702



SECTION | 11  
3/4" = 1'-0" S-702



SECTION | 12  
3/4" = 1'-0" S-702



SECTION | 13  
3/4" = 1'-0" S-702

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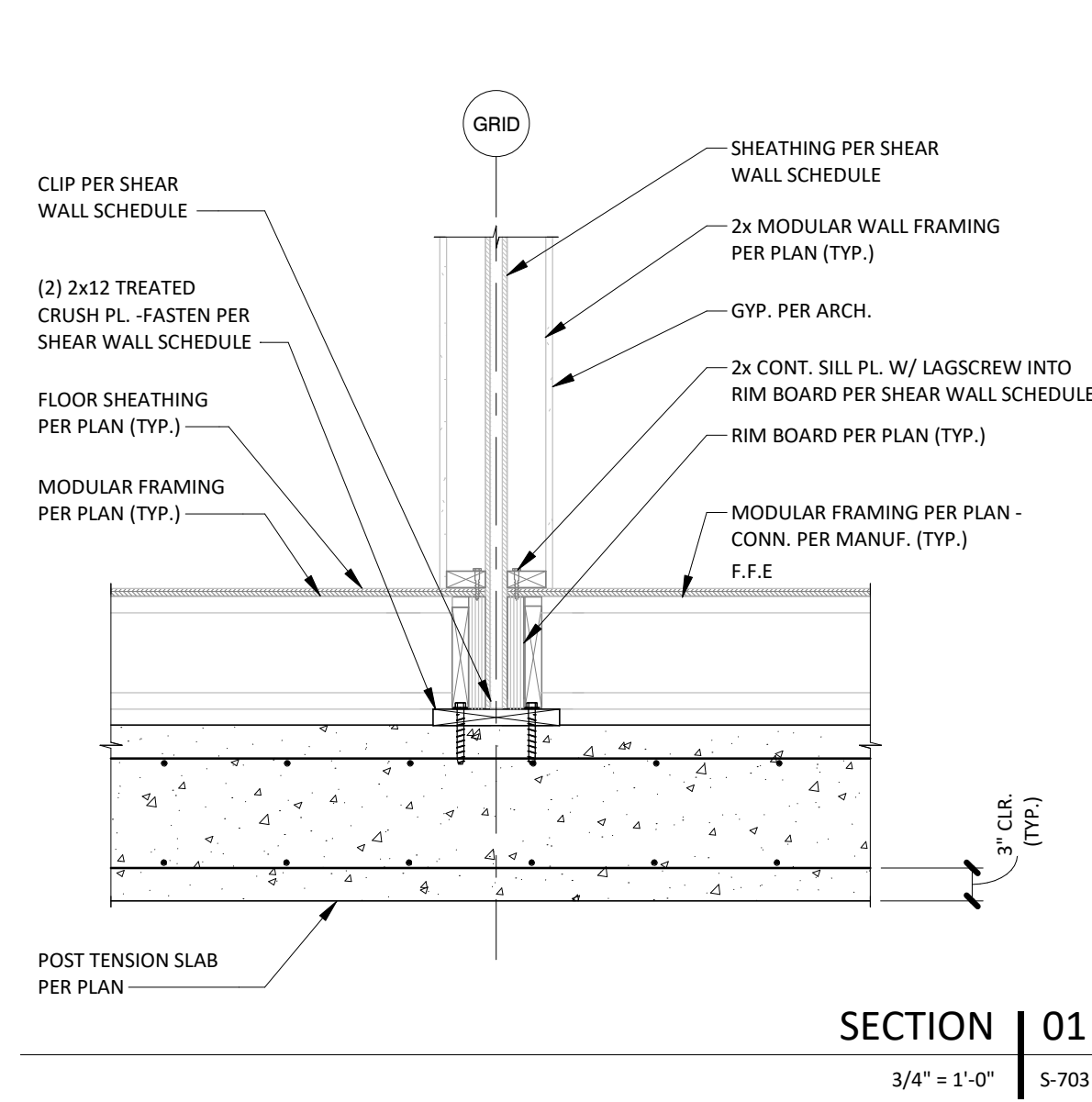


JOB NO: 18-007  
DATE: 2018.03.07  
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REVISIONS:  
1 05/17/2019

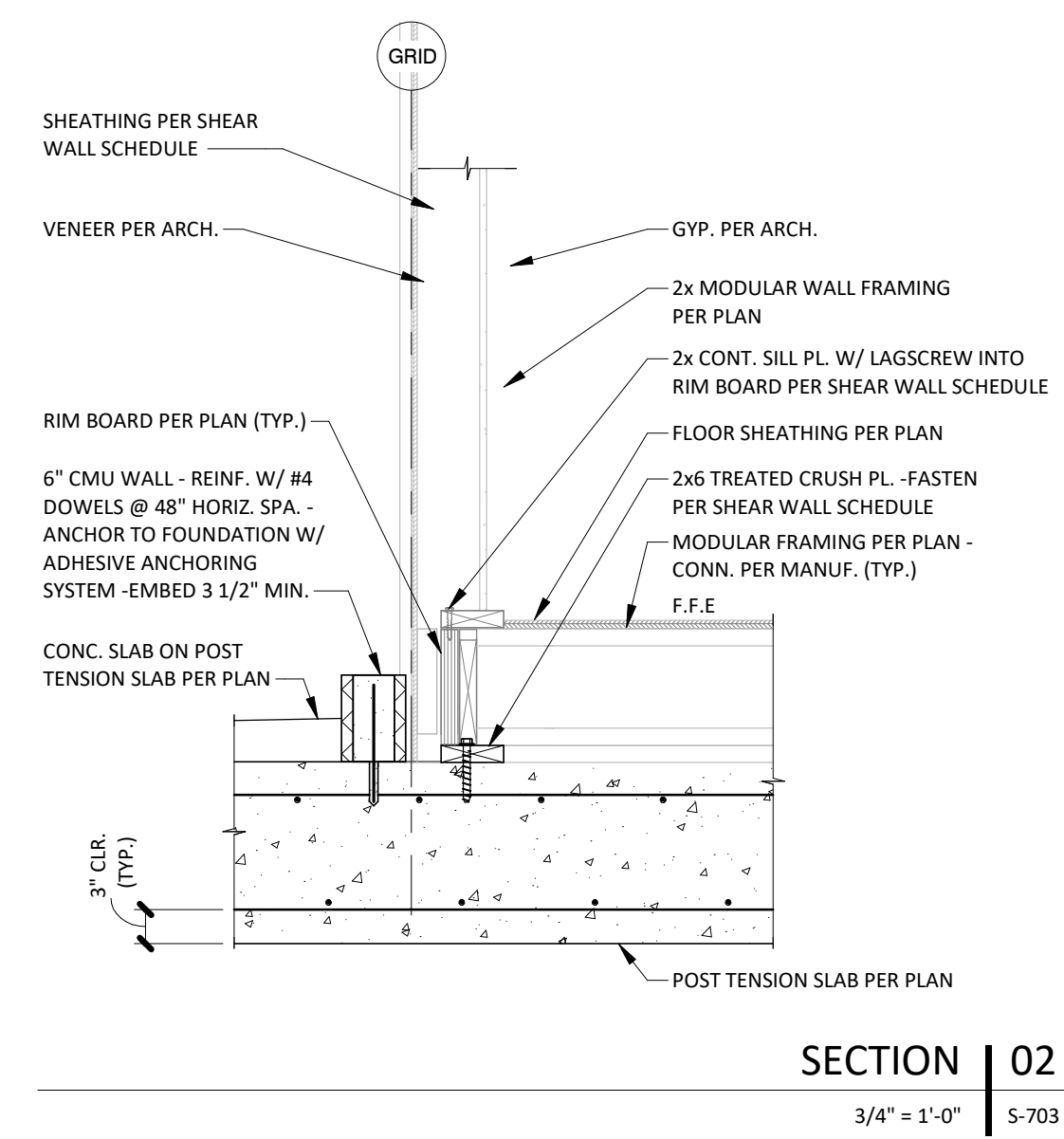
**NEW 2019.06.14**

**S-702**

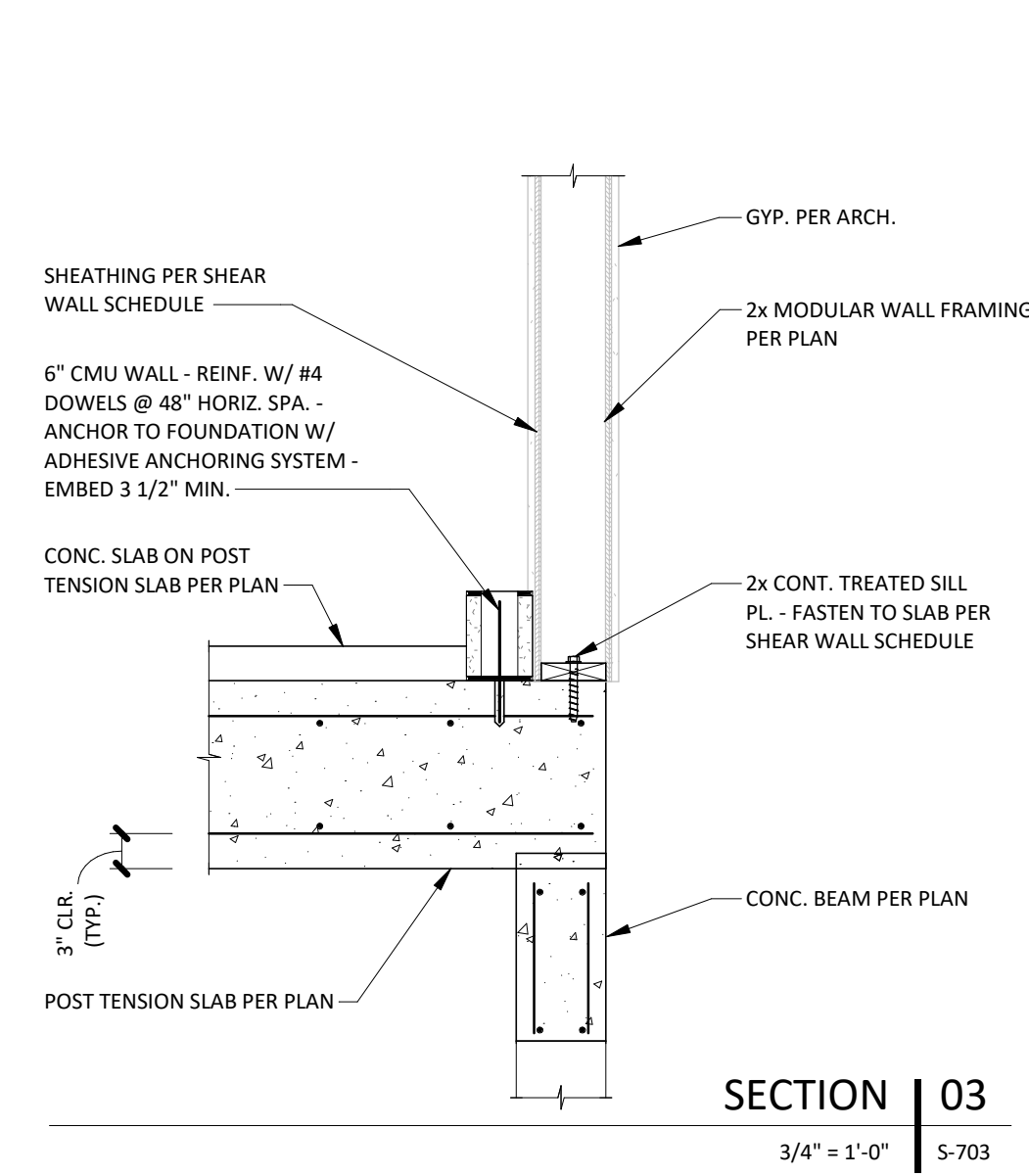
HOTEL FRAMING DETAILS



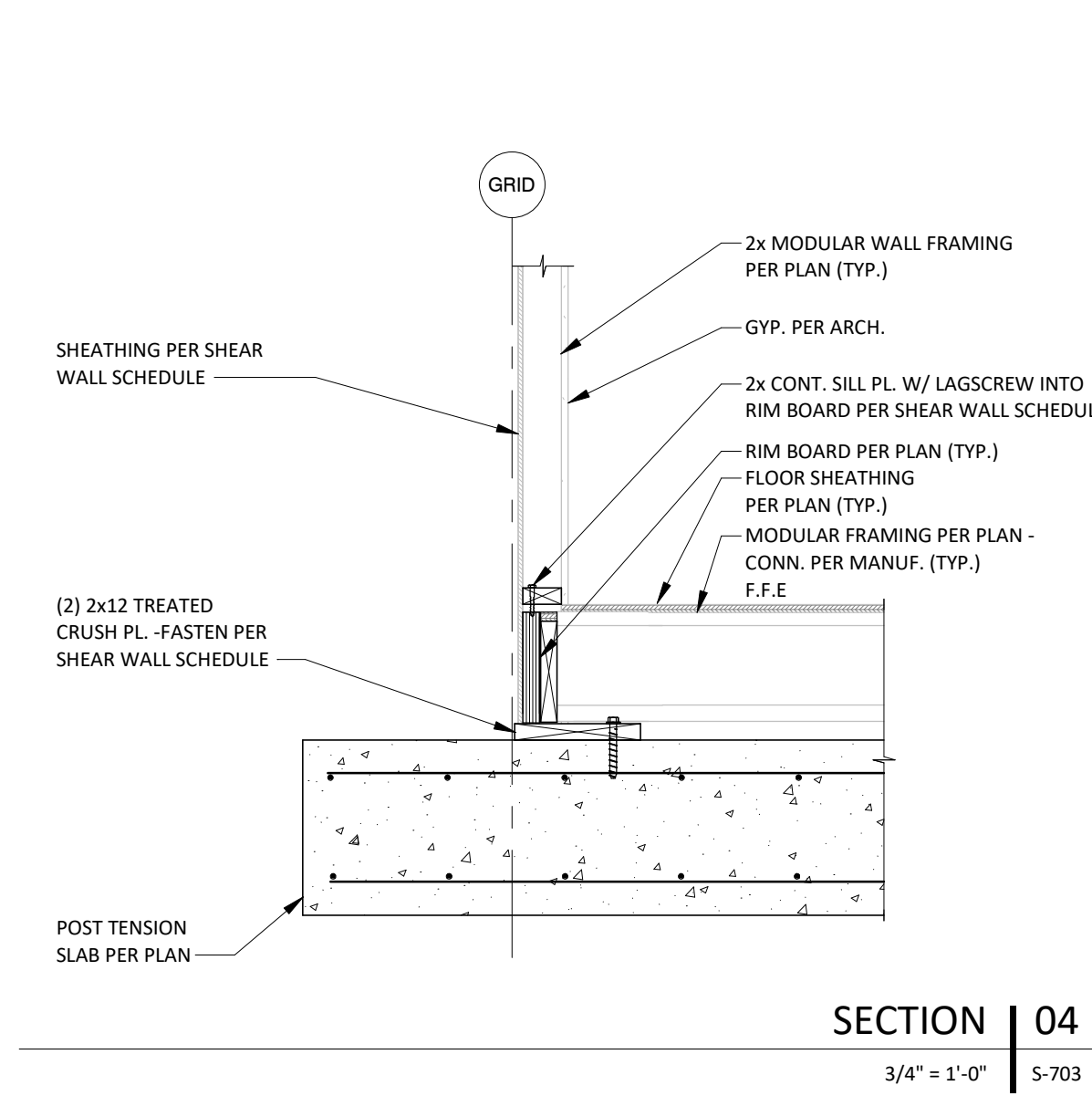
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3/4" = 1'-0" S-703



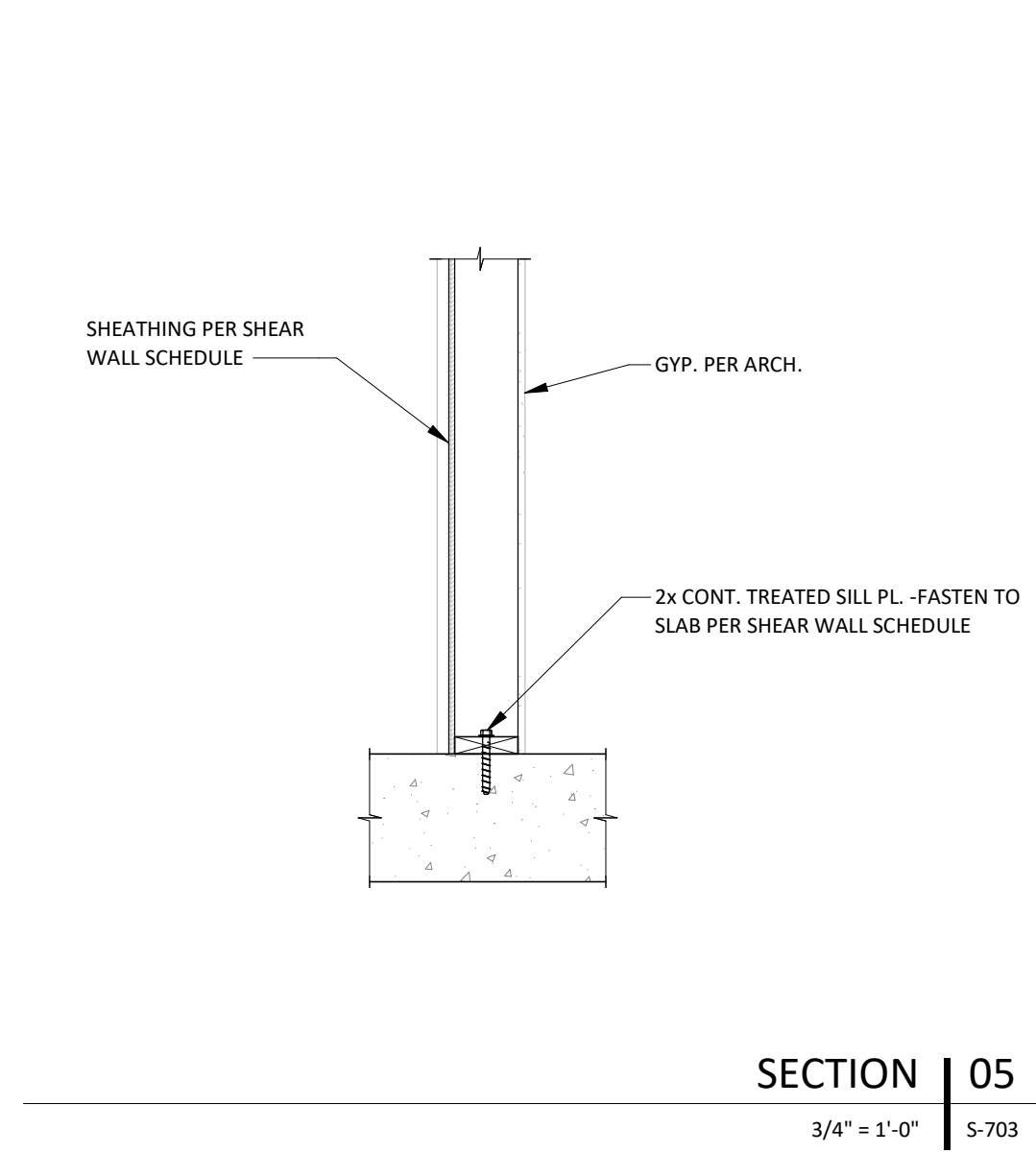
SECTION 02  
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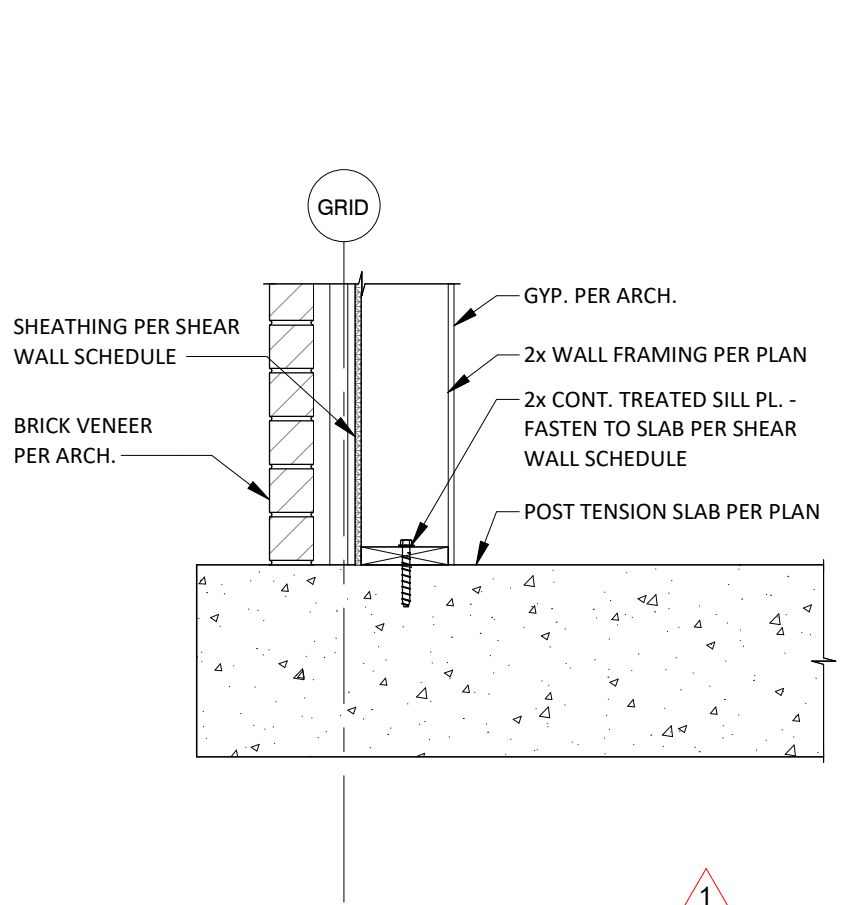
SECTION 03  
3/4" = 1'-0" S-703



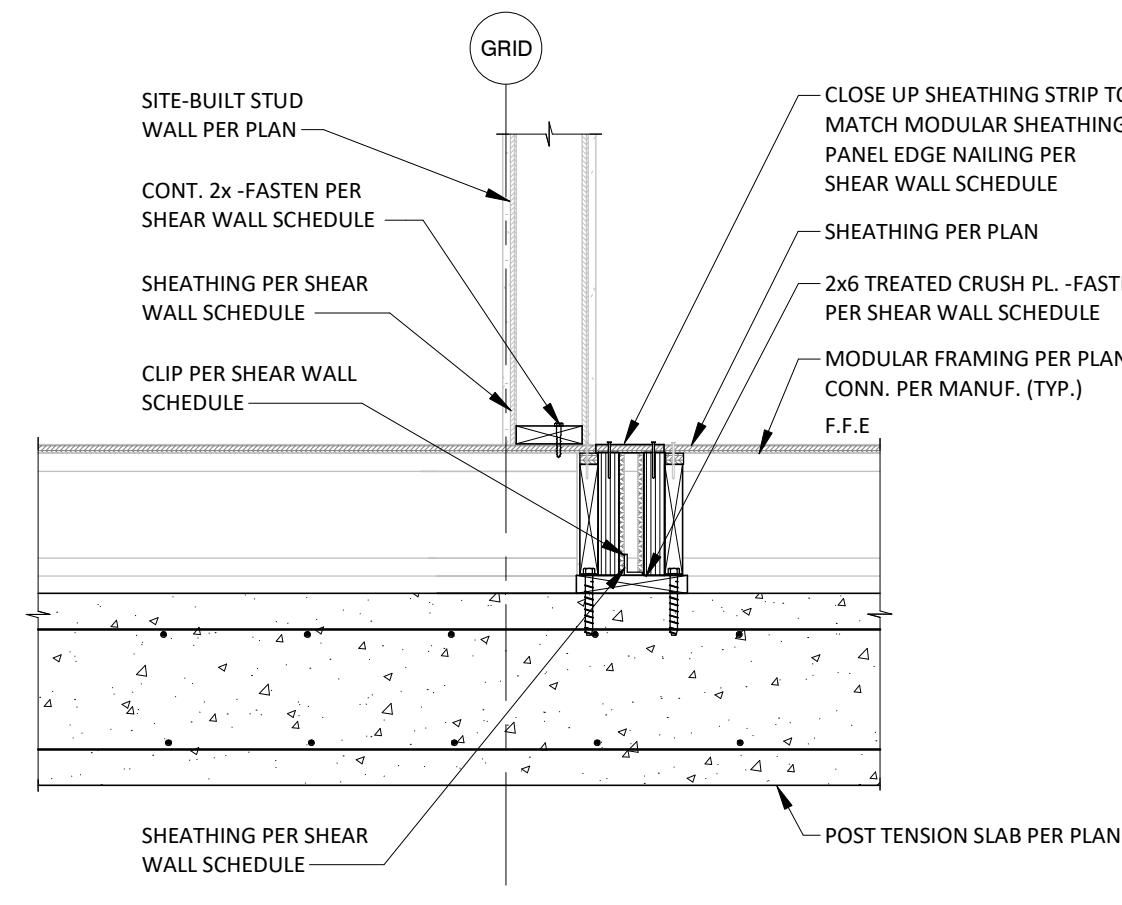
SECTION 04  
3/4" = 1'-0" S-703



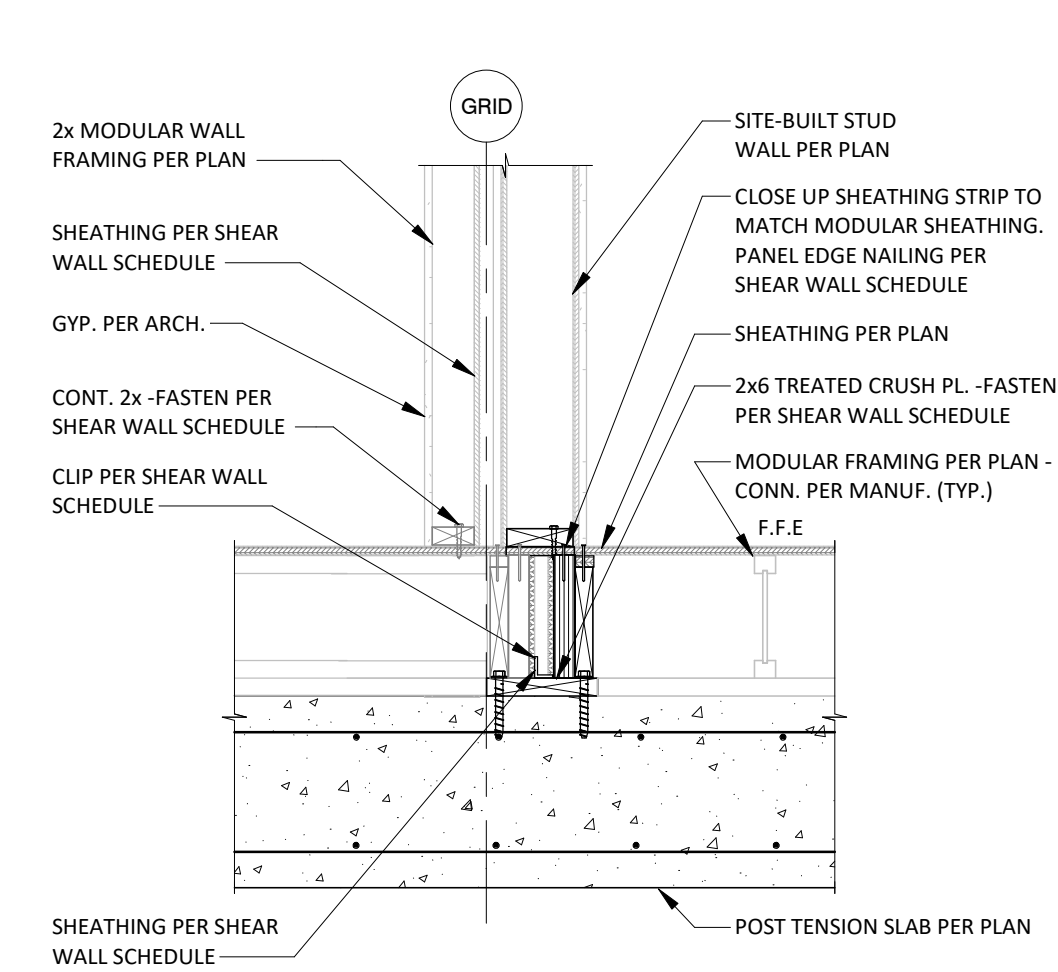
SECTION 05  
3/4" = 1'-0" S-703



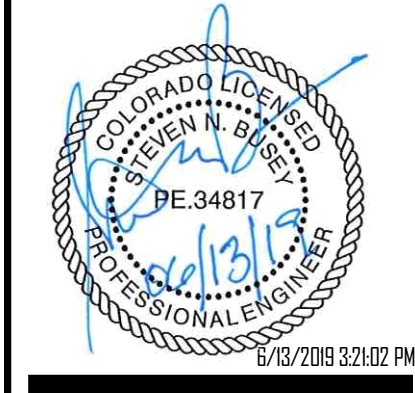
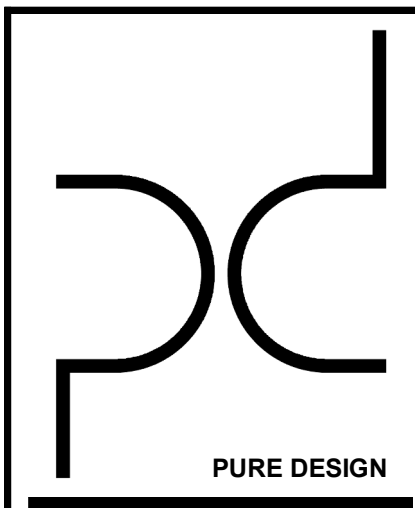
SECTION 06  
3/4" = 1'-0" S-703



SECTION 07  
3/4" = 1'-0" S-703



SECTION 08  
3/4" = 1'-0" S-703



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AC HOTELS

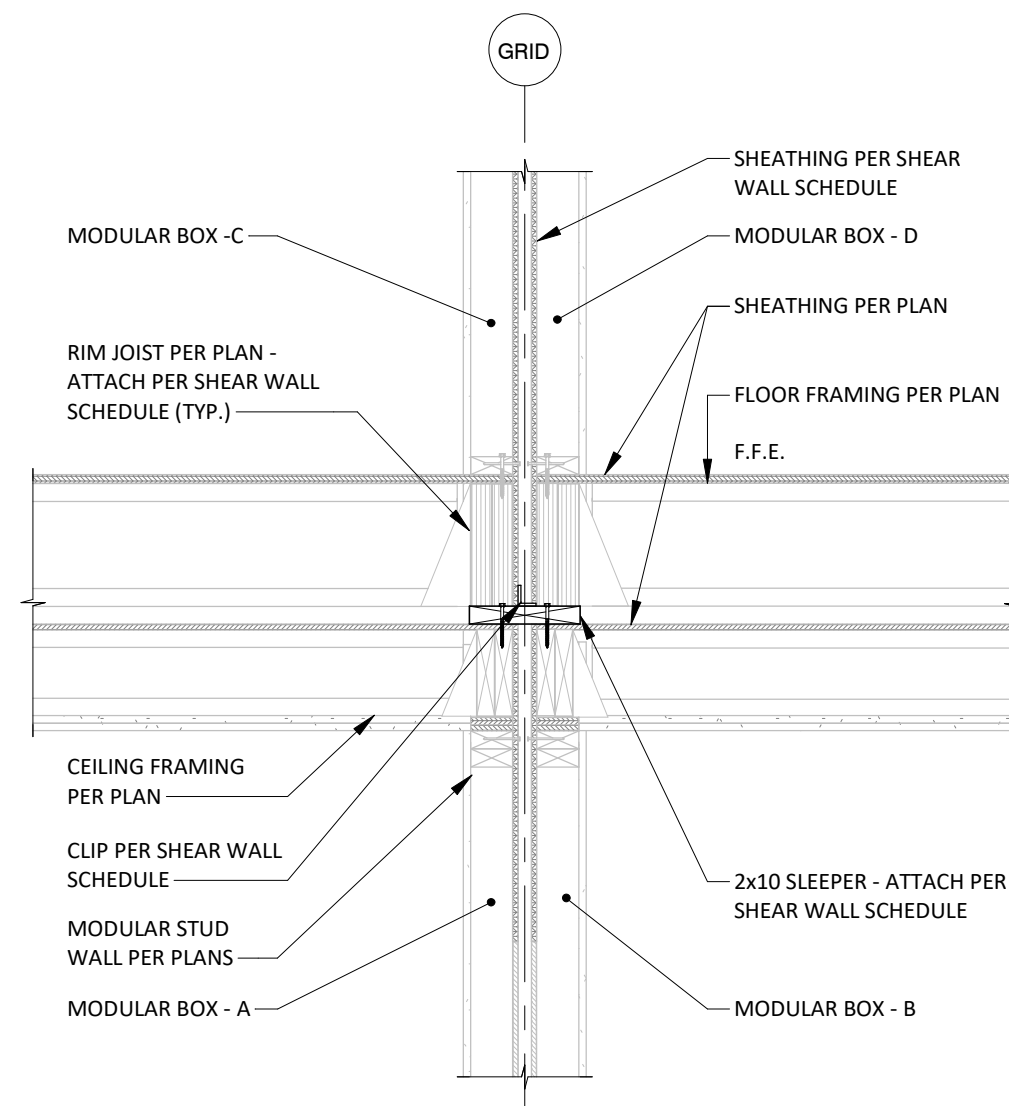
JOB NO: 18-007  
DATE: 2018.03.07  
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REVISIONS:	DATE
1	05/17/2019

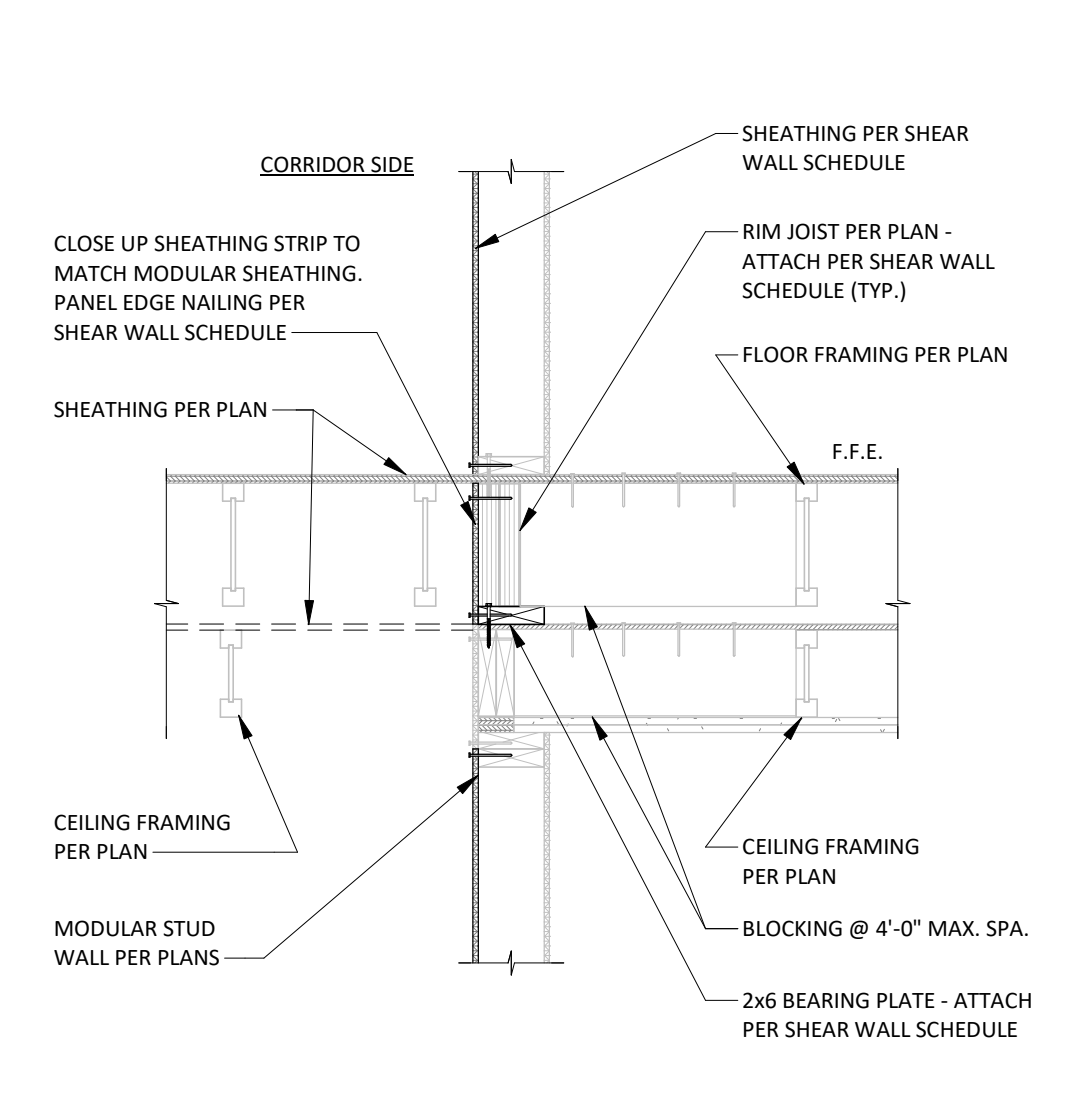
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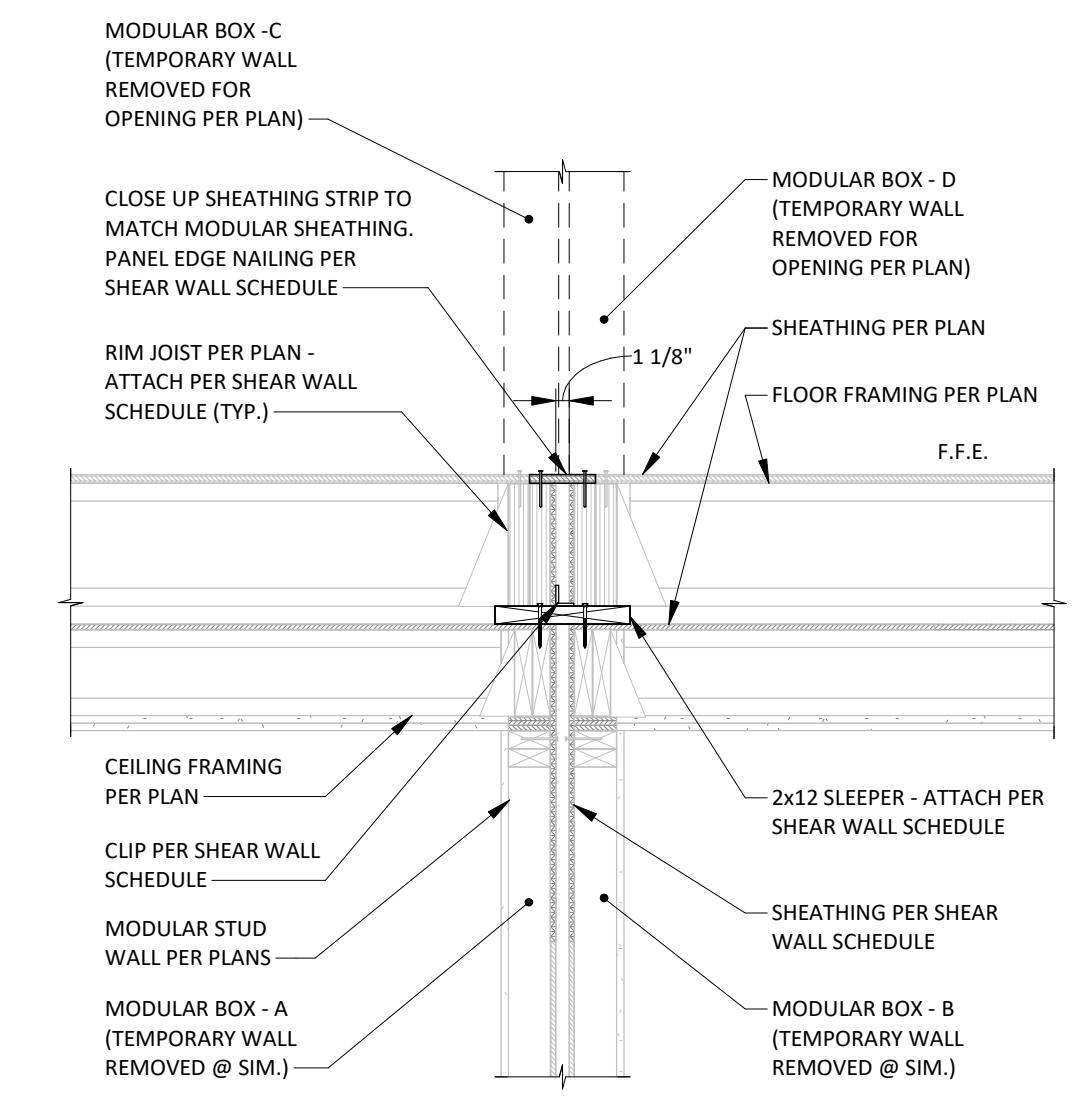
HOTEL FRAMING DETAILS  
**S-703**



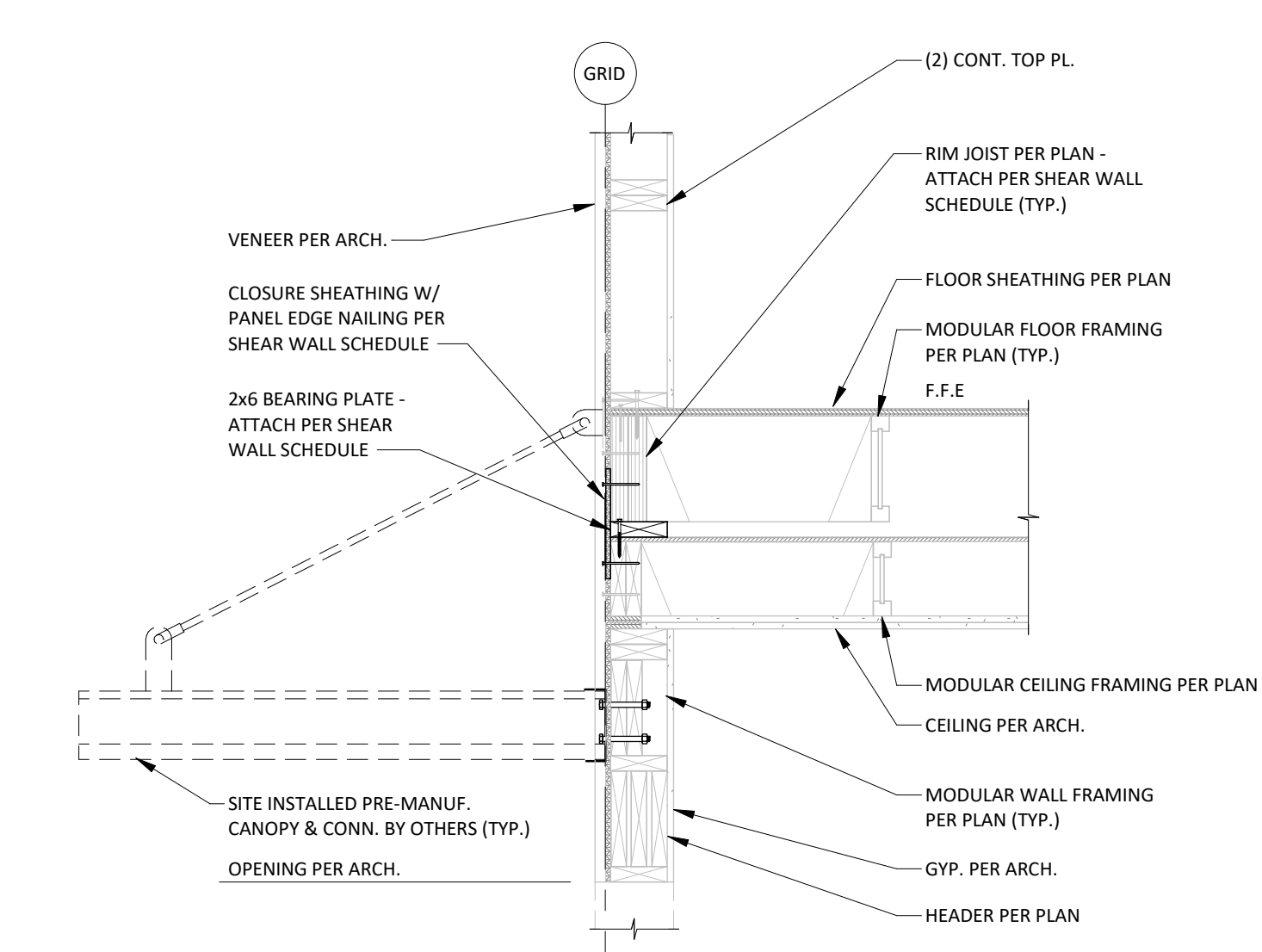
SECTION | 01  
3/4" = 1'-0" | S-704



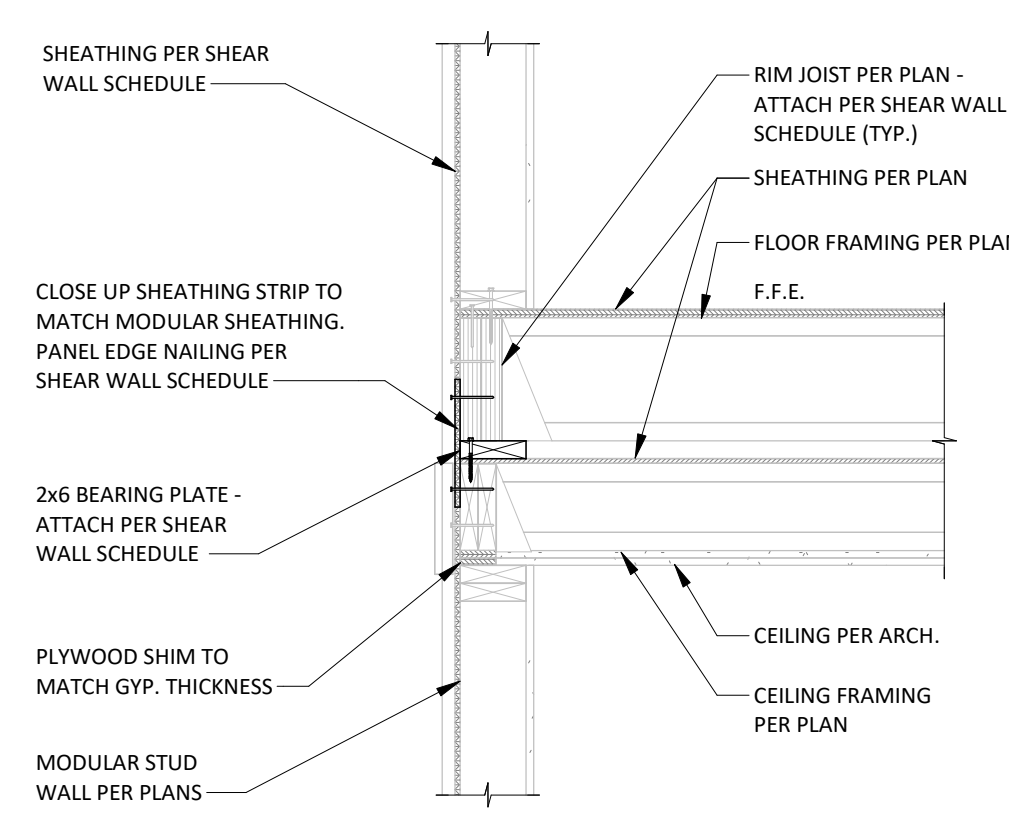
SECTION | 02  
3/4" = 1'-0" | S-704



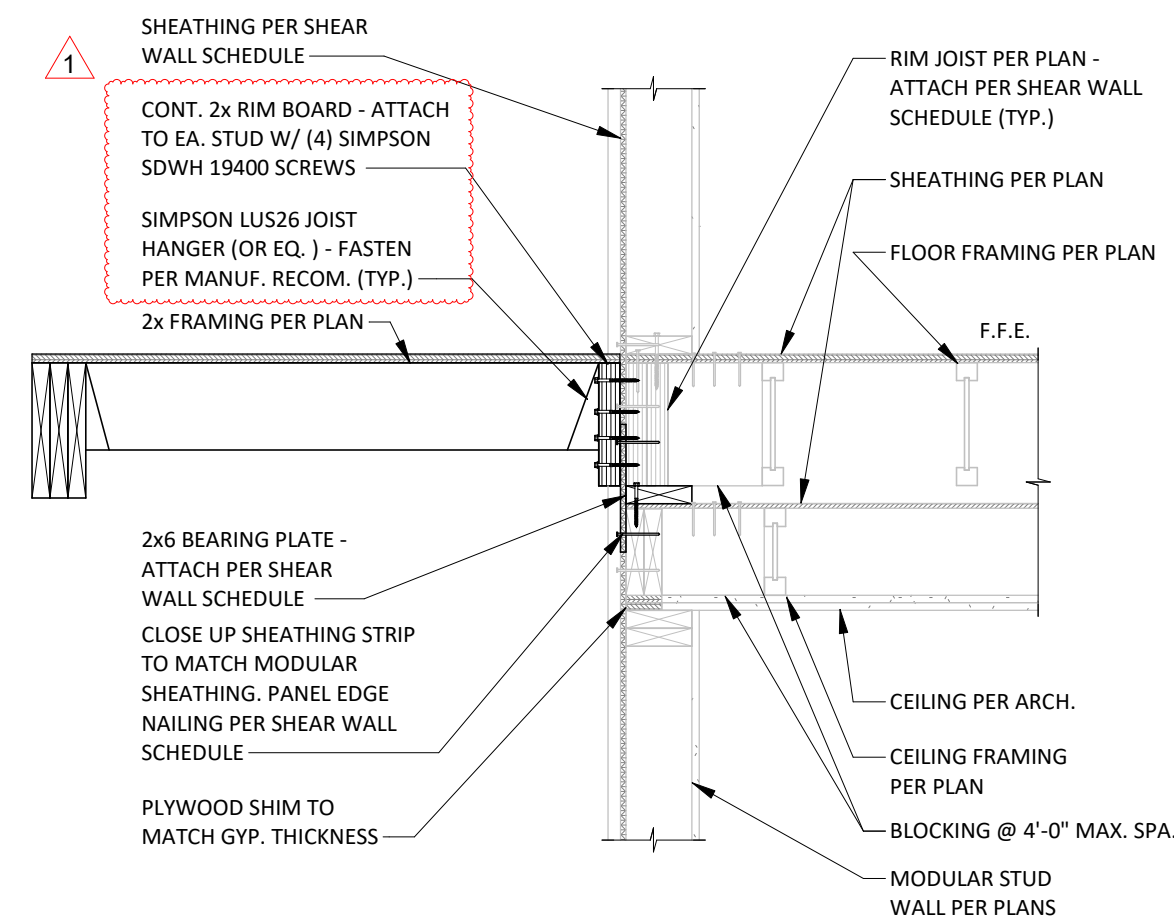
SECTION | 03  
3/4" = 1'-0" | S-704



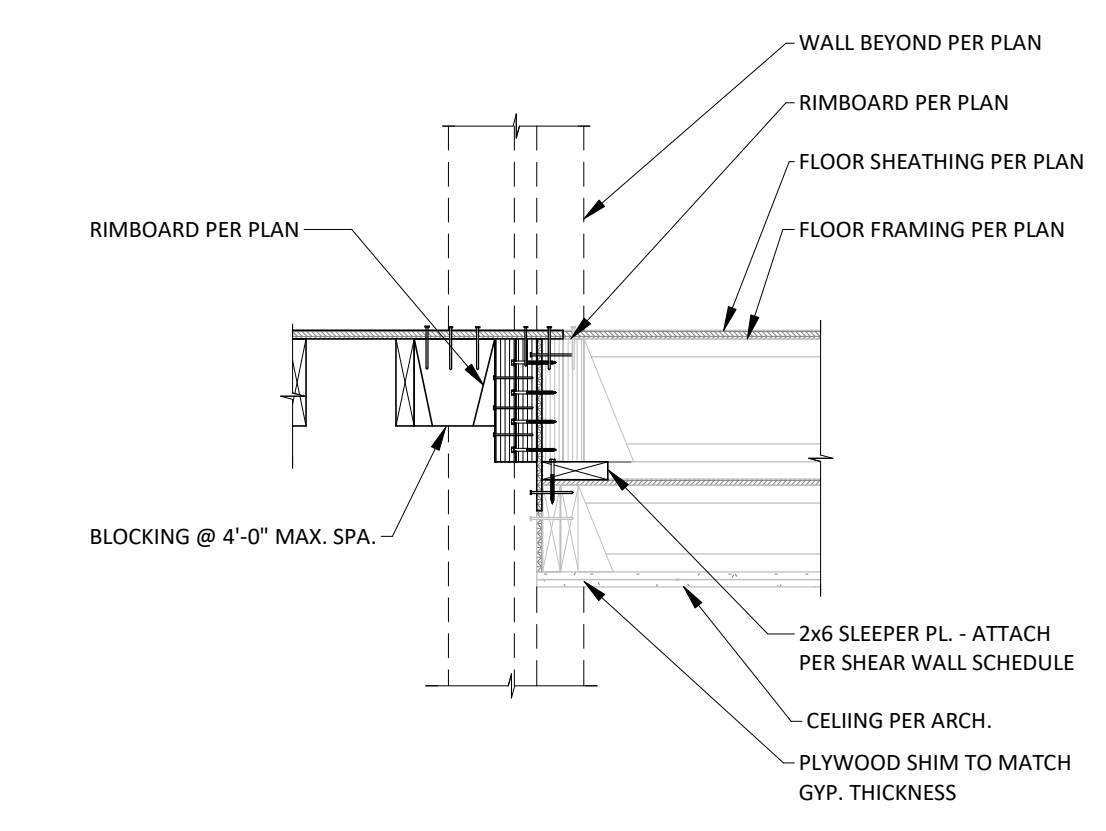
SECTION | 04  
3/4" = 1'-0" | S-704



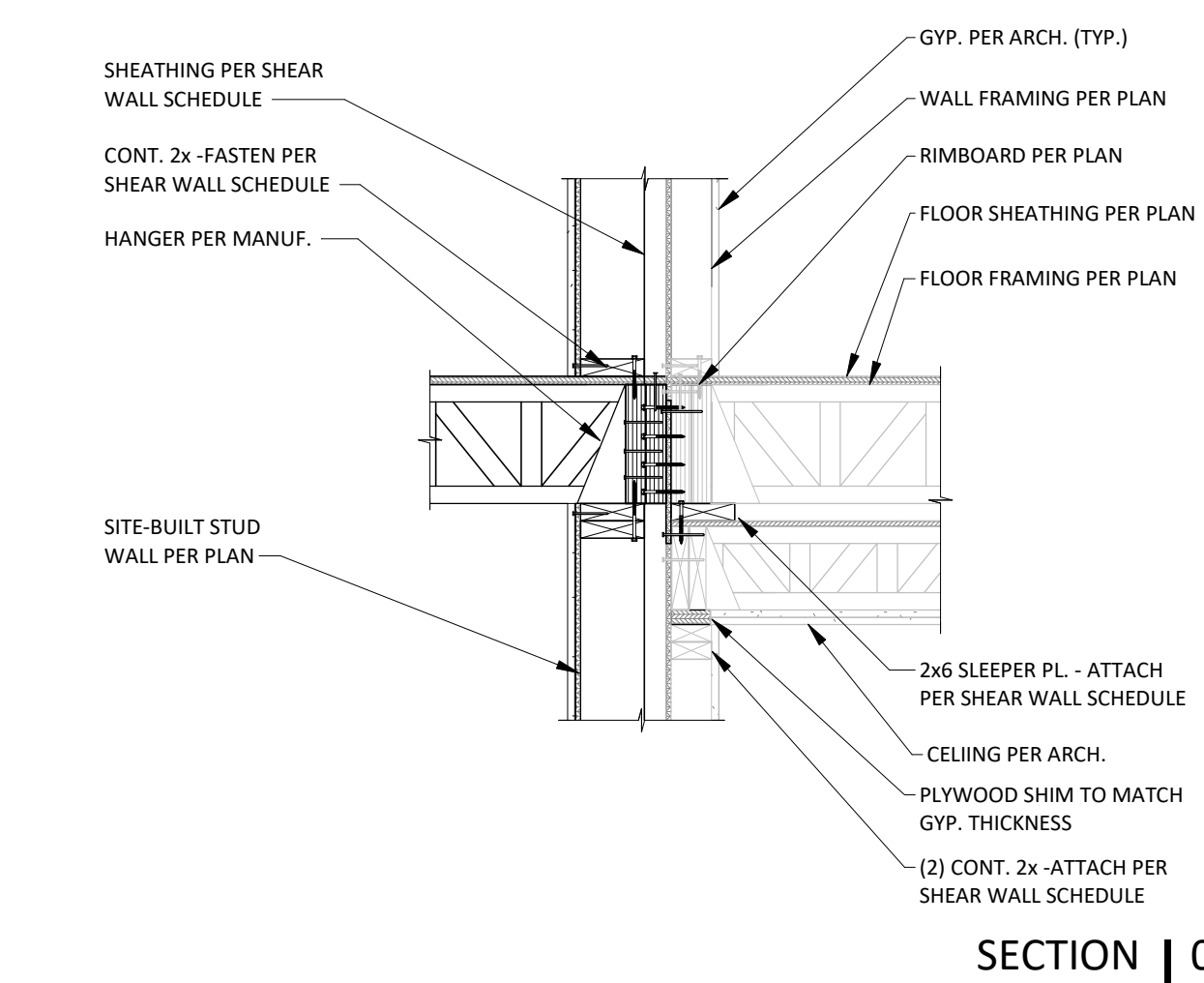
SECTION | 05  
3/4" = 1'-0" | S-704



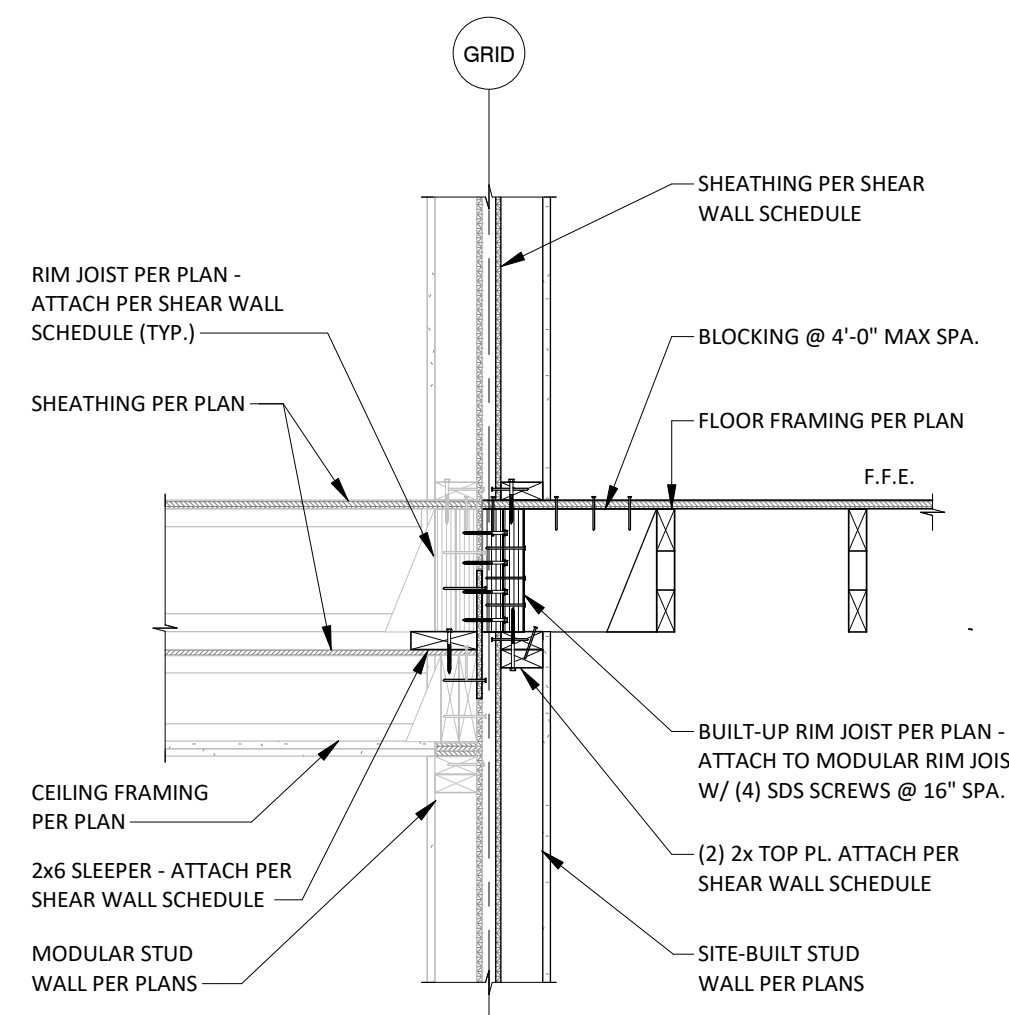
SECTION | 06  
3/4" = 1'-0" | S-704



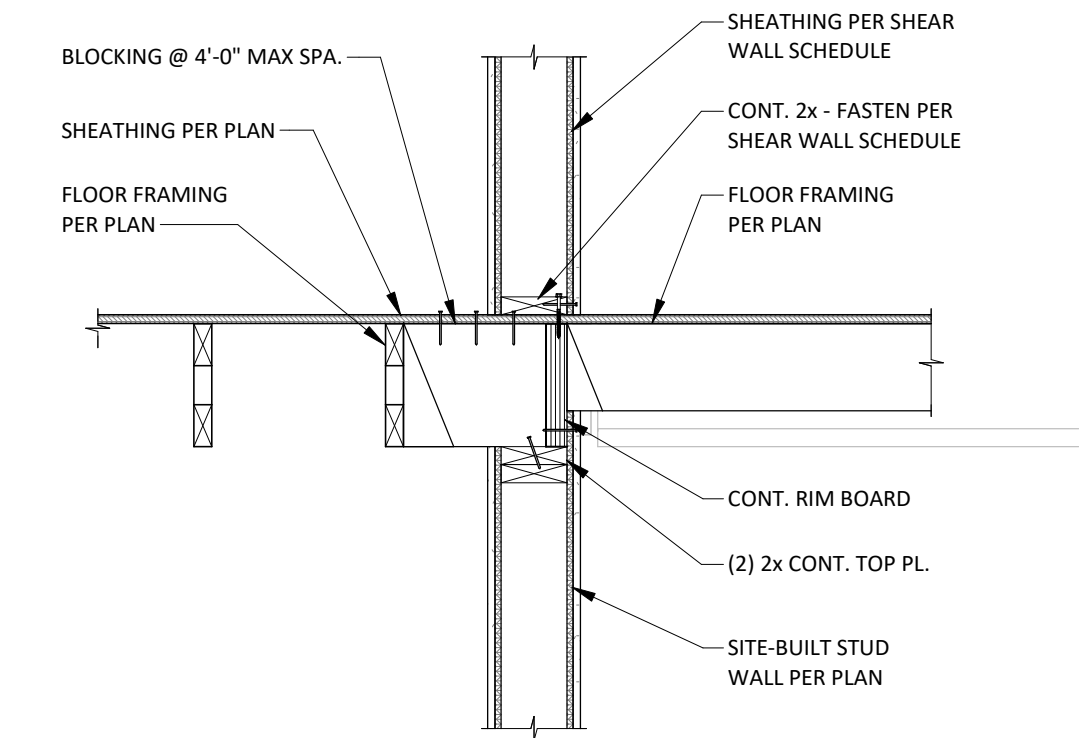
SECTION | 07  
3/4" = 1'-0" | S-704



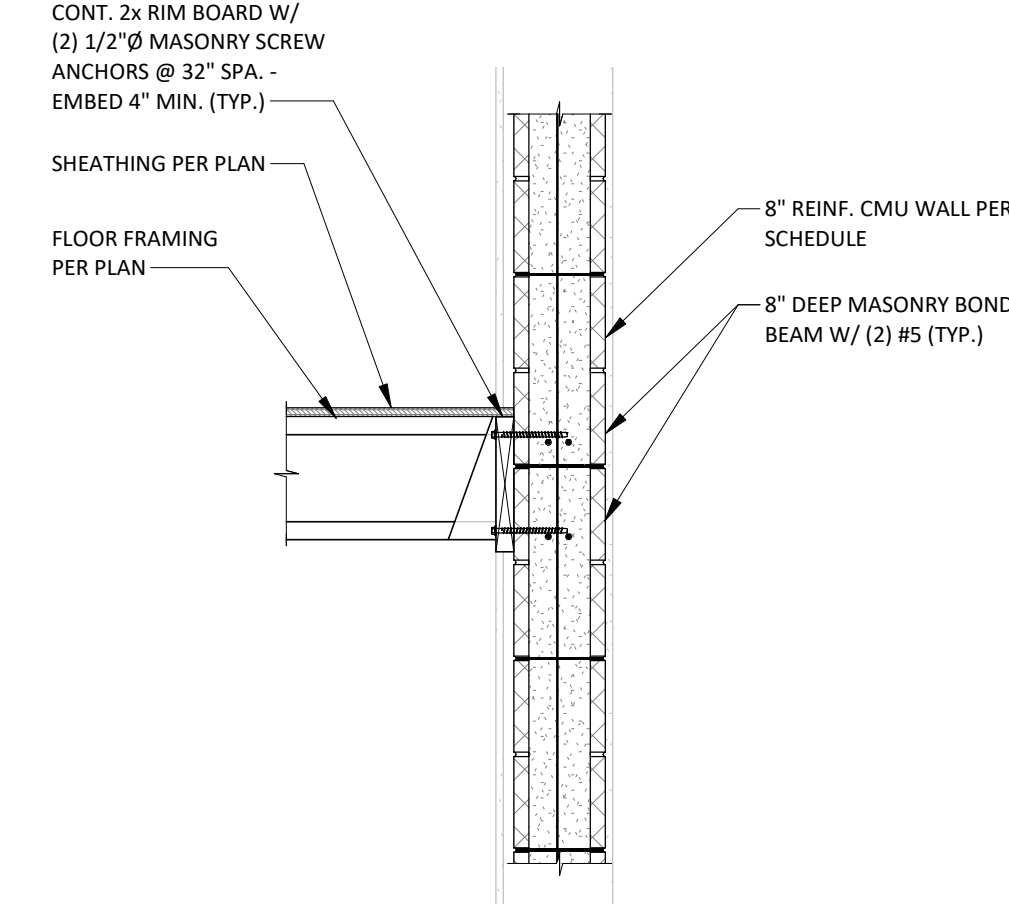
SECTION | 08  
3/4" = 1'-0" | S-704



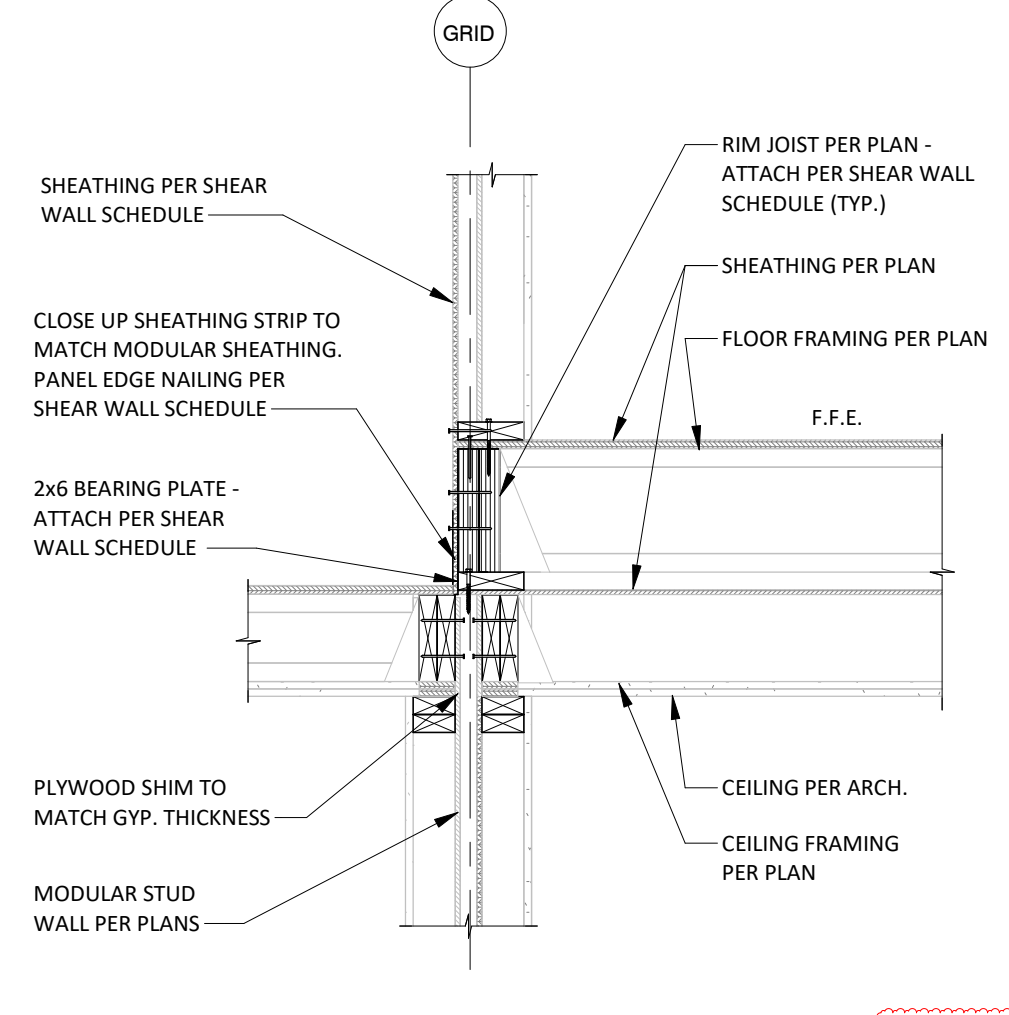
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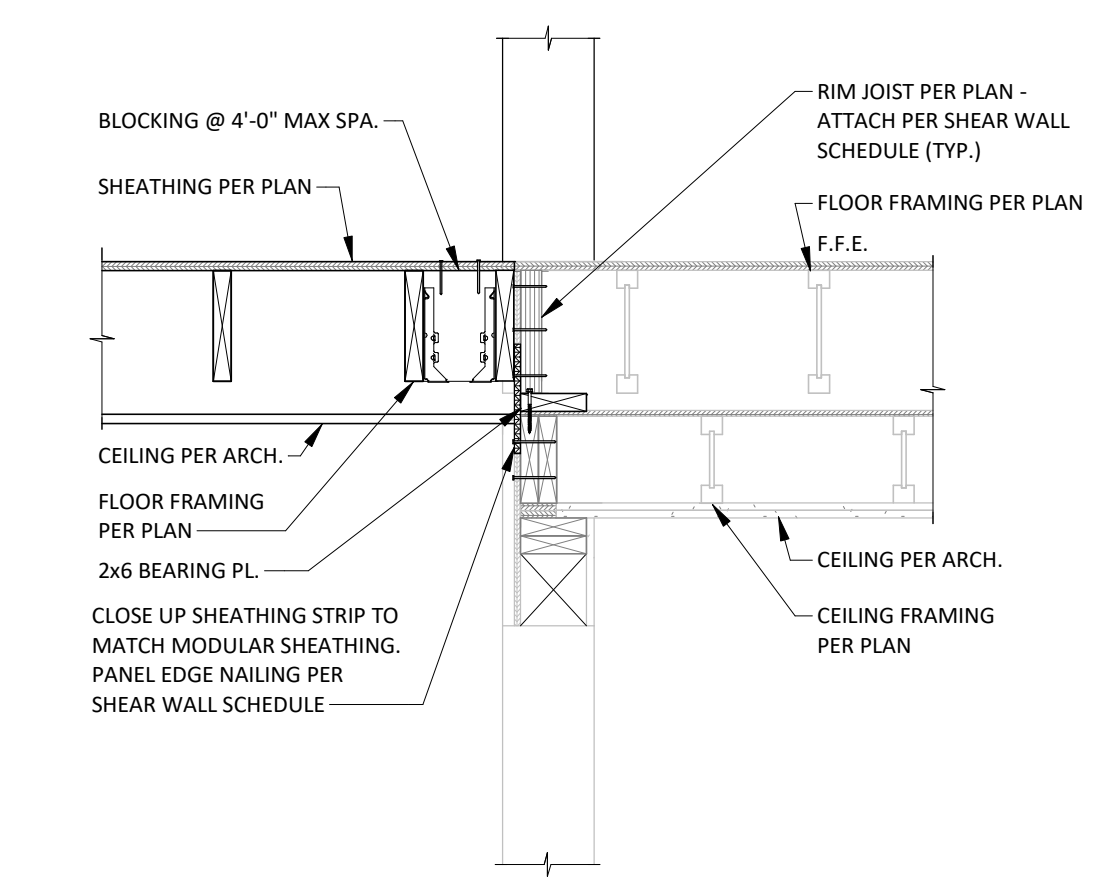
SECTION | 10  
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SECTION | 11  
3/4" = 1'-0" | S-704

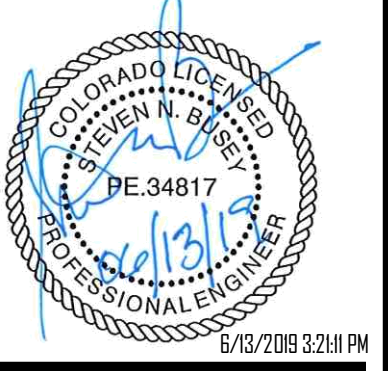
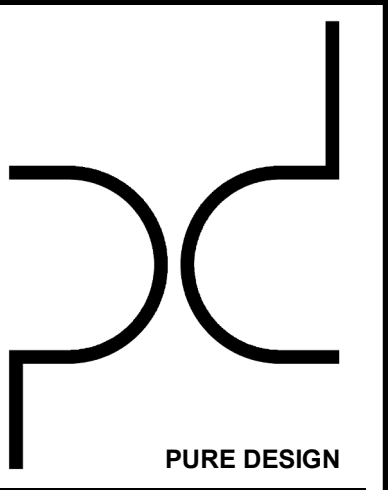


SECTION | 12  
3/4" = 1'-0" | S-704



SECTION | 13  
3/4" = 1'-0" | S-704

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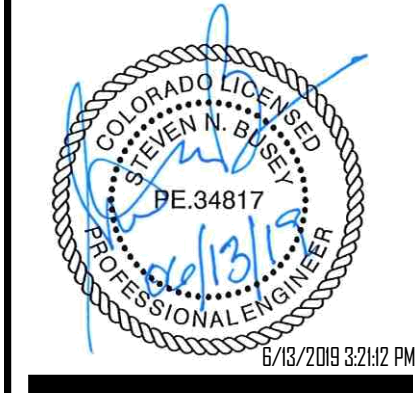
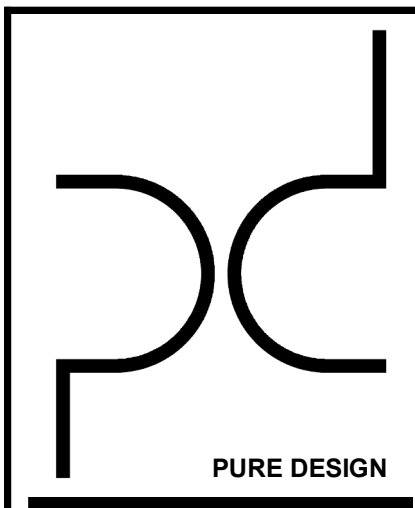
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HOTEL FRAMING DETAILS

**S-704**



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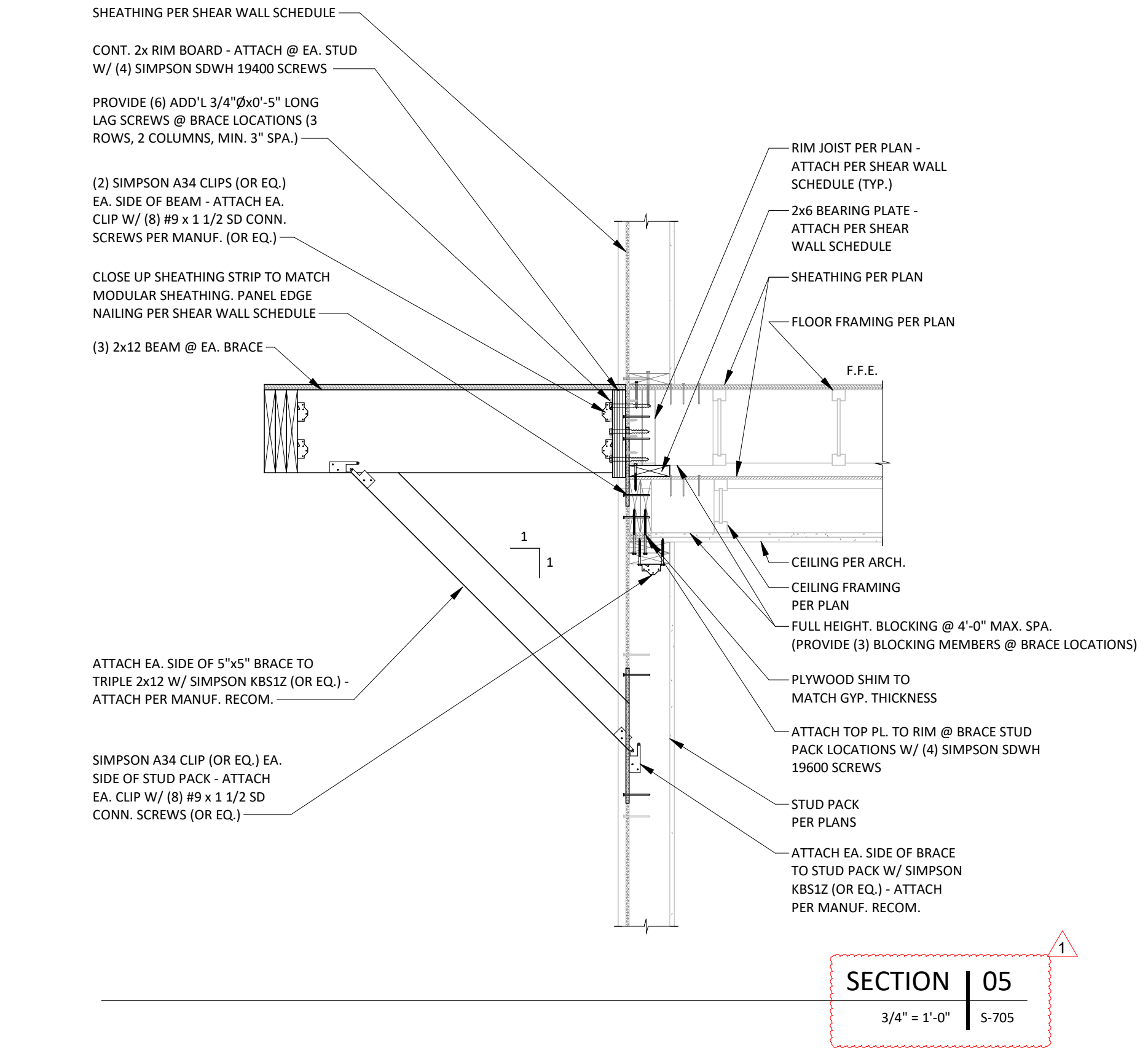
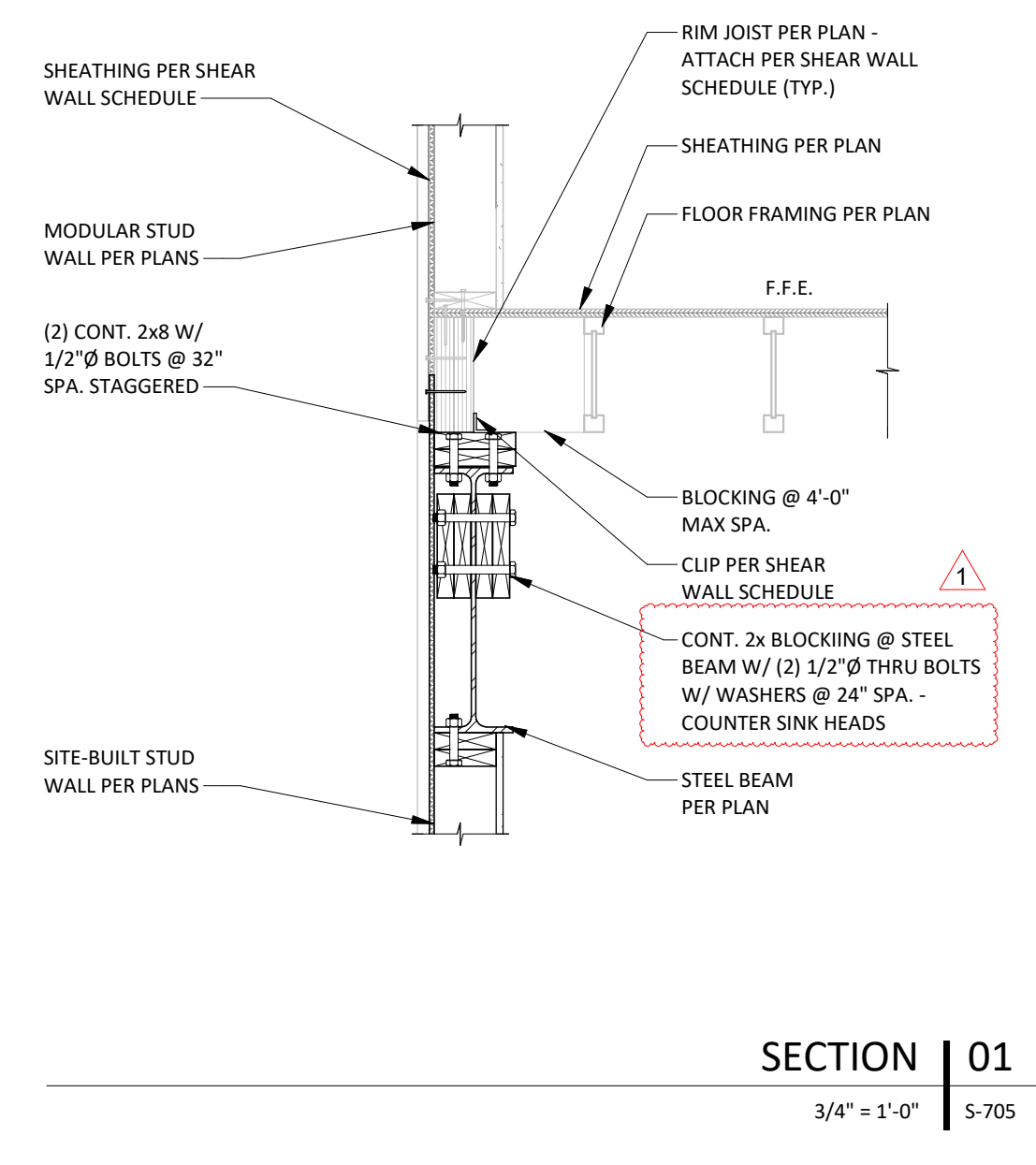
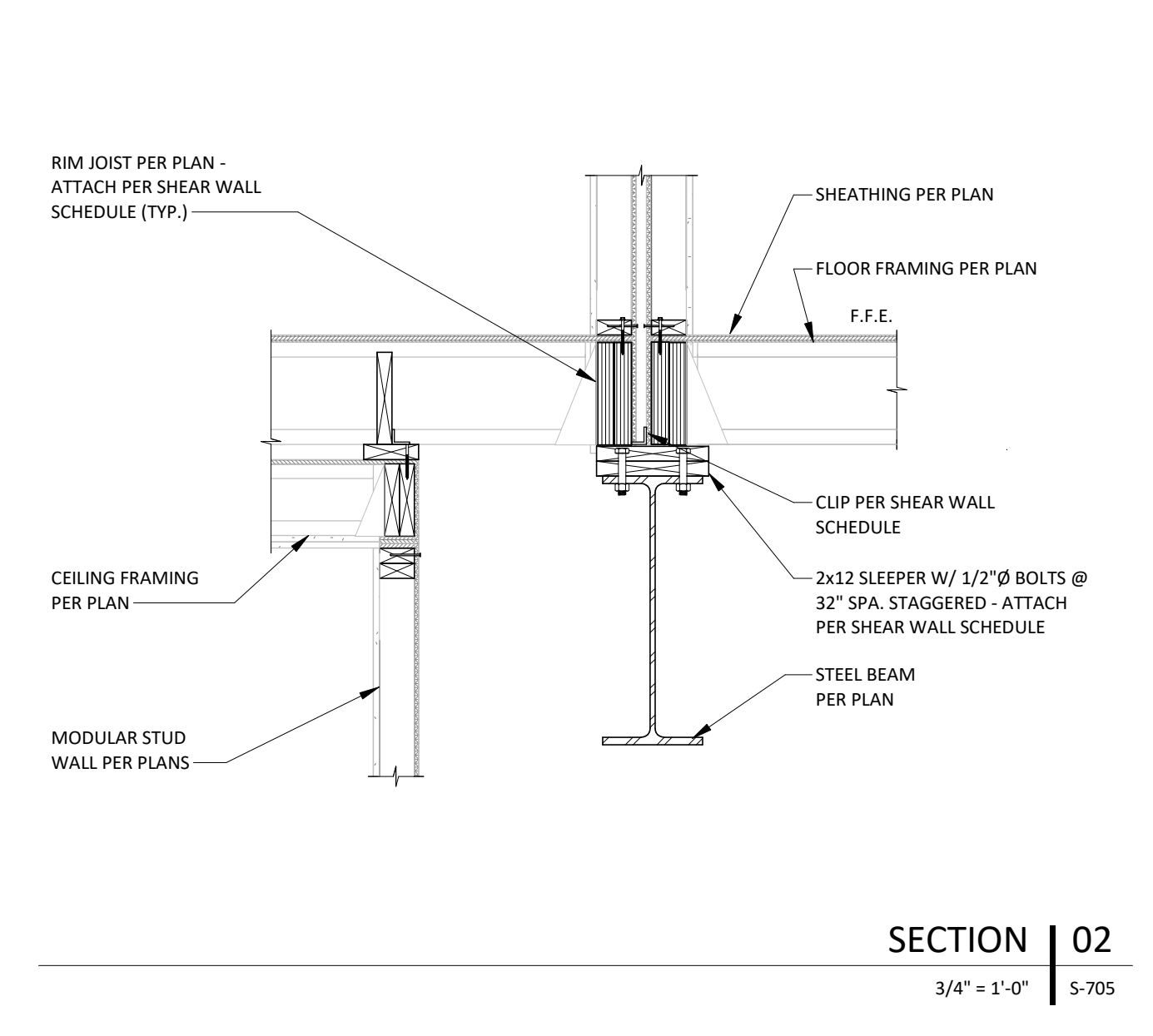
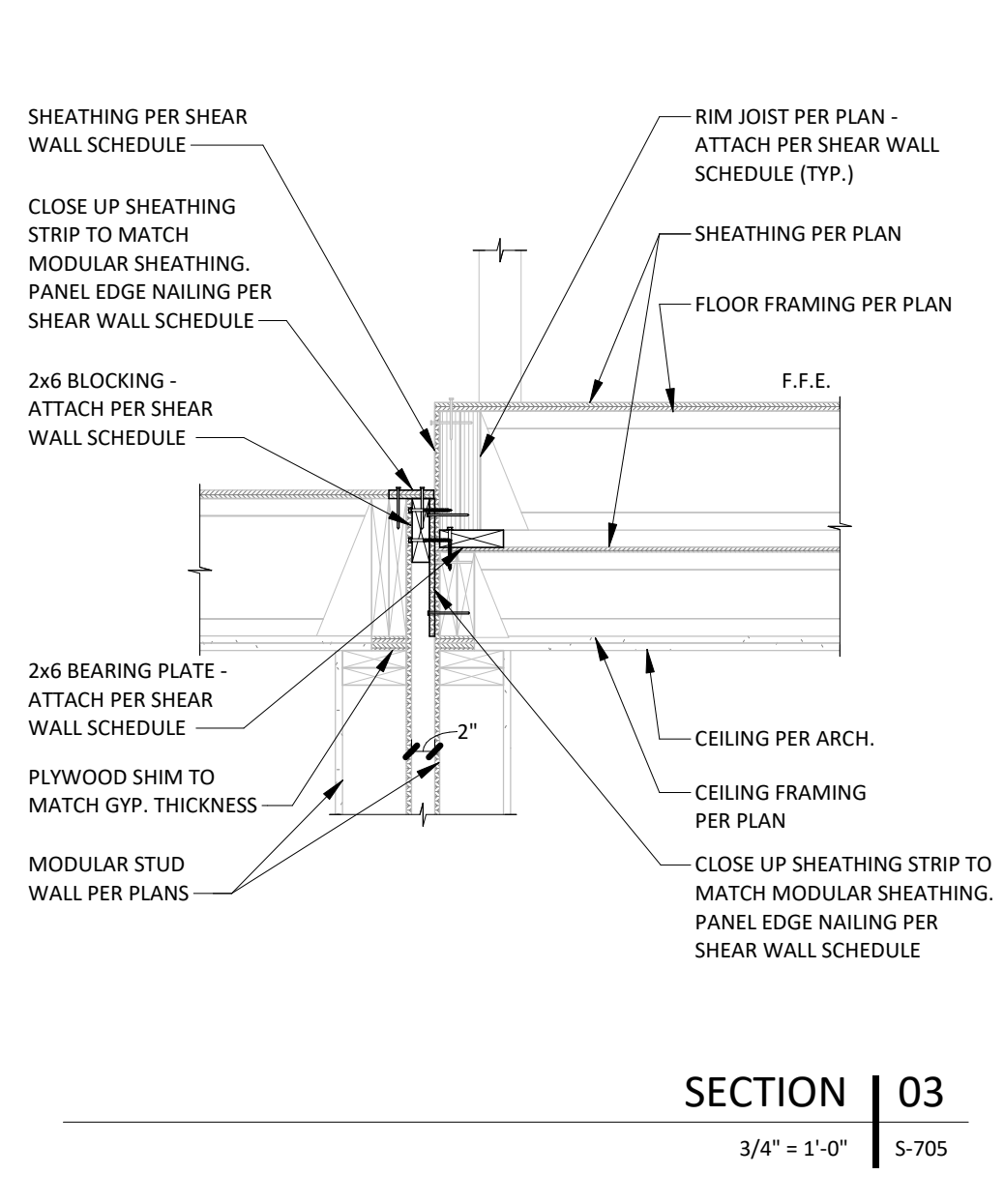
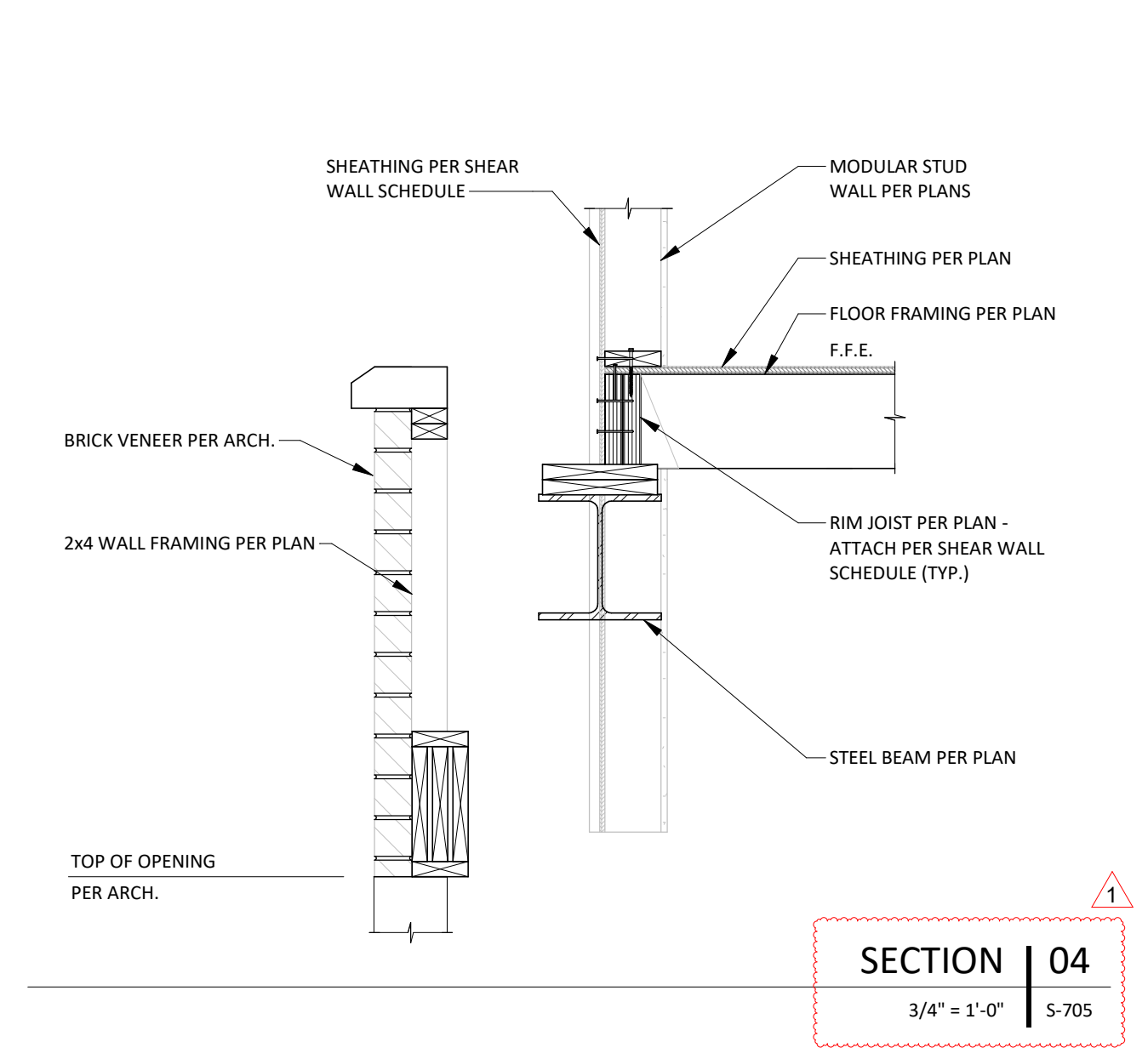
JOB NO: 18-007  
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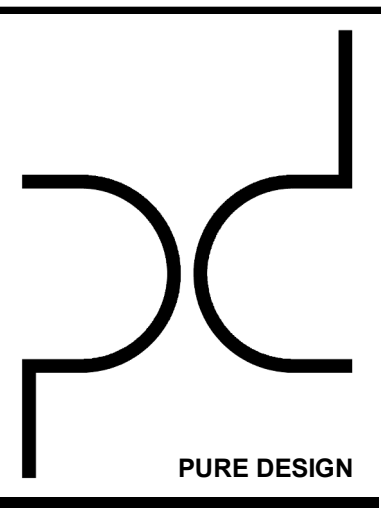
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HOTEL FRAMING DETAILS

**S-705**

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AC HOTELS

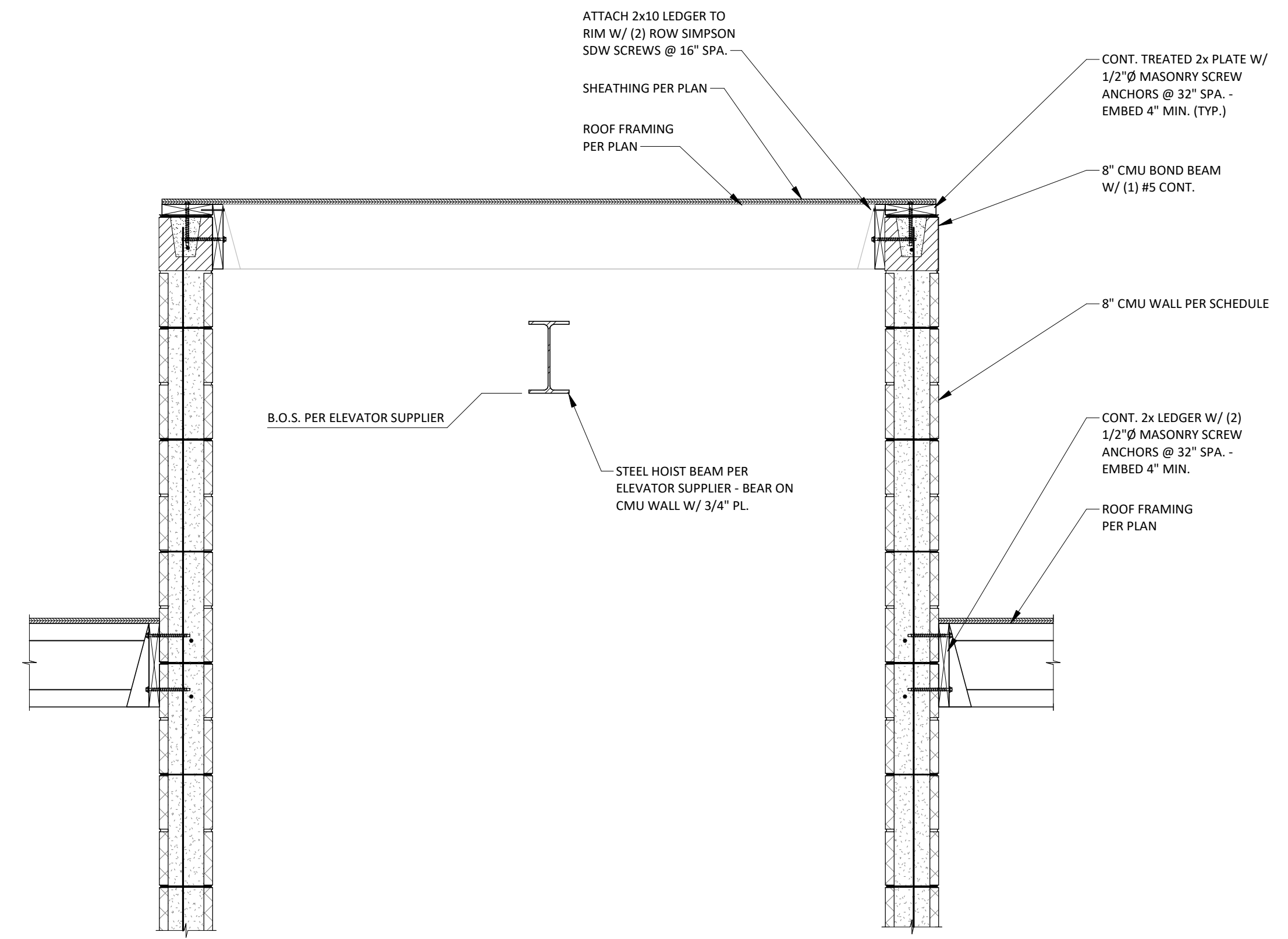
JOB NO: 18-007  
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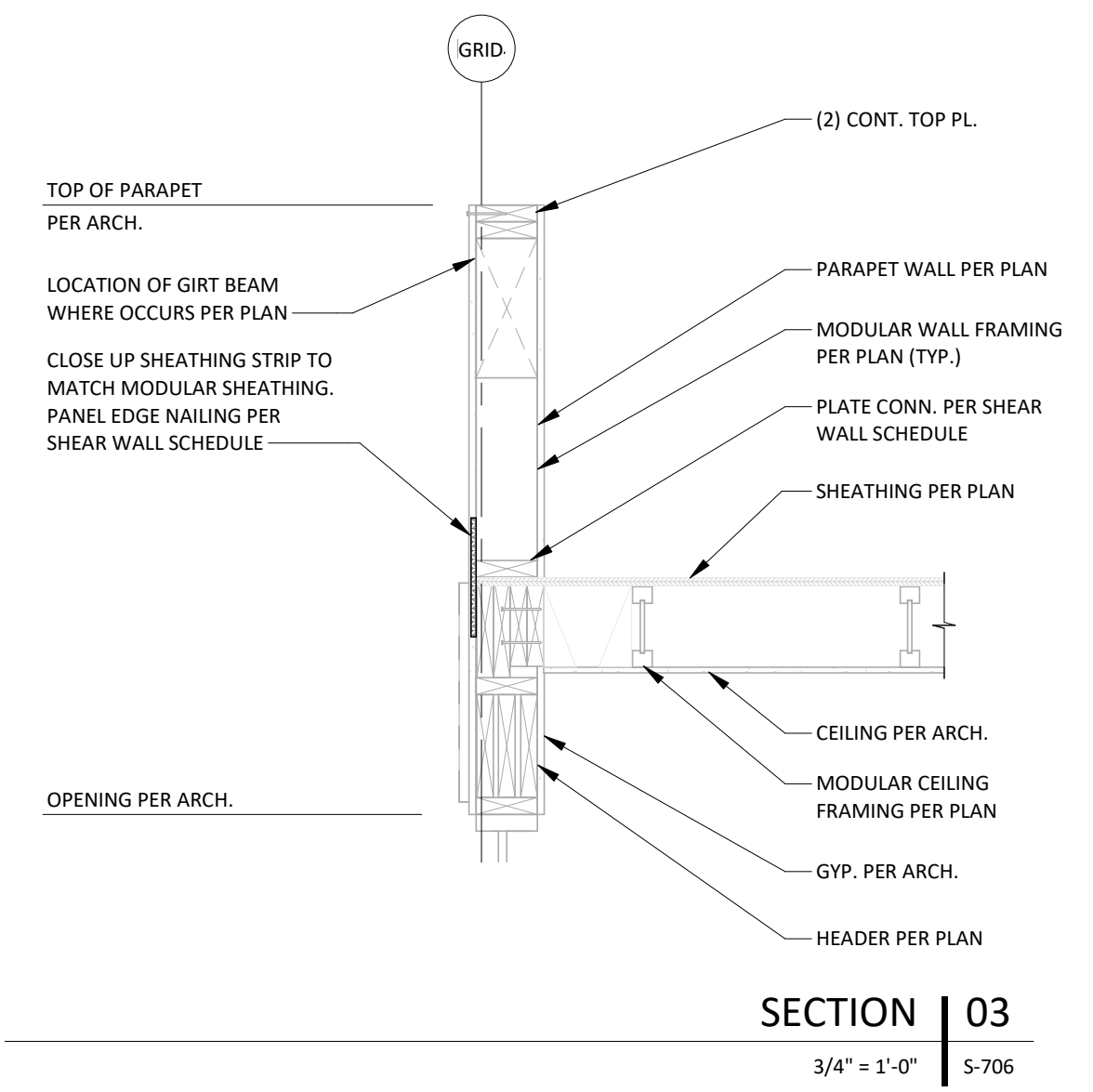
HOTEL FRAMING DETAILS

**S-706**

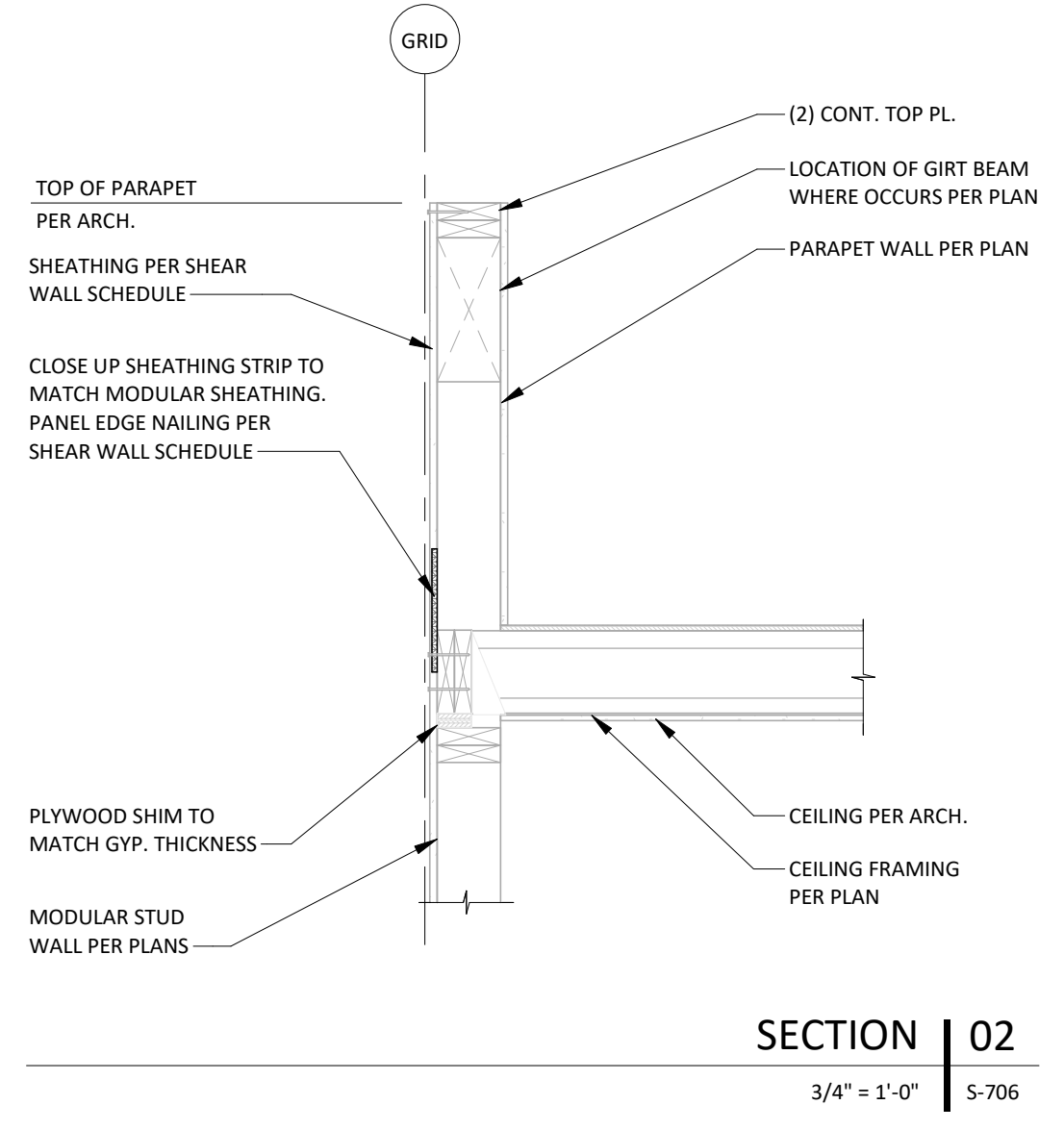
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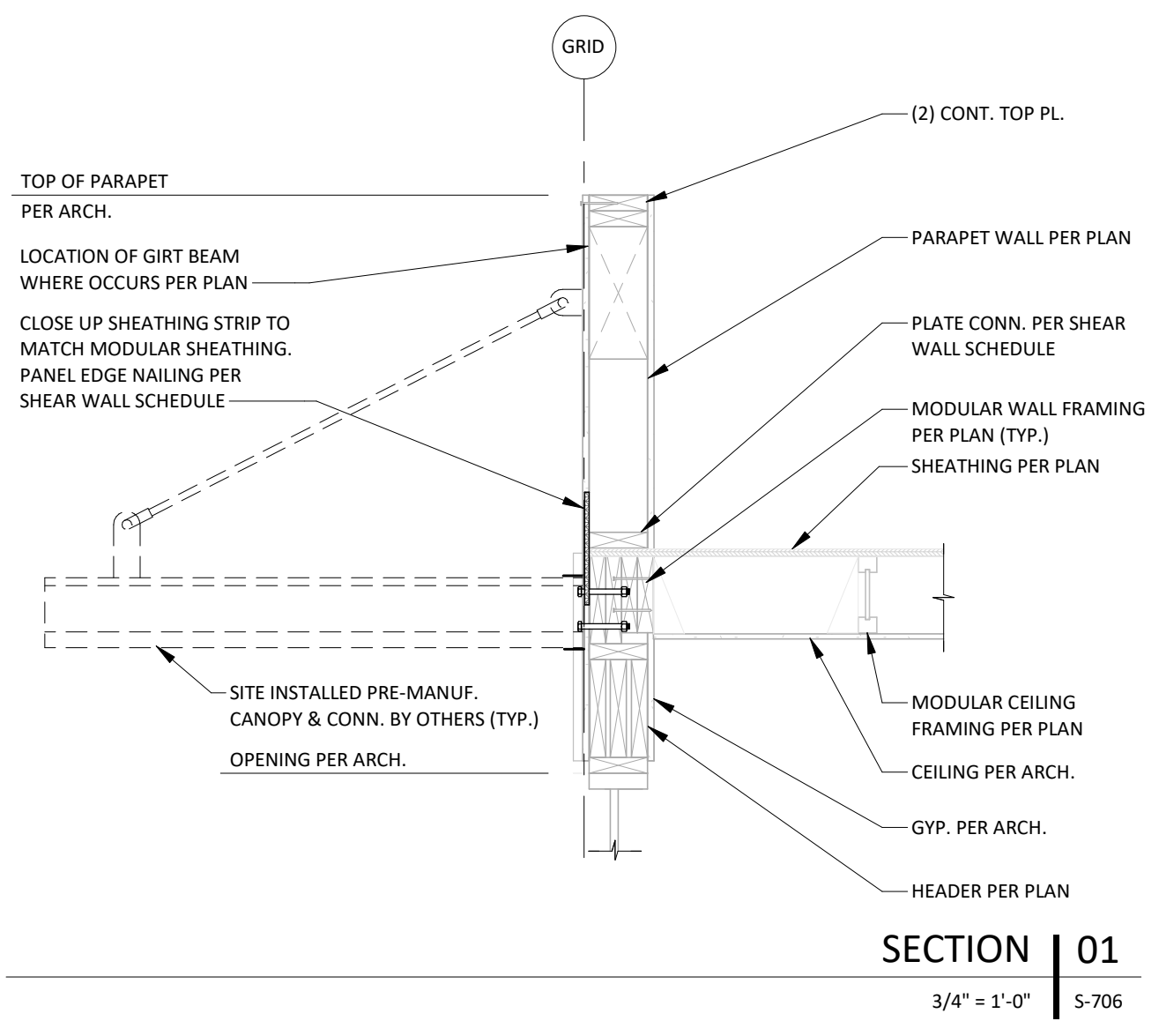
**SECTION 08**  
 3/4" = 1'-0" | S-706



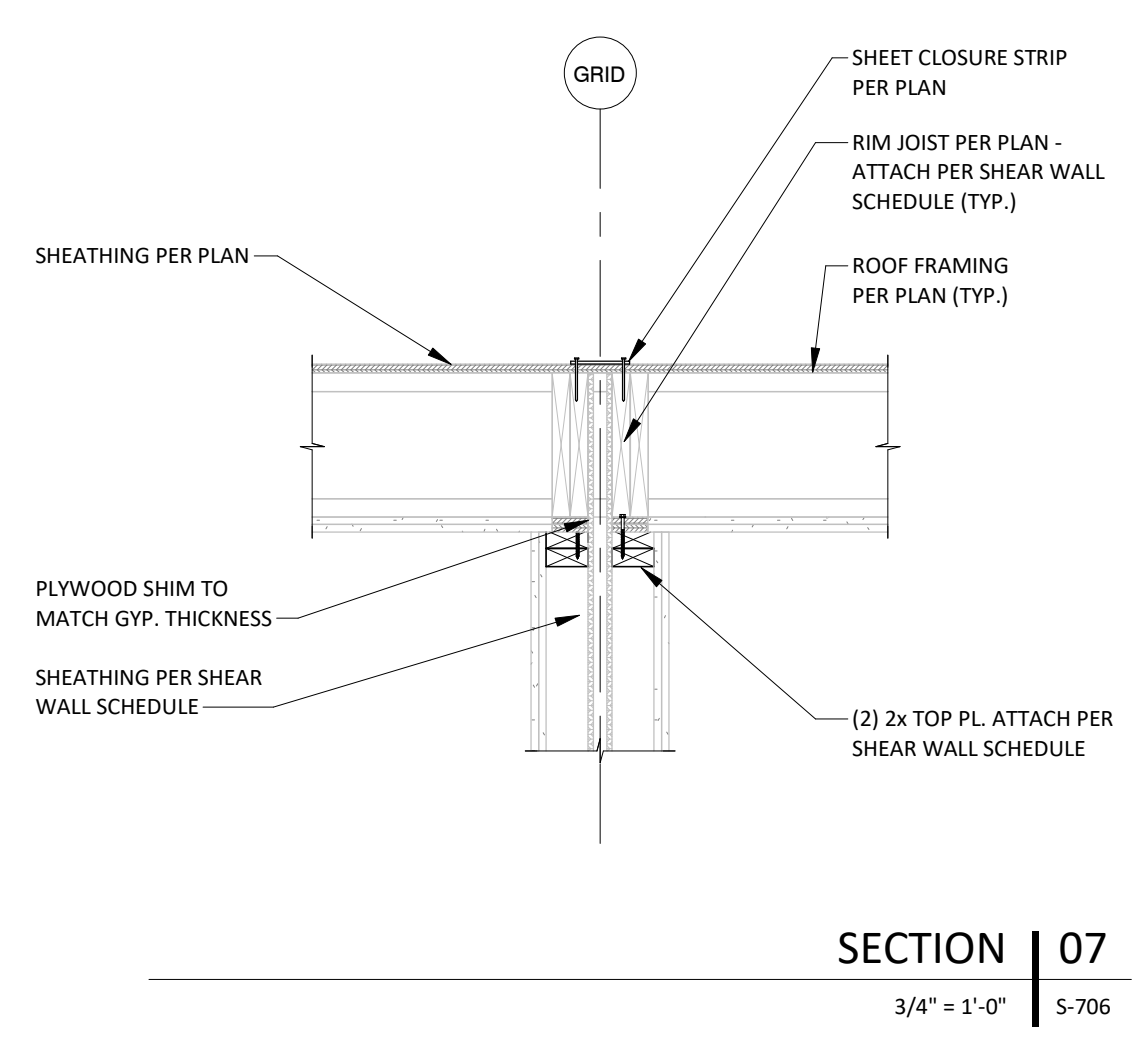
**SECTION 03**  
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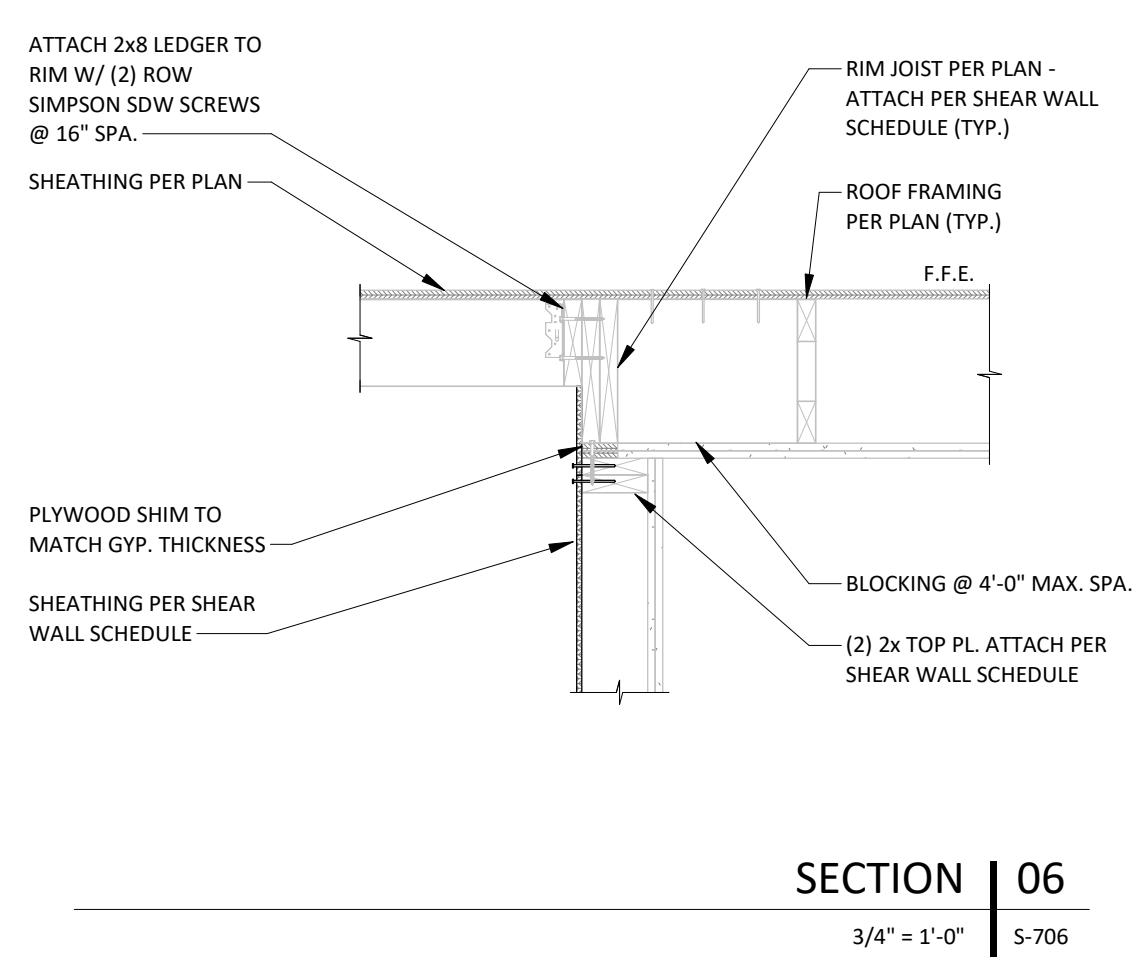
**SECTION 02**  
 3/4" = 1'-0" | S-706



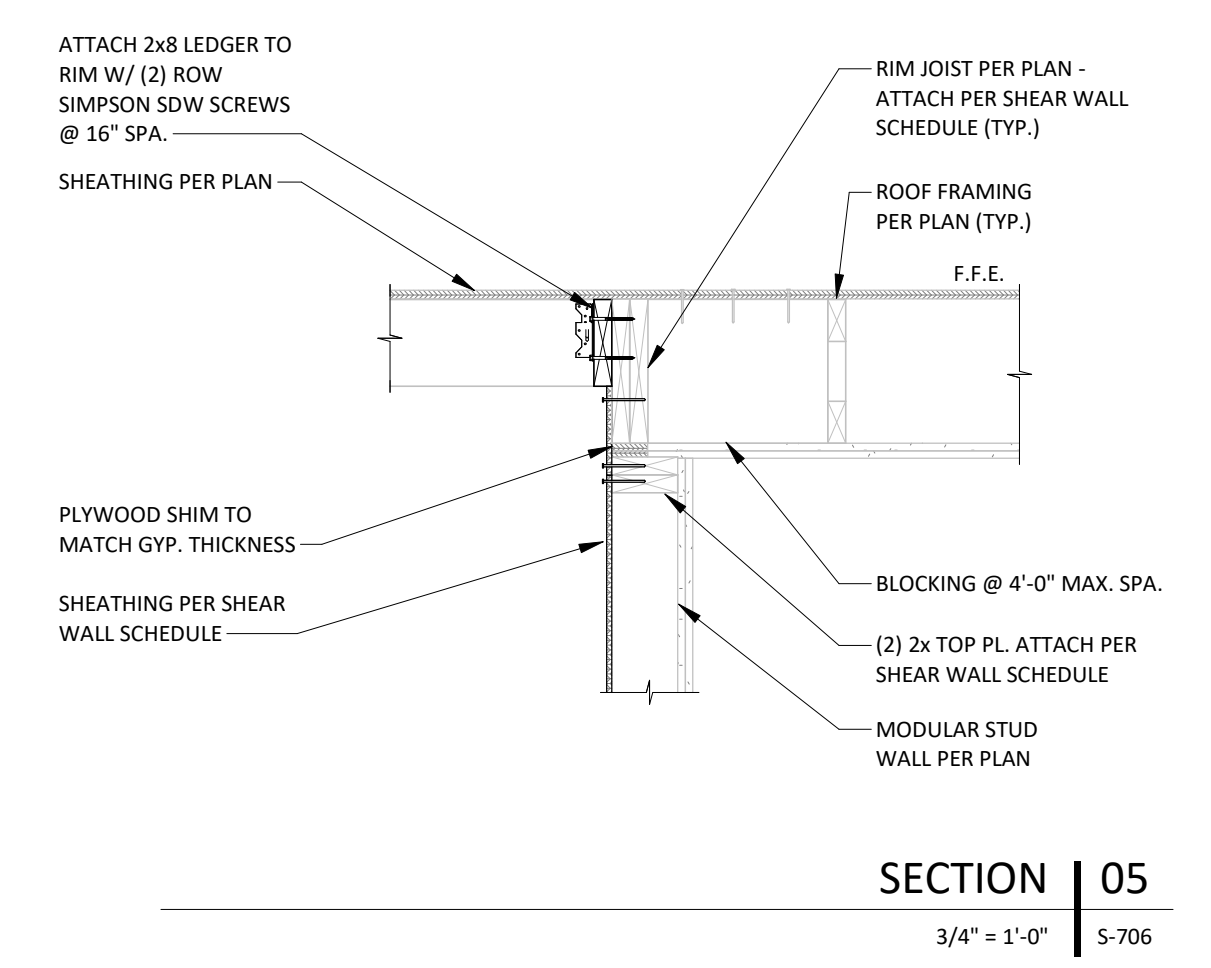
**SECTION 01**  
 3/4" = 1'-0" | S-706



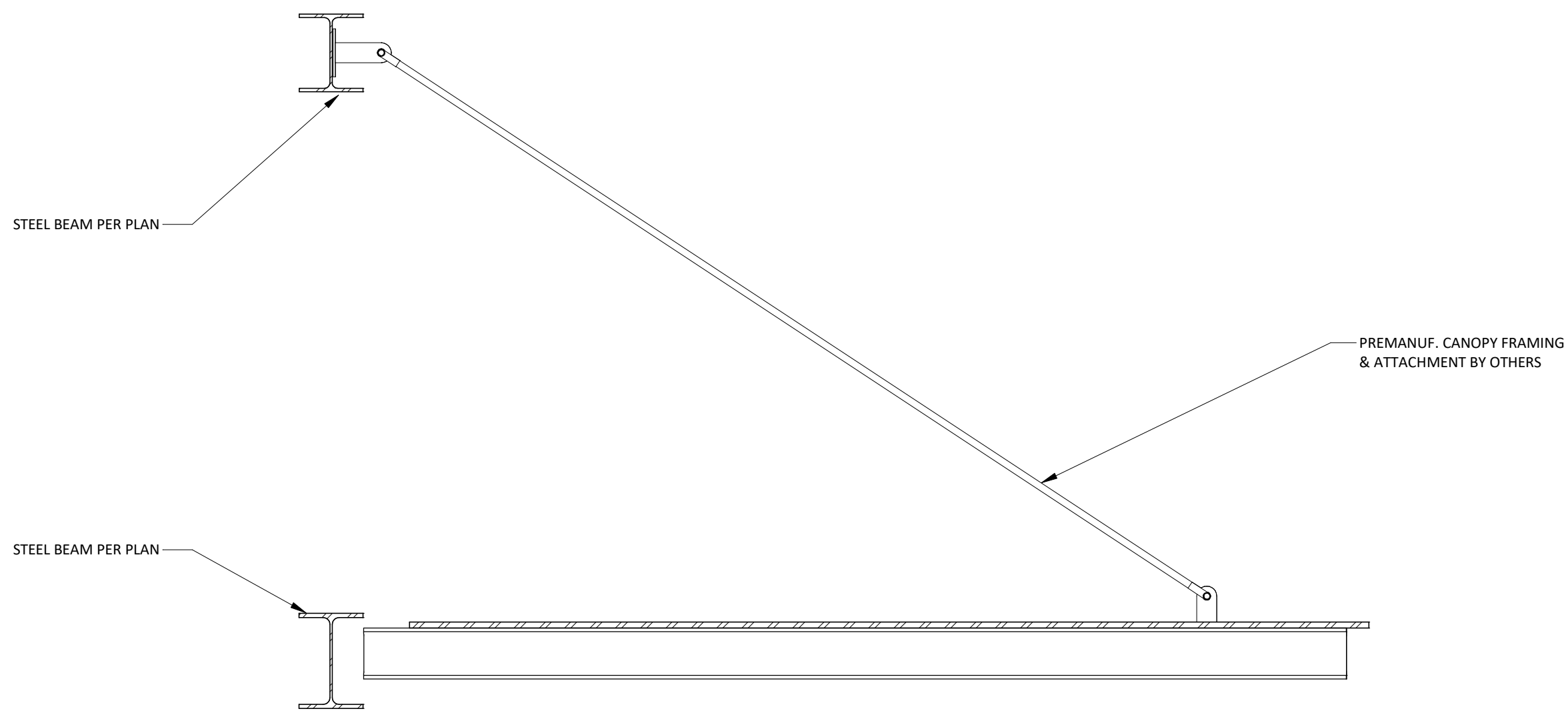
**SECTION 07**  
 3/4" = 1'-0" | S-706



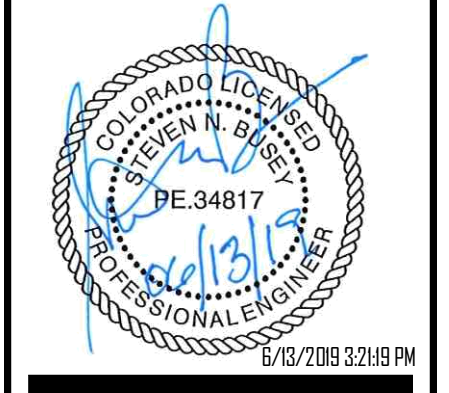
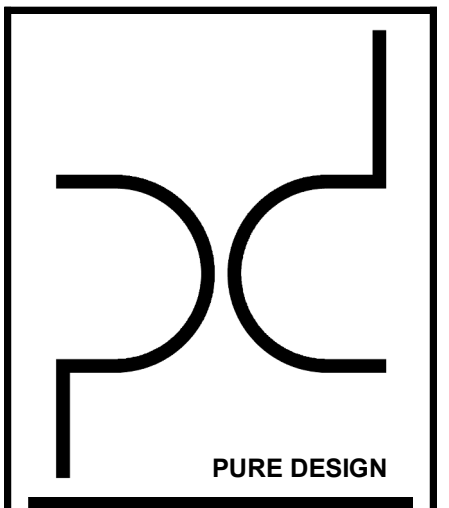
**SECTION 06**  
 3/4" = 1'-0" | S-706



**SECTION 05**  
 3/4" = 1'-0" | S-706



SECTION | 01  
3/4" = 1'-0" | S-707



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JOB NO: 18-007  
DATE: 2018.03.07

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**S-707**

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